

City of Stevenson

Phone (509) 427-5970 Fax (509) 427-8202 7121 E Loop Road, PO Box 371 Stevenson, Washington 98648

October 2023 Planning Commission Meeting

Monday, October 09, 2023

6:00 PM

A. Preliminary Matters

1. Public Comment Expectations:

In Person: Attendees at City Hall should follow current CDC and State guidance regarding use of masks, social distancing, and attendance.

Webinar: https://us02web.zoom.us/s/85637388112 Conference Call: +1 253 215 8782 or +1 346 248 7799 ID #: 856 3738 8112

Commenters must raise their hand and be acknowledged by the Chair. Individual comments may be cut off after 3 mins. Disruptive individuals may be required to leave the meeting. Persistent disruptions may result in the meeting being recessed and continued at a later date.

Tools: *6 to mute/unmute & *9 to raise hand

- **2. Public Comment Period:** (For items not located elsewhere on the agenda)
- **Minutes:** September 11, 2023 Planning Commission Meeting Minutes
- **B. New Business**
- 4. Shoreline Permit Request: (SHOR2023-01 Rock Cove Hospitality)
 - a. Appearance of Fairness Disclosures
 - b. Presentation by Staff
 - c. Presentation by Applicant

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- d. Public Hearing
 - 1. Comments in Favor
 - 2. Comments in Opposition
 - 3. Comments Neither in Favor nor Opposition
 - 4. Close Public Hearing
- e. Commission Deliberation
- f. Decision

C. Old Business

5. Subcommittee Reports: Updates from Subcommittee leads and discussions on Downtown Parking and Annexation Policy

D. Discussion

- 6. Thought of the Month:
 - Community Submission Walla Walla Design Standards

https://www.codepublishing.com/WA/WallaWalla/html/WallaWalla20/WallaWalla2 0178.html

7. Staff & Commission Reports: Shoreline Public Access Plan, Septic/Sewer Discussion, Parks Planning, Online Building Permitting

E. Adjournment

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DRAFT Minutes Stevenson Planning Commission Meeting Monday, September 11, 2023 6:00 PM

Planning Commission Chair Jeff Breckel called the meeting to order at 6:07 p.m.

MEMBERS PRESENT PC Chair Jeff Breckel; Commissioners Anne Keesee, Davy

Ray, Charles Hales. Commissioner Zettler was not in

attendance.

STAFF PRESENT Community Development Director Ben Shumaker, Planning

& Public Works Assistant Tiffany Andersen

GUESTS PRESENT Stevenson Public Works Director Carolyn Sourek

PUBLIC PRESENT Mary Repar, Nick Bossoff, Rick Levitt, Ann Bane, Robert Bane,

Robert Shandly, Therese Stacy, Ren Grendall, Dave Denning,

Dustin Conroy, Bernard Versari

A. Preliminary Matters

1. Public Comment Expectations

PC Chair Breckel asked Community Development Director

Shumaker to explain usage of online tools for remote

participants: *6 to mute/unmute & *9 to raise hand. Commenters must raise their hand and be acknowledged by the Chair.

Individual comments may be limited to 3 minutes. Disruptive individuals may be required to leave the meeting. Persistent disruptions may result in the meeting being recessed and

continued at a later date.

2. Public Comment Period For items not located elsewhere on the agenda)

Mary Repar invited all to a Community Roots Collaborative presentation on affordable housing in Vancouver. She shared information on a fermentation class to be held September 23rd at the Stevenson Library, and a plant and seed swap scheduled for

November.

3. August 14th, 2023 Minutes Minutes from the August 14th, 2023 Planning Commission

meeting were approved unanimously as presented upon a motion by **Commissioner Keesee**, seconded by **Commissioner Hales**.

B. New Business

4. Final Plat Review: Review Chinidere Subdivision Request to Vary the

Approved Preliminary Plat - Pedestrian Pathway.

Community Development Director Shumaker briefly explained the purpose of the Plat review for the Chinidere

Subdivision. The request is to modify the location of a pedestrian trail. Three alternative plans were submitted by the developer. Staff also provided recommendations. Any plat revision needs the support of the Planning Commission, with final approval

provided by the City Council.

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It was explained by **PC Chair Breckel** this was not a public hearing, but comments were welcome regarding the proposal.

There were three developer alternatives presented: The first removed the path completely, the second reduced sections of the path near several lots, and the third re-routed the path through a White Oak preservation area and then north along several other lot lines.

Much of the discussion centered on the pathway contained in the original plat that would have encircled the development. Dustin Conroy of Pioneer Surveying shared information on the difficulty of installing a path along certain lots due to the topography. He noted the installation of a sewer line had changed the site.

A site map was presented showing the various tracts.

Rick Levitt, developer of the site expressed his views on a privately owned path maintained by the Homeowner's Association requiring public access. A number of residents in the development commented on privacy loss and security as the pathway closely skirted their property.

After receiving meeting participant's comments throughout the discussion, **Community Development Director Shumaker** provided a recap of staff recommendations that could be made by the Planning Commission:

- 1. Along the west side of the subdivision remove a requirement to construct any path in that area or locate a specific easement within that area; and instead grant a blanket easement for the city to construct a path in the future;
- **2.** Remove the proposed pathway along the nother boundary of the subdivision (marked in red), remove the crosswalk drops and reconstruct a continuous curb and sidewalk;
- **3.** In the area marked in blue, relocate the path off the perimeter so it's only encumbering two lots and then engineer a way to get pedestrians safely from the path to the sidewalk.

Commissioner Hales motioned to approve the devloper's Option 1. The motion died from lack of a second.

Following further discussion **Commissioner Keesee motioned** to support Option 1, with the addition of the blue segment. The motion did not advance due to lack of a second. Further discussion ensued.

Commissioner Ray then motioned to accept staff recommendation for Option 2. **Commissioner Keesee** seconded.

Prior to the vote additional discussion took place. Points about the gradient, and accessing the oak habitat area continued to be a concern. **Commissioner Keesee** rescinded her second, and the motion died.

Commissioner Hales then motioned to accept Option 1 with several conditions: the removal of not just the path requirements

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that the staff has recommended from the red easement ,but also the other path required on the east side shown in blue.

Prior to a second, **Commissioner Breckel** requested to amend the motion to ensure provision of an easement on the west side for open space. A further consideration was added to ensure curbs and sidewalks were restored.

Voting aye: Commissioners Breckel, Hales, Keesee.

Voting nay: Commissioner Ray.

Agenda amendment:

It was agreed to amend the agenda order to allow Item 7, Subcommittee Reports, be moved forward.

7. Subcommittee Reports

PARKING

Commissioner Keesee provided an update on the recent parking study. Volunteers served two hour shifts on August 24th and 26th. The intent is to obtain data in order to help develop a parking master plan which will come before the Planning Commission in October.

ANNEXATION

PC Chair Breckel and Commissioner Hales are participating, as is City Councilmember Michael Johnson, and Community Development Director Shumaker. A number of considerations are being reviewed regarding the development of an annexation policy. PC Chair Breckel noted they are working to establish a policy framework with clear, usable criteria in the event someone comes to the city requesting annexation. Public Works Director Carolyn Sourek has been providing needed answers on how to logically extend services.

5. Planning Commission Bylaws:

Consider Amending Packet Delivery Day

Following a short discussion it was agreed to have the agenda emailed if available on Wednesday, followed by the packet on Friday. No bylaw amendment was required.

C. Old Business

6. Shoreline Public Access:

Community Development Director Shumaker highlighted changes and clarifications made to the final version of the Shoreline Public Access plan. Comments and concerns regarding a paved pathway on Bob's Beach were addressed. It was pointed out in the plan it is considered a potential consideration, not a scheduled project. or design. Following further discussion the Planning Commission recommended approval of the plan with the community concerns noted.

D. Discussion

8. Thought of the Month

- Trends in Vashon Island

https://www.seattletimes.com/business/realestate/vashonisland-grapples-with-affordablehousingshortage/

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- EV Planning

https://planning.org/planning/2023/summer/how-to-pave-the way for equitable EV adoption

9. Staff and Commission Reports

Septic/Sewer Discussion, Engineering Standards Update

The City Council will be considering changes to requirements for sewer connections. A public hearing and workshop will be held at the upcoming City Council meeting on September 21st.

Public Works is looking to amend Stevenson's engineering standards. It's current format is difficult to use by staff and developers due to lack of clarity and organization. The Planning Commission will likely be reviewing suggested changes in street requirements at some point.

10. Adjournment

MOTION to adjourn at 8:25 p.m. was approved unanimously.

Minutes recorded by Johanna Roe.

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(509)427-5970

7121 E Loop Road, PO Box 371 Stevenson, Washington 98648

TO: Planning Commission

FROM: Ben Shumaker
DATE: October 9th, 2023

SUBJECT: Shoreline Substantial Development Permit – SHOR2023-02 - Rock Cove Hospitality

Introduction

The Planning Commission is asked to review a proposal by FDM Development for a water-enjoyment commercial use on the shorelands of Rock Cove. The proposal is subject to review under SMC 18.08 and the Stevenson Shoreline Master Program (SMP). Complete application for the proposal was accepted on August 11, 2023 (Attachment 1). A template decision has been prepared (Attachment 2). The template is not a recommendation of approval as some key items of discussion are appropriate based on the site's history and public comments received on the project (Attachment 3).

The proposal is similar to a project approved by the City under the previous SMP SHOR2020-01. Underground site infrastructure was installed, however, the previous permit expired and the project was subsequently redesigned.

Key Discussions

Three topics warrant additional Planning Commission Discussion.

<u>Cultural Resources</u> – A cultural resources report was prepared to advance the 2020 project. The report recommended inclusion of inadvertent discovery procedures during construction. Public comments were solicited on the cultural resources report, and the City received a request to require the presence of a monitor during construction activities. The City Council disregarded the request when they issued the Shoreline Permit. The Planning Commission is faced with the same decision in its review under the new SMP. Pink highlighted text is available should the Planning Commission seek to require cultural resource monitoring.

<u>Mitigation Sequence</u> – Before fish and wildlife habitat conservation areas are disturbed, proposals must do their best to avoid, then minimize, then rectify and compensate for impacts. Public comments from State agencies challenge the proposed efforts to avoid and minimize impacts. Staff expects applicant testimony to address these comments and is ready to draft findings and conditions if necessary to assist the Planning Commission discussion of the topic.

Off-Site Mitigation – The proposal includes enhancement of fish and wildlife habitat conservation areas. The enhancement is required by the Stevenson Critical Areas Code because riprap at the site reduces the regulatory base habitat buffer area to less than 50% of what it would be on a greenfield site. With the enhancement requirement, the code removes the opportunity for additional on-site mitigation. The application proposes on-site mitigation to compensate for buffer impacts. This would effectively double count mitigation efforts. The issue was brought to the City's attention via public comments and staff expects applicant testimony to address alternate—off-site—mitigation for the project impacts. Staff is ready to draft findings and conditions if necessary to assist the Planning Commission's discussion after testimony is received

<u>Public Access</u> – The proposal includes visual public access to Rock Cove via pedestrian pathways. The proposal alters the location of existing public easements to better suit the site's redesign. The proposal also involves reducing and/or not developing public access where some easements already exist. The reduction makes it difficult to assess the approval criteria, in part because the issue of "feasibility" as that term is defined in the SMP is not addressed. Staff anticipates applicant testimony related to this topic and is prepared to draft findings and conditions if necessary to assist the Planning Commission discussion.

Effect

The project is very close to satisfying all applicable criteria, however additional information is necessary before a permit can be justified. Depending on the information provided at the hearing and the Planning Commission's preferences, a decision could be made tonight. Alternatively, the record could be left open and the public hearing continued to a future date.

Prepared by,

Ben Shumaker Zoning Administrator

Attachment

- Application
- Template Decision
- Written Public Comments
- Public Access History
- Public Comment Responsiveness Summary (Available at the Meeting)

Tracking	Number		



SHORELINES APPLICATION

Mail: PO Box 371, Stevenson, Washington 98648 Email: planning@ci.stevenson.wa.us Phone: (509)427-5970

	Brad Kilby, AICP
Applicant	t/Contact: Brad Kilby, AICP ag Address: 205 SE Spokane Street, Suite 200 Portland, OR 97202
	503-221-1131 E-Mail Address: bradk@hhpr.com
	Owner (when applicable): Dean Maldonado
Mailir	ng Address: PO Box 353 Ridgefield, WA 98642
	360-719-0276 E-Mail Address: dean@fdmdevelopment.com
	If There are Additional Property Owners, Please Attach Additional Pages and Signatures as Necessary
	Submittal Requirements Applicants must provide the following information for all Shoreline Proposals. The City will not accept applications without the required information. Application Fee (\$) Agreement to Pay Outside Consulting Fees (When applicable) Documentation of Preapplication Meeting Completed Application Signed by the Property Owner Associated Permit Applications (When applicable)
	The following information is required as a complete application for <u>Minor Project Authorization</u> . Minor Project Authorizations are exempt from Shoreline Substantial Development Permit requirements but not exempt from compliance with the City's Shoreline Management Program.
	A Narrative describing why the project qualifies for consideration as an MPA A Statement of Compliance with applicable sections of the SMP Such Additional Information outlined as necessary in the preapplication meeting documentation or in response to the application as-submitted
	The following information is required as a complete application for Shoreline Substantial Development Permits and Shoreline Conditional Use Permits. Shoreline Substantial Development Permits assure proposals' consistency with the provisions of the Shorelines Management Act and the SMP. Shoreline Conditional Use Permits provide a system which allows flexibility in the use regulations of the SMP while assuring consistency with the Shorelines Management Act and the SMP.
	A Complete Site Plan including parcel boundary, OHWM, vegetation characterization, critical areas locations, and dimensions and locations of all existing and proposed structures and improvements A Narrative describing the proposal in detail and how the proposal is consistent with the SMP
	Continued on Page 2



Shorelines Application

	Project Construction Details (e.g., building elevations, construction timelines, grading plans, (re)vegetation plans, etc.) Technical Assessments, Management Plans, and Mitigation Plans as necessary and prepared by qualified professionals Such Additional Information outlined as necessary in the preapplication meeting documentation or in response to the application as-submitted
	In addition to the information required for Shoreline Substantial Development Permits, the following information is required as a complete application for applications <u>Shoreline Variances</u> . Shoreline Variances grant relief from specific bulk, dimensional or performance standards set forth in the SMP. Shoreline Variances do not vary the allowed use of a shoreline.
	A Site Plan indicating where development could occur without a variance and the physical features and circumstances on the property that provide the basis for a variance request and the location of adjacent structures and uses. A 3D, SketchUp-compatible Model of the proposal in context with City-provided adjacent models
Tax Parcel Numbe Shoreline Waterbo Water Supply Sou	rce: City Well Sewage Disposal Method: City Septic Sewage Disposal Method: City Septic
proposal, with the underst Master Program. I/we hereby provide writte	property owners of the real property described in this proposal, our signatures indicate our approval of this tanding that the proposal is subject to review, approval, and/or denial under SMC Title 18 and the Shoreline an authorization for the City to reasonably access to the subject property to examine the proposal and carry ties of the Stevenson Municipal Code.
Incomplete	e applications will not be accepted. • Ensure all required submittals are included.
Signature of A	Pradd Kilby Digitally signed by Brad Kilby Righellis, Inc.*; OU-Planning, CN-Brad Kilby Righellis, Inc.*; OU

For Official Use Only

Date Application Received:

Date Application Complete:

Owner:

Rock Creek Cove Hospitality Revised Land Use Application Narrative & Findings Document

Site Plan Review, Substantial Shoreline Development Permit, and Plat Vacation

FDM Development Inc.

dean@fdmdevelopment.com

(360) 719-0276

Dean Maldonado

Engineer: Bruce Haunreiter, P.E.

Harper Houf Peterson Righellis Inc.

1220 Main Street, Suite 150

Vancouver, WA 98660 bruce@hhpr.com (360) 750-1131

Planner: Brad Kilby, AICP

Harper Houf Peterson Righellis Inc. 205 SE Spokane Street, Suite 200

Portland, OR 97202 bradk@hhpr.com (503) 221-1131

Tax Lot Number: 02070100130300 and 02070100130200

Parcel Size: 6.4 acres

Zoning Designation: Commercial Recreation (CR)

Summary of Request: The applicant proposes to construct 19 cabins that will serve as

nightly and weekly lodging, as well as an event space to be used for private weddings, reunions, and parties. On-site parking, public pedestrian access, landscaping, and enhancements to the riverbank will also be provided. Additionally, a plat vacation is proposed to provide a more cohesive property under one

ownership group.

Date: August 11, 2023



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I. PROJECT OVERVIEW

EXISTING CONDITIONS

The project site is located off SW Rock Creek Drive, north of the existing Rock Cove Assisted Living Community, and includes parcels 02070100130200 and 02070100130300. The site is approximately 6.40 acres in size and zoned Commercial Recreation (CR) on the Stevenson Zoning Map. The development area is currently undeveloped (no buildings on site) but retains improvements from prior industrial land uses that include concrete and gravel surfaces, gravel roads accessing various points of the site, a graveled boat launch, and riprap embankments that span a majority of the shoreline.



PROPOSAL

The project seeks to complement the existing tourism industry in Stevenson by proposing 19 cabins available for nightly and weekly rental. An event space will anchor the development and provide wide views of Rock Cove and the Columbia River Gorge. The development seeks to attract both local and regional visitors, with venue space available for weddings, company parties, family reunions, and corporate retreats.

In addition to the cabins and event space, the applicant proposes to restore water-side portions of the property for enhanced publicly accessible observation and enjoyment. This will equal to approximately 1. 12 acres of buffer improvement along the shoreline.



II. RESPONSE TO APPLICABLE DEVELOPMENT AND CODE STANDARDS

Note: Responses to all applicable development standards are included below. Sections that are not applicable or do not require a response may be omitted from the narrative text.

CHAPTER 16:02: SHORT PLATS AND SHORT SUBDIVISIONS

Section 16.02.270: Ancillary Proceedings – Plat Vacation

- A. Whenever any person is interested in the vacation of any short plat or any portion thereof, or any area designated or dedicated for public use, that person shall file an application for vacation with the council. The application shall set forth the reasons for vacation and shall contain signatures of the owner(s) of that portion of the short plat subject to vacation. If the short plat is subject to restrictive covenants which were filed at the time of the approval of the short plat, and the application for vacation would result in the violation of a covenant, the application shall contain an agreement signed by all parties subject to the covenants providing that the parties agree to terminate or alter the relevant covenants to accomplish the purpose of the vacation of the short plat or portion thereof.
- B. When the vacation application is specifically for a city road, the procedures for road vacation in RCW Chapter 35.79 and the city's road vacation procedures shall be utilized for the road vacation. When the application is for the vacation of the short plat together with the roads and/or streets, the procedure for vacation in this section shall be used, but vacations of roads may not be made that are prohibited under RCW 35.79.
- C. The council shall give notice as provided in Section 16.02.110(C) and shall conduct a public hearing on the application for vacation and may approve or deny the application for vacation of the short plat after determining the public use and interest to be served by the vacation of the short plat. If any portion of the land contained in the short plat was dedicated to the public for public use or benefit, such land, if not previously deeded to the city, shall be deeded to the city unless the council shall set forth findings that the public use would not be served in retaining title to such lands.

Response: The applicant is proposing a plat vacation on the project site to provide a more cohesive land use pattern. All of the land contained in the plat after the vacation will be under a single ownership group. All required procedures for a plat vacation, including a public hearing, will be complied with by the property owner.

CHAPTER 17.25: TRADE DISTRICTS

SECTION 17.25.040: USES

Table 17.25.040-1: Trade District Use Table						
Use	CR					
Overnight Lodging (Hotel)	Permitted					
Food Service	Permitted					
Arts, Entertainment, and Recreation Uses (Public Assembly)	Permitted					



Response: The applicant proposes 19 cabins that will be available as overnight lodging, as well as an event space with a kitchen that is available for private events. All three of these uses are permitted in the Commercial Recreation zoning district as shown in Table 17.25.040 above.

SECTION 17.25.050: TRADE DENSITY

A. Density and Lot Size. The maximum density and minimum lot dimensions for Trade Districts are contained in Table 17.25.050-1: Trade Density Standards.

Table 17.25.050-1: Trade Density Standards									
District	Use Minimum Lot Area Minimum Lot Minimum				Maximum Lot				
			Width	Depth	Coverage				
CR	All	10,000 square feet	ı	-	35%				
Proposed		262,334 square feet	~450 feet	~580 feet	10.6% (27,983 square				
					feet)				

Response: The project site is 6.4 acres in size, or approximately 262,334 square feet. The lot is irregularly shaped due to its location adjacent to Rock Cove, with a width of approximately 450 feet and a depth of approximately 580 feet. The applicant is proposing interior lot landscaping, including trees, shrubs, and ground covering, as well as enhancement of the 1.2 acres of the property's shoreline with additional native vegetation. In total, the lot coverage on the project site is approximately 10.6% or 27,983 square feet. Please see the attached landscape plan and site plan for details on lot dimension and coverage.

SECTION 17.25.060: TRADE DIMENSIONAL STANDARDS

- A. Compliance Required. All structures in Trade Districts must comply with:
 - 1. The applicable dimensional standards contained in Table 17.25.060-1: Trade Dimensional Standards.
 - 2. All other applicable standards and requirements contained in this title.

Table 17.2.	Table 17.25.060-1: Trade Dimensional Standards									
District	Minimum	Maximum	Maximum Minimum Setbacks Maximum Set							
	Height of	Height of	Front	Side	Side	Rear	Rear	Front	Side	
	Building	Building		(Interior)	(Street)				(Street)	
CR	N/A	35 feet	25 ft	0 ft	20 ft	0 ft	20 ft	N/A	N/A	
Proposed	32 feet		~60ft	~40 ft	N/A	~45 ft	~45ft	~60 ft	N/A	

Response: The site has been designed to comply with all setback requirements to the maximum extent possible, while also provided a sufficient buffer from the ordinary high water line and the existing fish and wildlife conservation area. All minimum setbacks of the CR zone are met, while some of the maximum setbacks are greater than generally allowed to accommodate the critical natural areas and Rock Cove that surround the site. Please see the attached site plan, Sheet C2.00, for details on setbacks and site layout.

SECTION 17.25.070: TRADE DISTRICT DESIGN

A. CR Design



1. Buildings shall be appropriately scaled and compatible with their locations and surrounding environment, including adjacent buildings, landscaping, water bodies and other natural features.

Response: The applicant is proposing two different styles of cabins ranging from 629 square feet to 853 square feet. Each residential building and the larger event space are scaled to be compatible with all other buildings on site in height and mass. Please see the attached site plan and building elevations for details.

2. Exterior building materials and finishes shall be compatible with the unique setting of the Columbia River Gorge. Preference should be given to nonglossy finishes and earthtone colors.

Response: The proposed building materials will be primarily wood, and any paint or color used will be selected to compliment the surrounding nature by being earth toned. Please see the attached elevations for details on proposed building materials.

- 3. Outdoor storage shall be visually screened by landscaping, fences, walls or enclosures.
- 4. Refuse containers shall be fully enclosed and covered. Enclosures shall be constructed of materials compatible with the main structure.

Response: All storage areas will be visually screened from view by either landscaping or a wall.

5. Screening and buffering shall be provided between dissimilar uses to minimize negative impacts, such as those from noise, traffic, lighting and glare.

Response: The entire site will be used as a nightly and weekly lodging and event space. There are no dissimilar uses on the project site that will require additional screening or buffering.

6. Screening and buffering shall be located along the perimeter of a lot or parcel.

Response: Landscaping is provided along the entire perimeter of the site, including along Rock Creek Drive. Please see the attached landscape plan for details.

7. The location and number of access points to the site, their relationship to existing streets and traffic, the interior circulation patterns, and the separation between pedestrians and vehicles shall be designed to maximize safety and convenience.

Response: The applicant is proposing one vehicular access point on the site from Rock Creek Drive, the only street that borders the project site. The on-site vehicular circulation has been designed to provide easy access to visitors and employees, while keeping cars separate from pedestrians by providing sidewalks connecting all buildings and amenities on site. Please see the attached site plan, Sheet C2.00, for details.

- 8. Pedestrian sidewalks, pathways and access ways shall be located and constructed to minimize conflicts with vehicular traffic and natural hazards.
- 9. Safety crossings and adequate sight lines shall be provided at pathway intersections with roads.

Response: All proposed pedestrian pathways on site are proposed to be a minimum of eight feet wide and separated from the on-site vehicular parking and circulation by a curb. Where the pedestrian pathways cross



vehicular drive aisles, a clearly marked crosswalk will be provided. Please see the attached site plan, Sheet C2.00, for details.

10. Roads, buildings and other structural improvements shall be located and designed to minimize grading and modification of existing landforms and natural characteristics.

Response: The site has been designed and structures laid out to take into account the natural topography and features of the site. The parking area and cabin layout has been designed to limit grading to the maximum extent possible, while limiting any impact to the water edge. Please see the attached site plan, Sheet C2.00, for details.

11. Developments shall not contribute to the instability of a parcel or to adjoining lands.

Response: None of the proposed improvement will cause instability to the project site or adjacent properties.

12. Surface drainage systems shall be designed so as not to adversely affect neighboring properties, roads or water bodies.

Response: Surface drainage has been designed to prevent any draining to the neighboring properties or adjacent roads and water bodies. The applicant is proposing multiple stormwater facilities on the project site, including catch basins, rock outflows, and storm cleanouts. Please see the attached stormwater plan, Sheet C4.00, for details on surface drainage systems.

13. Developments within the designated shoreline areas of the CR district shall provide ample public visual and physical access to the water.

Response: The project site is located within the shoreline area of the CR zoning district and the applicant has designed the site to include a path on the eastern perimeter of the property to allow for visual and physical access to the water. This will be accessible to visitors and customers of the cabins. Please see the attached site plan, Sheet C2.00, for details on the path and access to Rock Cove.

SECTION 17.25.100: TRADE DISTRICT LANDSCAPING

- A. CR Landscaping.
 - 1. Minimum landscaping shall include 100% of the area between the building line and the street right-of-way line excluding drives, parking areas and pathways.

Response: All areas of the site not proposed for cabins, parking and maneuvering areas, pedestrian pathways and trails, or the event space are proposed to be landscaped. Please see the attached preliminary landscape plan for details.

- 2. New trees, shrubs, groundcover and other materials shall be compatible with other nearby landscaping.
- 3. New plantings shall be of such size, condition and density that they are initially effective.



Response: The proposed groundcover, shrubs, and conifer and deciduous trees will be chosen to be compatible with the surrounding landscaping and vegetation and be survive successfully after being planted.

4. Wherever practical, natural vegetation and existing grade should be retained.

Response: To the maximum extent practical, the natural vegetation on site is being preserved. This includes all land on site between the ordinary high water mark buffer and the shoreline. Please see the attached preliminary landscape plan for details.

5. In areas where vegetation plays an important role in erosion control, aesthetic considerations or slope stabilization, any vegetation removed during construction, excavation or grading shall be promptly replaced.

Response: All native existing native vegetation is being preserved between the shoreline and the ordinary high water mark buffer. This will ensure erosion control of the site during construction of the proposed development.

6. Trees and shrubs which are intended to be retained on a site shall be protected during construction.

Response: All tree and vegetation protection measures will be utilized during site grading and construction for vegetation proposed to be retained on the development site.

SECTION 17.25.130: TRADE DISTRICTS PARKING AND LOADING

- A. CR Parking and Loading.
 - 1. Off-street parking shall be provided in accordance with the requirements of SMC 17.42 Parking and Loading Standards.

Response: All applicable standards of Chapter 17.42 are met with the proposed off-street parking lot. Please see the response in Chapter 17.42 of this narrative for details on compliance.

2. Parking areas, aisles, loading aprons and access ways shall be paved with an all-weather surface of a strength adequate for the traffic expected and shall be well drained.

Response: All proposed parking areas, drive aisles, loading areas and accessways are paved with an all-weather surface of a strength adequate for the expected traffic and needed drainage on site.

CHAPTER 17.42: PARKING AND LOADING STANDARDS

SECTION 17.42.040: SIZE AND ACCESS REQUIREMENTS

A. Each off-street parking space shall have a minimum width of 9 feet and a minimum length of 18 feet, except that each off-street parking space for compact vehicles shall have a minimum width of 8 feet and a minimum length of 16 feet.



Response: All standard parking spaces on the project site are proposed to be 20 feet long and nine feet wide. Please see the attached site plan, Sheet C2.00, for details on the proposed parking spaces.

B. Aisles shall have a minimum width of 20 feet.

Response: The drive aisle on the project site is 24 feet wide, complying with the minimum width of 20 feet. Please see the attached site plan, Sheet C2.00, for details.

C. Up to one-third of the required off-street parking spaces on a site may be sized and designated for compact vehicles.

Response: None of the off-street parking spaces on site are proposed to be compact.

D. Each parking space shall be of usable shape and condition.

Response: All proposed off-street parking spaces on site are of a usable shape and comply with the minimum dimensional standards of 17.42.040(A). Please see the attached site plan, Sheet C2.00, for details.

SECTION 17.42.060: JOINT USE OF PARKING PERCENTAGE OF AREA PERMITTED

The planning commission may authorize the joint use of parking facilities for the following uses or activities under conditions specified:

- A. Up to 50% of the parking facilities required by this chapter for a theater, bowling alley, restaurant, retail, service or other similar uses, may be supplied by the off-street parking provided by other types of uses or by a community parking lot.
- B. Reserved.
- C. Up to 100% of the parking facilities required by this chapter for a church, auditorium, stadium, or sport arena incidental to a public, private or parochial school may be supplied by the off-street parking facilities serving primarily uses or by a community parking lot.
- D. Up to 100% of the parking facilities required by this chapter for a hotel may be supplied by the offstreet parking provided by other types of uses or by a community parking lot.

Response: The primary use proposed on the project site is cabins, falling under the use of a hotel. However, the applicant is also proposing an event space, most similar to an auditorium use in the City of Stevenson Development Code. Auditoriums are permitted to provide up to 100% of the parking required by the offstreet parking facilities serving the primary use. Because the hotel is the primary use on site, the applicant is proposing a single off-street parking lot to serve both the cabins and the event space on site.



SECTION 17.42.080: OFF-STREET FACILITIES - LOCATION REQUIREMENTS

Off-street facilities shall be located as specified in this section. Where a distance is specified, such distance shall be the maximum walking distance measured from the nearest point of the parking facility to the nearest point of the building that such facility is required to serve:

- A. For a single-family dwelling: on the home lot with the building they are required to serve;
- B. For multiple dwellings: 150 feet;
- C. For retail, food service, and hotel uses: 1,000 feet;
- D. For all other uses: 300 feet.

Response: The proposed parking area is located within ten feet of the nearest cabin and event space, complying with the above distance standards. Please see the attached site plan, Sheet C2.00, for details.

SECTION 17.42.090: TABLE OF MINIMUM STANDARDS — OFF-STREET PARKING

A. Off-street parking shall be provided in accordance with Table 17.42.090-1: Off-Street Vehicle Parking Requirements.

Table 17.42.090-1: Off-Street Vehicle Parking Requirements							
Use	Unspecified or All Other Districts (including CR)						
Hotel	1 for each sleeping unit plus 1 space for each 2						
	employees on the evening shift						

Response: The proposed use will include nightly and weekly lodging for local and regional tourism, similar to a hotel use. Under Table 17.42.090-1, hotel uses require one parking space for each sleeping unit plus one space for each two employees on the evening shift. The applicant is proposing a total of 71 parking spaces which will sufficiently serve the 19 hotel cabins, employees, and any event hosted at the development site. Please refer to the attached site plan, Sheet C2.00, for details on proposed off-street parking.

SECTION 17.42.100: LOADING AND UNLOADING AREAS

A. Subject to subsection E of this section, whenever the normal operation of any development requires that goods, merchandise or equipment be routinely delivered to or shipped from that development, a sufficient off-street loading and unloading area must be provided in accordance with this section to accommodate the delivery or shipment operations in a safe and convenient manner.

Response: The proposed project will include cabins available for nightly and weekly rentals, as well as an associated event space to be used by individuals, families, or businesses staying at the site. A permanent loading space is not anticipated to be needed, as there will be no regular delivery of goods, merchandise, or equipment delivered to the development.



CHAPTER 18.13: CRITICAL AREAS AND NATURAL RESOURCE LANDS

SECTION 18.13.095: CRITICAL AREA – FISH AND WILDLIFE HABITAT CONSERVATION AREAS

C. FWHCA Reports.

- 1. Preliminary Assessments. In order to determine the extent of the appropriate buffers on a site when the nature of the fish and wildlife habitat conservation area is unclear, the applicant may submit a preliminary habitat assessment report as prepared by a qualified professional in accordance with SMC 18.13.050 Critical area reports—Requirements. This report shall suffice for the purpose of the development application if no habitat buffer impacts are proposed. In addition to the minimum requirements for critical area reports contained in SMC 18.13.050, a preliminary FWHCA report should also contain the following information:
 - a. Confirmation or correction of the classifications for the FWHCA and/or stream type as defined in this chapter;
 - b. Characterization of riparian (streamside) vegetation species, composition, and habitat function;
 - c. Description of the soil types adjacent to and underlying the stream, using the Soil Conservation Service soil classification system;
 - d. Identification of the qualities of the area that are essential to maintain feeding, breeding, and nesting, and an assessment of potential project impacts to the use of the site by the species;
 - e. A discussion of any federal, state, or local species/habitat management recommendations, including the WDFW habitat management recommendations that have been developed for the identified species or habitat;
 - f. Recent photographs of the property, including detailed photographs of the habitat resource in question;
 - g. An outline of standard buffer widths, available buffer reductions, or potential opportunities for enhancement/mitigation.

Response: Ecological Land Services, Inc. (ESL) prepared a Critical Areas Report and Conceptual Mitigation Plan for the preliminary review of this application that includes all of the above information. The City's environmental consultant, Olson Environmental LLC, provided review and comments on this report dated and provided findings that concur with the FWHCA buffer isolation. Please see the attached report for details.

- D. Habitat Buffer Widths.
 - 1. Base Buffer Widths. The following buffer widths have been established in accordance with the best available science. They are based on category of fish and wildlife habitat conservation area.
 - 2. Buffer Averaging. Buffer averaging to allow reasonable use of a parcel may be permitted when all of the following are met: [...]

Response: The project site area is designated as a fish and wildlife habitat conservation area (FWHCA) and has Type F waters is 100 feet and Type S waters is 150 feet. There are no proposed buffer reductions for this project and the applicant is proposing all development to occur outside of the Shoreline Management Plan Setback and approximately 0.19 acres of improvements within the FWHCA Buffer for Type S. However, the applicant is proposing 1.12 acres of buffer enhancements (a 1:5.9 impact to enhancement area ratio.



- 3. Functionally Isolated Buffers. Lawns, walkways, driveways, other mowed or paved areas, and areas which are functionally separated from a FWHCA and do not protect the FWHCA from adverse impacts due to pre-existing roads, structures, or vertical separation, shall be excluded from buffers otherwise required by this chapter. If existing developments cause the width of the remaining buffer to be less than 50% of the base buffer, both of the following conditions shall apply:
 - a. If the reduced buffer exists in a degraded condition, the reduced buffer shall be enhanced in accordance with 18.13.095.D.5. unless the area in question is utilized for activities consistent with water dependent uses.
 - b. The buffer cannot be further reduced through averaging or on-site mitigation.

Response: There are portions of the site that have historically been used for development and have existing gravel or concrete surfacing that are located within the buffer. The applicant is not proposing to further impede the fish and wildlife conservation area of Rock Cove but is proposing permanent development in the already impacted area. Please see the attached site plan, Sheet C2.00, for details.

- F. Habitat Mitigation.
 - 1. Compensatory Mitigation, Required. Compensatory mitigation for impacts to FWHCA's shall be used only for impacts that cannot be avoided or minimized and shall achieve equivalent or greater functions as those affected by the proposed project. Out-of-kind replacement of FWHCA type or functions may be considered if the applicant demonstrates it will best meet watershed goals formally identified by the city, such as replacement of historically diminished FWHCA types.

Response: The proposed FWHCA enhancement increases the existing buffer functions and values. The applicant is proposing approximately 1.12 acres of buffer enhancement by densely planting the remaining buffer area with native shrubs and removing non-native invasive Himalayan blackberry. This is a 1:5.9 ratio of impact to enhancement, significantly exceeding the 1:1 enhancement ratio required for on-site FWHCA mitigation. Please see the attached Critical Areas and FWHCA Report for details.

- 2. FWHCA Mitigation Plan. When a project involves FWHCA or FWHCA buffer impacts, enhancements, or reductions, a habitat mitigation plan by a qualified professional shall be required. At a minimum, the habitat mitigation plan must contain the following information:
 - a. Baseline Information. All the information required in the FWHCA Report prepared under SMC 18.13.095(C).
 - b. Site Plan. A copy of the site plan for the development proposal showing identified critical areas, buffers, and dimensions and limits of any areas to be cleared. This plan should include the proposed construction sequencing, grading and excavation details, erosion and sedimentation control features, and detailed site diagrams and any other drawings appropriate to show construction techniques or anticipated final outcome.
 - c. Project Impacts and Mitigation. A description of the mitigation sequence developed for the project according to SMC 18.13.055. This should involve a description of the existing and estimated future conditions of the enhancement area and/or compensatory mitigation site, including location and rational for selection. Include an assessment of all appropriate technical information necessary to assess the compensatory mitigation proposed.



- d. Goals and Objectives. The environmental goals and objectives of the mitigation, and the goals and objectives must be related to the functions and values of the impacted critical area.
- e. Monitoring and Maintenance Program. A proposed Monitoring Program compliant with SMC 18.13.059 Performance and monitoring standards.
- f. A bond estimate for the entire enhancement and/or compensatory mitigation project, including the following elements: site preparation, plant materials, construction materials, installation oversight, maintenance twice per year for up to 5 years, annual monitoring field work and reporting, and contingency actions for the monitoring period established under SMC 18.13.059 Performance and monitoring standards.
- g. Where proposed activities, uses, and alterations are located below the OHWM, identification of how the preservation and enhancement of anadromous fish habitat will be achieved including, but not limited to, the following:
 - The allowable work window as designated by the WDFW.
 - ii. Alternative alignments or locations for the activity that were determined infeasible.
 - iii. Stream width and flow rate, stability of the channel including erosion or aggradation potential, type of substratum, discussions of infiltration capacity and biofiltration before and after alteration, presence of hydrologically associated wetlands, analysis of fish and wildlife habitat, and any proposed floodplain limits.
 - iv. Methods to minimize the degradation of the downstream functions or values of the fish habitat or other critical areas.

Response: The attached Critical Areas and FWHCA Report prepared for the project site by Ecological Land Services includes all of the above requirements and information. Please see the attached report for details on compliance.

III. RESPONSE TO APPLICABLE STEVENSON SHORELINE MASTER PROGRAM AND APPROVAL CRITERIA

CHAPTER 3: SHORELINE ENVIRONMENT DESIGNATION PROVISIONS

SECTION 3.2: ENVIRONMENT DESIGNATIONS

3.2.5: ACTIVE WATERFRONT ENVIRONMENT

- 3. Management Policies:
 - a. Prefer uses that preserve the natural character of the area or promote preservation of open spaces and sensitive lands, either directly or over the long term. Allow uses that result in restoration of ecological functions if the use is otherwise compatible with the purpose of the environment and the setting.
 - b. Give priority to water-oriented uses, with first priority to water-dependent, then second priority to water-related and water-enjoyment uses. For shoreline areas adjacent to commercially navigable waters, give highest priority to water-dependent uses.
 - c. Prohibit new non-water-oriented uses, except:
 - i. As part of mixed use development;



- ii. In limited situations where they do not conflict with or limit opportunities for water oriented uses;
- iii. On sites where there is no direct access to the shoreline;
- iv. As part of a proposal that result in a disproportionately high amount of restoration of ecological functions.

Response: WAC 173-26-020 defines a water-enjoyment use as a use that provides for recreational use or aesthetic enjoyment of the shoreline for a substantial number of people as a general characteristic of the use and which through location, design, and operation ensures the public's ability to enjoy the physical and aesthetic qualities of the shoreline. A hotel with cabins and a shared event space located on the project site will allow for groups and individuals to stay and have events with a view of Rock Cove and the Columbia River. The proposed development is located on a part of the shoreline with very limited access due to the steep slopes, and is not an ideal location for a water-dependent use.

d. Assure no net loss of shoreline ecological functions as a result of new development through shoreline policies and regulations. Where applicable, new development shall include environmental cleanup and restoration of the shoreline to comply in accordance with any relevant state and federal law.

Response: The applicant is proposing approximately 0.19 acres of buffer impacts that include permanent development. As mitigation, the applicant is proposing approximately 1.12 acres of buffer enhancement by densely planting the remaining buffer area with native shrubs and removing non-native invasive blackberry. The proposed mitigation is in-kind buffer enhancement at a 5.9:1 ratio, significantly exceeded the 1:1 enhancement ratio required for the site and assuring no net loss in shoreline ecological functions. Please see the attached Critical Areas and FWHCA Report for details.

e. Require public visual and physical access and implement public recreation objectives whenever feasible and where significant ecological impacts can be mitigated.

Response: As part of the site design, the applicant is proposing pedestrian pathways that connect to the right-of-way, cabins and event center, and around the perimeter of the site adjacent to the shoreline. Please see the attached site plan for details on public access.

CHAPTER 4: GENERAL PROVISIONS FOR ALL SHORELINE ACTIVITIES

Section 4.3: Environmental Protection & No Net Loss

4.3.2: REGULATIONS

- 1. Management Sequence. In order to ensure that review activities contribute to meeting the no net loss provisions by avoiding, minimizing, and mitigating for adverse impacts to ecological functions or ecosystem-wide processes, applicants shall describe how the proposal will follow the sequence of mitigation as defined below:
 - a. Avoid the impact altogether by not taking a certain action or parts of an action;



- b. Minimize the impacts by limiting the degree or magnitude of the action and its implementation by using appropriate technology or by taking affirmative steps (e.g., project redesign, relocation, timing to avoid or reduce impacts, etc.);
- c. Rectify the impact by repairing, rehabilitating, or restoring the affected environment to the conditions existing at the time of the initiation of the project or activity;
- d. Reduce or eliminate the impact over time by preservation and maintenance operations during the life of the action;
- e. Compensate for the impact by replacing, enhancing, or providing substitute resources or environments; and
- f. Monitor the impact and the compensation projects and take remedial or corrective measures when necessary.

Response: The applicant will be impacting parts of the buffer on site, but will be mitigating the impact at a near 6:1 ratio. The site has been designed to avoid all buffer impact areas to the maximum extent possible. However, with the current design there is approximately 0.19 acres of buffer impacts that include permanent development. As mitigation, the applicant is proposing approximately 1.12 acres of buffer enhancement by densely planting the remaining buffer area with native shrubs and removing non-native invasive blackberry. The proposed mitigation is in-kind buffer enhancement at a 5.9:1 ratio, significantly exceeded the 1:1 enhancement ratio required for the site and assuring no net loss in shoreline ecological functions. Please see the attached Critical Areas and FWHCA Report for details.

2. The mitigation sequence is listed in the order of priority. Applicants shall consider and apply lower priority measures only where higher priority measures are determined to be infeasible or inapplicable.

Response: The applicant avoided impacts to the maximum extent possible (a) and where impacts could not be avoided the were minimized with site design (b). All buffer enhancements include the planting of native species and restoring the shoreline by removing invasive species (c). Please see the attached Critical Areas and FWHCA Report for details.

3. SEPA Compliance. To the extent SEPA applies to a proposal, the analysis of environmental impacts and mitigation related to the proposal shall be conducted consistent with WAC 197- 11—SEPA Rules and SMC 18.04—Environmental Policy.

Response: The previously submitted SEPA covers the proposed development. No new SEPA is required.

- 4. Cumulative Impacts. As part of the assessment of environmental impacts subject to this SMP, new uses, developments, and modifications shall evaluate and consider cumulative impacts of reasonably foreseeable future development on shoreline ecological functions. Evaluation of cumulative impacts shall consider:
 - a. Current circumstances affecting the shorelines and relevant natural processes;
 - b. Reasonably foreseeable future development and use of the shoreline; and
 - c. Beneficial effects of any established regulatory programs under other local, state, and federal laws.



Response: The applicant is proposing approximately 0.19 acres of impact to the shoreline on the project site. However, a total of 1.12 acres of enhancements are proposed within the shoreline area on site, creating a cumulative positive impact to the site. The enhancements proposed will benefit the site and surrounding ecology and include the planting of native species. Please see the attached Critical Areas Report and site plan for details.

- 5. Mitigating for Impacts. When impacts related to a proposal require mitigation, the following shall apply:
 - a. The proposal shall achieve no net loss of ecological functions.
 - b. The City shall not require mitigation in excess of that necessary to assure the proposal 1) results in no net loss of ecological function and 2) does not have a significant adverse impact on other shoreline functions fostered by this SMP.
 - c. Compensatory mitigation shall give preference to measures that replace the impacted function directly and in the immediate vicinity of the impact. However, alternative compensatory mitigation located elsewhere in the same reach or watershed that addresses limiting factors or identified critical needs for shoreline resource conservation may be authorized, including appropriate actions identified in the Restoration Plan.
 - d. Unless waived by the City, authorization of compensatory mitigation shall require appropriate safeguards, terms or conditions (e.g. performance bonding, monitoring, conservation covenants) as approved by the City Attorney and necessary to ensure no net loss of ecological functions.

Response: There is approximately 0.19 acres of impacts to the buffer and a total of 1.12 acres of buffer enhancements proposed on site. This is a 1:5.9 impact to enhancement area ratio and does not result in a net loss of ecological functions on site. Please see the attached Critical Areas Report and site plan for details.

- 6. Environmental protection and no net loss shall be achieved by complying with the combination of use regulations, shoreline setbacks, critical area buffers, and vegetation removal restrictions:
 - a. Shoreline Allowances & Setbacks Table 5.1 establishes a list of permitted, conditional, and prohibited uses in each shoreline environment designation (SED). This table also establishes the minimum shoreline setback applicable to each use, activity, or development within each SED where development cannot occur; and

Response: The applicant proposes 19 cabins that will be available as overnight lodging, as well as an event space with a kitchen that is available for private events. This is considered a commercial use that is water-enjoyment and is permitted in the Active Waterfront as shown in Table 5.1. The minimum shoreline setback for this use and zone is 33 feet. The project has been designed to meet this setback, as shown on the attached site plan.

 b. Critical Areas Buffers – Section 4.4 Critical Area provisions, including separately incorporated SMC 18.13 provisions that establish Wetland and Riparian buffer standards as additional areas where mitigation sequencing must be applied and unavoidable impacts must be mitigated; and

Response: The project site is adjacent to a Fish & Wildlife Habitat Conservation Area and has provided a preliminary FWHCA assessment, determining necessary buffers and confirming there will be no net loss of ecological function with the proposed development. Please see the attached assessment for details.



c. Modifications & Vegetation – Shoreline modification standards, vegetation standards, and prescriptive mitigation measures of Chapter 6 apply to all vegetation impacts occurring within shoreline jurisdiction.

Response: All applicable policies of Chapter 6 are met with the proposed development and vegetation mitigation. Please see the responses to the regulations of Chapter 6 for details.

SECTION 4.6: PUBLIC ACCESS

4.6.2: POLICIES

1. Continuous public pedestrian access should be provided along the City's shorelines, especially the Columbia River, Rock Cove, and Lower Rock Creek.

Response: The project site is located on the western edge of Rock Cove and is providing continuous public pedestrian access along the perimeter of the project site and connecting to the public right-of-way on Rock Creek Drive and the existing easement on the southern property. All pathways are accessible to the public and will provide access to adjoining properties and rights-of-way. Please see the attached site plan for details on the provided connections.

2. The system of public physical and visual access to Stevenson's shorelines should be maintained, enhanced, and protected over time on both private and public lands.

Response: The property owner will maintain all proposed public and private pedestrian pathways on the project site, including surfacing and landscaping.

3. Public access and recreational facilities should be located in a manner that will preserve the natural characteristics and functions of the shoreline.

Response: The proposed public access pathway is located along the perimeter of the project site and provides access to the shoreline of Rock Cove without negatively impacting the shoreline or natural vegetation since the pathway is located primarily outside of the buffer area. Please see the attached site plan for details.

4. Private property rights, public safety, and navigational rights should be considered when providing public access opportunities.

Response: The applicant has designed the site to be developed as a private business while also providing public access opportunities to the right-of-way, adjacent properties, and the shoreline on Rock Cove. Please see the attached site plan for details on site layout and public access.

5. New development should identify and preserve key shoreline views and avoid obstructing such views from public areas.



Response: The proposed development has been thoughtfully designed to preserve the shoreline views along Rock Cove. The applicant is providing public access via a pedestrian pathway that will circumnavigate the site adjacent to the shoreline and connect to the adjacent properties and rights-of-way.

6. The City's should develop a comprehensive and integrated public access and trail plan (consistent with WAC 173-26-221(4)) that identifies specific public access needs and opportunities to replace these site-by-site requirements. Such plan should identify a preference for pervious over impervious surfaces, where feasible.

Response: The applicant is proposing connections to existing public access easements to the south property and along Rock Cove. Please see the attached site plan for all pedestrian pathway and public access proposed with this development.

4.6.3: REGULATIONS

- Consistent with legal/constitutional limitations, provisions for adequate public access shall be incorporated into all proposals for Shoreline Permits that have one or more of the following characteristics:
 - a. The proposed development or use will create a demand for, or increase demand for public access;
 - The proposed use is water-enjoyment, water-related, or non water-dependent, except for individual single-family residences not part of a development planned for 5 or more parcels;
 - c. The proposed use involves the subdivision of land into 5 or more parcels;
 - d. The proposed development or use will interfere with existing access by blocking access or discouraging use of existing access;
 - e. The proposed development or use will interfere with public use of waters of the state;
 - f. The proposed development or use will involve public funding or occur on public lands, provided that such access would not result in a net loss of ecological function. Public funding includes any funds from federal, state, municipal or local taxation districts.

Response: The project site is a water-enjoyment use and is providing public access to and through the site to the shoreline. Please see the attached site plan for the location of this access.

- 2. Additional public access will not be required where suitable public access is already provided by an existing public facility on or adjacent to the site and the Planning Commission makes a finding that the proposed development would not negatively impact existing visual or physical public access nor create a demand for shoreline public access that could not be accommodated by the existing public access system and existing public recreational facilities in the immediate vicinity.
- 3. Public access will not be required where the applicant demonstrates it is infeasible due to at least one of the following: [...]
- 4. To meet any of the conditions under Regulation 3 above, the applicant must first demonstrate to the satisfaction of the Planning Commission that all reasonable alternatives have been exhausted including, but not limited to, the following: [...]



- 5. For projects that meet the criteria of Regulation 3 above, the City may consider off-site public access or, if approved by the Planning Commission and agreed to by the applicant, the applicant may contribute a proportional fee to the local public access fund (payment in lieu).
- 6. If the City determines that public access is required pursuant to Regulation 1 above, the City shall impose permit conditions requiring the provision of public access that is roughly proportional to the impacts caused by the proposed development or use. The City shall demonstrate in its permit decision document that any such public access has a nexus with the impacts of the proposed development and is consistent with the rough proportionality standard.

Response: The applicant is providing public access on the project site. Therefore, the above standards do not apply.

- 7. When required, public access shall:
 - a. Consist of a dedication of land or a physical improvement in the form of a walkway, trail, bikeway, corridor, viewpoint, park, deck, observation tower, pier, boat launch, dock or pier area, or other area serving as a means of view and/or physical approach to public waters and may include interpretive centers and displays, view easements, and/or decreased building bulk through height, setback, or façade limitations;

Response: To provide public access on the project site, the applicant is providing a pedestrian walkway that circumnavigates the site along the Rock Cove shoreline and provides access to viewpoints of the cove and Columbia River. The shoreline is relatively steep on the site, so it is not feasible to provide direct access to the water. In addition to the on-site pedestrian pathways, the applicant is providing connections via pathways to the right-of-way and the adjacent property to the south where an additional easement exists. Please see the attached site plan for details on the proposed public access.

b. Include features for protecting adjacent properties from trespass and other possible adverse impacts;

Response: In addition to the pedestrian pathways, the applicant is providing extensive landscaping to buffer the site from the adjacent properties. These features will ensure public access is limited to the pedestrian pathways and not result in any trespassing or adverse impacts.

c. Be fully developed and available for public use at the time of occupancy of the proposed use or activity;

Response: The pedestrian pathways will be fully developed with the site and be available for public use at the time of occupancy of the cabins and event space.

d. Result in no net loss of shoreline ecological functions.

Response: The proposed public access pathways do not result in any net loss or negative impact to the shoreline ecological functions. Please see the attached report for details.

8. When required, physical public access shall be constructed to meet the following requirements for location, design, operation and maintenance:



a. Public access sites shall be connected directly to the nearest public street or non-motorized trail through a parcel boundary, tract, or easement, wherever feasible;

Response: The proposed pedestrian pathways is connected directly to the right-of-way on Rock Creek Drive. Please see the attached site plan for details on public access to the site.

b. Signs indicating the public's right of access to shoreline areas shall be installed and maintained in conspicuous locations.

Response: The property owner will ensure the public's right of access to the shoreline area are installed with the development of the site.

c. Public access easements and permit conditions shall be recorded on the deed of title and/or on the face of a plat or short plat as a condition running in perpetuity with the land, provided, that the Planning Commission may authorize a conveyance that that runs contemporaneous with the authorized land use for any form of public access other than parallel pedestrian access. Said recording with the County Auditor's Office shall occur at the time of permit approval.

Response: All required easements and permit conditions will be recorded on the deed of title or the final plat.

d. Maintenance of the public access facility shall be the responsibility of the owner unless otherwise accepted by a public or nonprofit agency through a formal agreement approved by the City and recorded with the County Auditor's Office.

Response: The property owner will maintain and be responsible for the public access pathway.

e. Public access sites shall be made barrier-free for the physically disabled where feasible, and in accordance with the ADA.

Response: The applicant is not proposing any barriers, including gates or fences, along the public access pedestrian pathway on the project site.

f. Any trail constructed shall meet the conditions described for shoreline areas in any trail or parks plan officially adopted by the City Council.

Response: All proposed pathways on the project site have been designed to meet all applicable standards of the City code. Please see the attached site plan for details.



CHAPTER 5: SHORELINE USE REGULATIONS

SECTION 5.3: SHORELINE USE TABLE

TABLE 5.1 – SHORELINE USE & SETBACK STANDARDS										
	Shoreline Environment Designation									
	AQU	AQUATIC NATURAL SHORELINE URBAN ACTIVI								TIVE
					RESIDENTIAL		CONSERVANCY		WATERFRONT	
	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)
Р	= Permitte	d, C=Cond	litional Use	X= Not P€	ermitted, n	/a= Not Ap	oplicable			
Agriculture & Mining										
Agriculture	X	n/a	X	n/a	X	n/a	X	n/a	X	n/a
Mining	X	n/a	X	n/a	X	n/a	X	n/a	X	n/a
Aquaculture										
Water-Oriented	C	n/a	x	n/a	X	n/a	С	0	С	0
Non-Water Oriented	X	11/4	^	11/4	^	11/4	X	n/a	С	150
Boating Facilities & Overwater S	Structures									
Non-motorized Boat Launch			С		Р		P		Р	
Motorized Boat Launch		_	X		C	C C	С		Р	
Mooring Buoy		neu	С		C		P		Р	
Float	See Adjacent	Ono.	X		C		С		Р	
Private Leisure Deck	١dja	invii	X	n/a	X	n/a	X	n/a	X	n/a
Public Leisure Pier	ee	Da B	X		С		P		P	
Single-User Residential Dock	S	Upland Environment	X		С		С		Р	
Joint-Use Moorage		_	X		Р		P		Р	
Marina			X		X		С		Р	
Commercial & Industrial										
Water-Dependent	Р				X ¹	0	Р	0	Р	0
Water-Related, Water Enjoyment	С	n/a	X	n/a	X ¹	75	Р	50	Р	33
Nonwater-Oriented	Х				Х	_	C ²	150	C ²	100

Response: The proposed cabins and event space are considered a water-enjoyment use. This is a permitted use in the Active Waterfront designation as shown in Table 5.1 above and requires a 33 foot setback. All proposed development on the project site is located outside of the 33 foot setback. Please see the attached site plan for details on setback location and proposed development.

SECTION 5.4: SPECIFIC SHORELINE USE POLICIES & PROVISIONS

5.4.4: COMMERCIAL & INDUSTRIAL

- 3. Policies:
 - a. Give first preference to water-dependent commercial and industrial uses over non-water dependent commercial and industrial uses; and second, to water-related commercial and



- industrial uses over non- water-oriented commercial industrial uses. Existing non-water oriented commercial and industrial uses should phase out over time.
- b. Prohibit new non-water-oriented industrial development on shorelines, unless the circumstances in WAC 173-26-241(3)(f) are found to exist.

Response: The applicant is proposing a water-enjoyment use that is permitted in the underlying zone. There are no non-water-oriented industrial development uses proposed with this application.

c. Ensure shoreline commercial development provides public access to the shoreline where opportunities exist, provided that such access would not pose a health or safety hazard.

Response: The proposed commercial development includes cabins and an event space. The applicant has incorporated pedestrian pathways and connections into the site plan to ensure public access to the shoreline. Please see the attached site plan for details on the access provided.

d. Encourage industrial development to incorporate public access as mitigation for impacts to shoreline resources and values unless public access cannot be provided in a manner that does not result in significant interference with operations or hazards to life or property.

Response: There is no industrial development proposed on the project site. Therefore, the above standard does not apply to this application.

e. Limit overwater commercial development to that which is water-dependent, or if not water dependent, that which is accessory and subordinate as necessary to support a water dependent use.

Response: There is no overwater development proposed on the project site. Therefore, the above standard does not apply to this application.

f. Locate and design industrial development in shoreline areas to avoid significant adverse impacts to other shoreline uses, resources, and values, including shoreline geomorphic processes, water quality, fish and wildlife habitat, and the aquatic food web. However, some industrial facilities are intensive and have the potential to negatively impact the shoreline environment. When impacts cannot be avoided, they should be mitigated to assure no net loss of the ecological functions necessary to sustain shoreline resources.

Response: There is no industrial development proposed on the project site. Therefore, the above standard does not apply to this application.

g. Encourage restoration of impaired shoreline ecological functions and processes as part of new or expanded commercial development, especially for non-water-oriented uses.

Response: As part of the proposed site development, the applicant is proposing to remove invasive species from the shoreline and plant native vegetation along the perimeter of the development. Please see the attached landscape plan for details on proposed restoration and plantings.



h. Give priority to industrial facilities proposed in areas of the shoreline already characterized by industrial development over such facilities proposed in shoreline areas not currently developed for industrial or port uses.

Response: There is no industrial development proposed on the project site. Therefore, the above standard does not apply to this application.

i. Locate industrial development where restoration of impaired shoreline ecological functions and processes and environmental cleanup can be included in the design of the project.

Response: There is no industrial development proposed on the project site. Therefore, the above standard does not apply to this application.

4. Regulations:

a. Water-dependent commercial and industrial uses shall be given preference over water related and water-enjoyment commercial and industrial uses. Second preference shall be given to water-related and water-enjoyment commercial and industrial uses over non-water oriented commercial and industrial uses.

Response: WAC 173-26-020 defines a water-enjoyment use as a use that provides for recreational use or aesthetic enjoyment of the shoreline for a substantial number of people as a general characteristic of the use and which through location, design, and operation ensures the public's ability to enjoy the physical and aesthetic qualities of the shoreline. A hotel with cabins and a shared event space located on the project site will allow for groups and individuals to stay and have events with a view of Rock Cove and the Columbia River. The proposed development is located on a part of the shoreline with very limited access due to the steep slopes, and is not an ideal location for a water-dependent use.

b. Prior to approval of water-dependent uses, the City shall review a proposal for design, layout and operation of the use and shall make specific findings that the use qualifies as a water dependent use.

Response: The proposed use is not a water-dependent use. Therefore, the above standard does not apply to this application.

c. When allowed, industrial development shall be located, designed and constructed in a manner that assures no net loss of shoreline ecological functions.

Response: The proposed use is not industrial development. Therefore, the above standard does not apply to this application. However, there is no net loss of shoreline ecological functions, as demonstrated in the attached critical area report.

d. Commercial development that is not water-dependent shall not be allowed over water except where it is located within the same existing building and is necessary to support a water-dependent use.



Response: The proposed development is a water-enjoyment use and is not proposed to be constructed over water.

e. Overwater and in-water construction of non-water-oriented industrial uses is prohibited. This provision is not intended to preclude the development of docks, piers, or boating facilities, or water-related uses that must be located in or over water (e.g., security worker booths, etc. that are necessary for the operation of the water-dependent or water-related use).

Response: The proposed development does not include overwater or in-water construction. Therefore, the above standard does not apply to this application.

f. Only those portions of water-oriented industrial uses that require over or in-water facilities shall be permitted to locate waterward of the OHWM, provided they are located on piling or other open-work structures, and they are limited to the minimum size necessary to support the structure's intended use.

Response: The proposed development does not include water-oriented industrial uses or require over or inwater facilities. Therefore, the above standard does not apply to this application.

g. Water-related and water-enjoyment uses shall avoid impacts to existing navigation, recreation, and public access.

Response: The proposed development is a water-enjoyment use and has no anticipated impact to the navigation, recreation, or public access existing on site. The project site already has limited existing access and navigation since it is located on a part of the shoreline that has steep slopes. Please see the attached grading plan and site plan for details.

h. Non-water-oriented commercial and industrial development shall not be allowed unless: [...]

Response: The proposed hotel with cabins and a shared event space located is a water-enjoyment use. Therefore, standard (h) does not apply to this project and the remaining code language has been omitted.

 New commercial and industrial developments shall provide public access to the shorelines, subject to SMP Section 4.6.

Response: The applicant is incorporating public access via pedestrian pathways that connect the Rock Cove right-of-way, shoreline, site features, and adjacent properties to provide public access on and through the project site. Please see the attached site plan for details on public access.

5.4.8: LAND DIVISIONS

- 3. Policies:
 - a. Land divisions should not result in a net loss of ecological functions.
 - b. Land division should not complicate efforts to maintain or restore shoreline ecological functions.



Response: The applicant is proposing approximately 0.19 acres of buffer impacts that include permanent development. As mitigation, the applicant is proposing approximately 1.12 acres of buffer enhancement by densely planting the remaining buffer area with native shrubs and removing non-native invasive blackberry. The proposed mitigation is in-kind buffer enhancement at a 5.9:1 ratio, significantly exceeded the 1:1 enhancement ratio required for the site and assuring no net loss in shoreline ecological functions. Please see the attached Critical Areas and FWHCA Report for details.

c. Land divisions involving the subdivisions of land into more than 4 parcels should provide community and/or public access in conformance with SMP Section 4.6.

Response: The applicant is proposing a plat vacation, not a land division. The plat vacation will provide a more logical land use pattern by consolidating ownership of the property. Public access is also being provide around the site.

4. Regulations

a. Plats and subdivisions shall be designed, configured, and developed in a manner that assures no net loss of ecological functions results from the plat or subdivision at full buildout of all lots.

Response: As mentioned, no net loss of ecological function will occur due to the plat vacation and proposed development. Please see the attached Critical Areas and FWHCA Report for details.

- b. The layout of lots within 1) new plats and subdivisions, 2) plat amendments, or 3) boundary line adjustments shall:
 - i. Prevent the need for new shoreline stabilization or flood hazard reduction measures that would cause significant impacts to other properties or public improvements or a net loss of shoreline ecological functions.
 - ii. Not result in lots containing inadequate buildable space due to critical areas and/or their buffers.

Response: The proposed plat vacation will provide a more logical land use pattern by consolidating ownership of the property and will not require additional or new shoreline stabilization or cause inadequate buildable space. The proposed development will comply with all dimensional standards and includes mitigation and buffer enhancement on site.

c. To ensure the success of restoration and long-term maintenance, the City may require that critical areas and/or aquatic lands be placed in a separate tract which may be held by an appropriate natural land resource manager (e.g., homeowner's association, land trust, natural resource agency, etc.).

Response: Acknowledged by the applicant.

5.4.9: RECREATIONAL

2. Regulations:



- a. Water-oriented recreational development shall be given priority and shall be primarily related to access, enjoyment, and use of the water and shorelines.
- b. Non-water-oriented recreational developments may be permitted only where it can be demonstrated that: [...]

Response: WAC 173-26-020 defines a water-enjoyment use as a use that provides for recreational use or aesthetic enjoyment of the shoreline for a substantial number of people as a general characteristic of the use and which through location, design, and operation ensures the public's ability to enjoy the physical and aesthetic qualities of the shoreline. A hotel with cabins and a shared event space located on the project site will allow for groups and individuals to stay and have events with a view of Rock Cove and the Columbia River. The proposed development is located on a part of the shoreline with very limited access due to the steep slopes, and is not an ideal location for a water-dependent use.

c. Non-water-oriented accessory uses (e.g., offices and parking areas that are part of recreational facilities) should be located landward of water-oriented facilities.

Response: The proposed parking area serving the cabins and event center are located on the western portion of the site to provide maximum distance between the shoreline and the accessory uses. Please see the attached site plan for details on layout.

5.4.11: Transportation & Parking Facilities

- 3. Policies.
 - a. New non-water-oriented transportation facilities should be located outside shoreline jurisdiction unless there is no reasonably feasible alternative alignment or location as determined by an alternatives analysis.

Response: The proposed use is a water-enjoyment use as described in this narrative. All parking proposed on site is in association with the proposed hotel use and there are no new non water-oriented transportation facilities located in the shoreline jurisdiction.

b. When it is necessary to locate transportation facilities in shoreline areas, they should be located where routes will have the least impact to shoreline ecological functions, will not result in a net loss of shoreline ecological functions, and will not adversely impact existing or planned water-dependent uses. Where feasible, a perpendicular alignment to shoreline should be preferred for transportation facilities over a parallel alignment which uses more shoreline area.

Response: The proposed parking lot associated with the proposed cabins and event space are located as far away from the shoreline as possible. The parking lot will not adversely impact the shoreline's ecological function and there is no net loss of the shoreline with the proposed site design. Please see the attached site plan for details.

c. Given that the City's Columbia River Shoreline is bisected by the BNSF railroad and the SR 14, the City should explore opportunities for pedestrian over- and underpasses linking upland areas with the waterfront.



Response: The project site does not border the BNSF railroad or SR 14. Therefore, the above standard does not apply.

- d. Pursuant to RCW 47.01.485, the City should review and act on WSDOT proposals within 90 days.
- e. Public visual and physical access areas should be encouraged as part of new transportation facilities (e.g., viewpoints, rest areas, picnic facilities, trail/bike systems adjacent to roads or railroads, etc.) where feasible and safe to do so. For bridges, public pedestrian access should be considered 1) on the bridge over the waterbody and 2) under or over the bridge parallel to the waterbody.
- f. The City should consider adopting special standards to ensure public and private roads within shoreline jurisdiction do not result in net loss of shoreline ecological functions.

Response: Acknowledged.

g. Parking is not a preferred shoreline use and should be allowed only to support a use authorized under the SMP.

Response: The parking lot located on the project site is provided to support the primary use of the property (cabins and event space) which are permitted under the SMP. No parking is proposed as a primary use with this application.

- h. Parking facilities should be located outside of shoreline jurisdiction or as far landward from the OHWM as feasible. Parking facilities serving individual buildings on the shoreline should be located landward, adjacent, beneath, or within the principal building being served. When located within shoreline jurisdiction, the location and design of parking facilities should:
 - i. Minimize visual and environmental impacts to adjacent shoreline and critical areas including provision of adequate stormwater runoff and treatment facilities. Parking areas should be adequately fenced and/or screened along the waterward edges of parking facilities and along the sides of such facilities when they abut differing land uses; and
 - ii. Provide for pedestrian access through the facility to the shoreline.

Response: The parking lot proposed on site to serve the primary use of the property is located as far landward from the OHWM as feasible near Rock Creek Drive. The proposed event space and cabins are primarily located between the parking lot and the shoreline. The applicant is proposing landscaping around the parking lot to minimize the visual effect of the lot and ensure adequate stormwater runoff and treatment. Pedestrian pathways are provided through the parking lot, connecting the public right-of-way on Rock Creek Drive to the Rock Cove shoreline. Please see the attached site plan for details.

- 4. Regulations.
 - a. Applications for redevelopment of transportation facilities in shoreline jurisdiction shall include:
 - i. Analysis of alternative alignments or routes, including, where feasible, alignments or routes outside of shoreline jurisdiction;



- ii. Description of construction, including location, construction type, and materials;and, if needed,
- iii. Description of mitigation and restoration measures.

Response: The applicant is not proposing to redevelop any transportation facilities with this application.

- b. Proposed transportation projects shall plan, design, and locate where routes:
 - i. Will have the least possible adverse effect on unique or fragile shoreline features,
 - ii. Will not result in a net loss of shoreline ecological functions, and
 - iii. Will not adversely impact existing or planned water-dependent uses.
- c. Alternative designs for transportation facilities that have less impact on shoreline resources (i.e., narrower rights-of-way, realignment) shall be considered in compliance with the SMC.

Response: The only transportation facility proposed with this application is an on-site parking lot to serve the primary use of the development. There will be no net loss of shoreline or adverse effect to the shoreline features with this application. Please see the attached environmental report and mitigation plan for details.

d. Roads and railroads of all types shall cross shoreline jurisdiction by the most direct route feasible, unless such a route would result in greater impacts on wetlands and fish and wildlife habitat conservation areas, or channel migration than a less direct route.

Response: There are no proposed roads or railroads with this application.

e. Wherever feasible and in compliance with the SMC, transportation facilities, including local access roads and surface parking facilities, shall be shared across shoreline uses to reduce the need for redundant facilities.

Response: The surface parking lot proposed on the project site with this application will serve the proposed development (cabins and event space) and provide sufficient parking for visitors and employees. Due to the unique shape of the project site, and amount of shoreline directly adjacent, it is not feasible to provide shared facilities on site.

f. New, replacement and enlarged transportation facilities shall provide public access pursuant to SMP Section 4.6.

Response: The applicant is not proposing a new or replacement transportation facility.

g. The City shall seek opportunities to obtain public easements and construct pedestrian connections over or under the railroad and state highway. The City shall place the pedestrian connection in its capital improvement plan and may require it as a condition of approval for Shoreline Permits, including permits involving new or replacement bridges and other transportation facilities.

Response: The applicant is providing public access to align with the City's desire to obtain opportunities for pedestrian connections. Please see the attached site plan for details.



h. Primary parking facilities (pay parking lots, park-and-rides) are not allowed within shoreline jurisdiction. Accessory parking (including parking for vista purposes) and loading facilities necessary to support an authorized shoreline use are permitted.

Response: The proposed parking lot is not a primary use on the project site, and will be used to serve the primary use (hotel and event space).

- i. All of the following conditions shall be met when an accessory parking facility is proposed in the shoreline jurisdiction:
 - The facilities shall be located landward, adjacent to, beneath or within the building being served.

Response: The proposed parking lot is located landward on the project site and the proposed event space and cabins are located between the shoreline and the parking area.

ii. Upland parking facilities shall provide safe and convenient pedestrian circulation from the parking area to the shoreline.

Response: The applicant is providing pedestrian pathways connecting the public right-of-way, parking area, and shoreline in a convenient and direct manner. Please see the attached site plan for the provided circulation on site.

iii. Loading spaces for development in the shoreline jurisdiction shall be located on the landward or side wall of non-water-dependent uses or activities.

Response: There are no proposed loading spaces with this application.

iv. All facilities shall provide parking suitable to the expected usage of the facility, with preference given to pavement or other dust-free all-weather surfaces.

Response: The parking lot is constructed of AC pavement, a suitable material for the use and location of the site.

v. All facilities shall be screened from adjacent, dissimilar uses through the use of perimeter landscaping, fencing, or some other approved material.

Response: The applicant is providing landscaping along the perimeter of the site and parking lot to screen from adjacent uses. Please see the attached landscaping plan for details.

CHAPTER 6: SHORELINE MODIFICATION PROVISIONS

SECTION 6.4: SPECIFIC SHORELINE MODIFICATION PROVISIONS

6.4.1: VEGETATION REMOVAL

- 3. Regulations
 - a. Vegetation removal shall be limited to the minimum necessary to accommodate approved shoreline development that is consistent with all other provisions of this SMP. This includes



- the design, location, and operation of the structure or development, including septic drain fields, which shall minimize vegetation removal and meet all applicable requirements.
- b. If removal of shoreline vegetation is unavoidable, vegetation removal shall be mitigated in accordance with the requirements in SMP Table 6.2 Mitigation for Vegetation Removal within Shoreline Jurisdiction. Exceptions:
 - i. The removal of native vegetation within established gardens, landscaping that serve a horticultural purpose shall not require mitigation under SMP Table 6.2.
 - ii. Mitigation plans prepared by a qualified professional may establish mitigation ratios that deviate from SMP Table 6.2.

Response: The proposed mitigation plan has been prepared by a qualified professional landscape architect from Cascara Land Design. The mitigation ratios required under Table 6.2 have been met with the proposed plan. Please see the mitigation plan and landscape plan for details on compliance.

c. No tree containing an active nest of an eagle, osprey, or other protected bird (as defined by WDFW or the Bald and Golden Eagle Protection Act) shall be removed and the nest shall not be disturbed unless the applicant obtains approval from WDFW.

Response: None of the trees proposed to be removed have active eagle or osprey nests located in them.

d. Vegetation removal conducted for the purposes outlined in SMC 18.13.025(D)(1)(a through d) shall comply with the regulations therein.

Response: All applicable regulations are complied with for vegetation removal as demonstrated in the landscape plans and submitted application materials.

e. Aquatic weed control shall be allowed only where the presence of aquatic weeds will affect native plant communities, fish and wildlife habitats, or an existing water dependent use adversely. Aquatic weed control efforts shall comply with all applicable laws and standards.

Response: There is no aquatic weed control proposed with this application.

- f. Mitigation Area, Location. The location of the mitigation area shall:
 - i. Be on site unless there is insufficient area on site;
 - ii. Improve an area of low habitat functionality;
 - iii. Be within 50 feet of the OHWM or as close as possible to the shoreline waterbody;
 - iv. Prioritize south and west banks of waterbodies to provide shade.

Response: The mitigation proposed is located completely on site and as close to the OHWM as possible. Please see the proposed mitigation plan for details.

- g. Mitigation Area, Monitoring.
 - i. The project shall be monitored annually for 5 years to document plant survivorship.
 - ii. Monitoring reports shall be provided to the Administrator once per year.



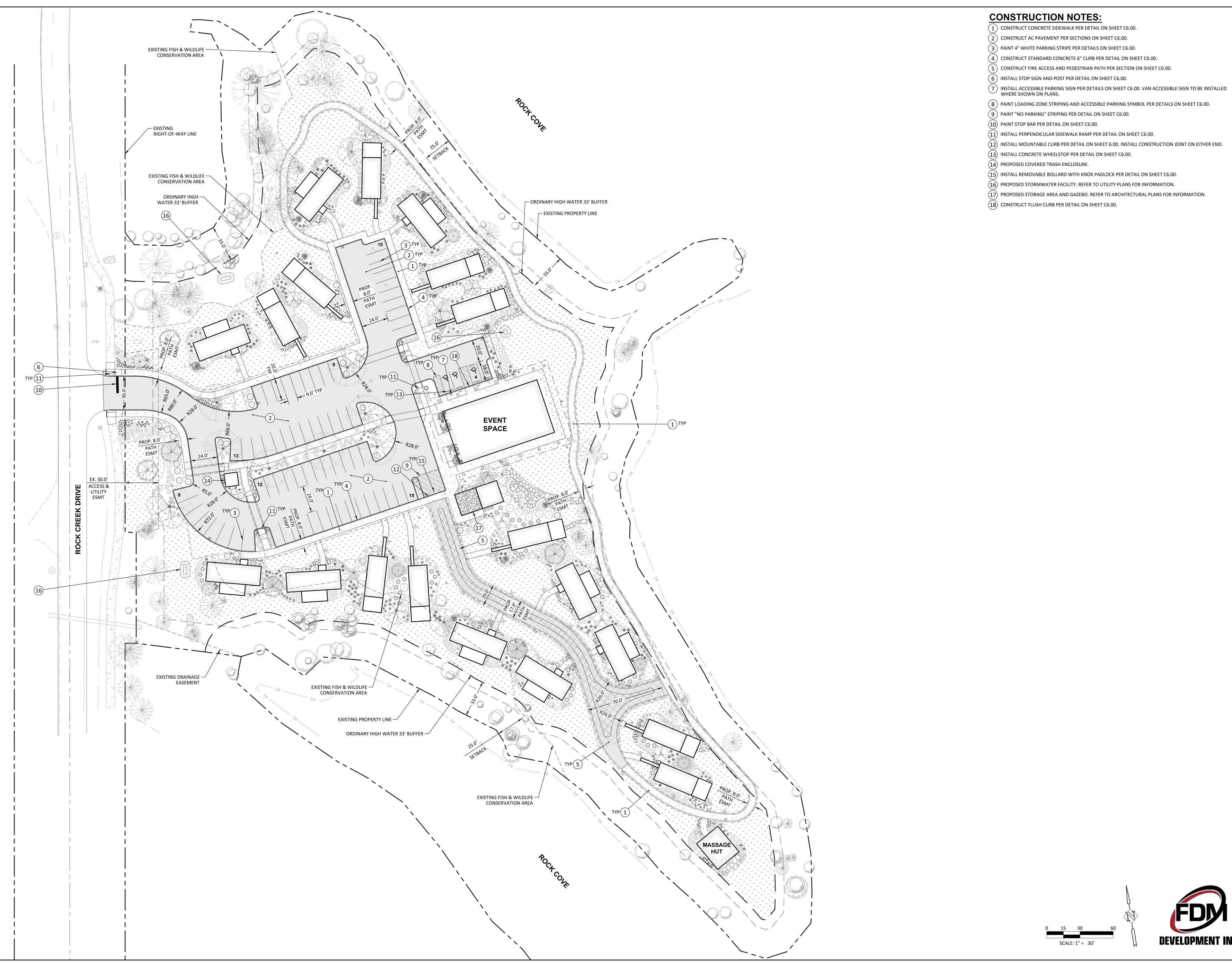
- iii. The planted mitigation area shall achieve a plant survival standard of 80% at the end of 5 years.
- iv. Monitoring results may require additional/replacement planting to meet the survival standard. If the survival standard is not met, then additional planting may be required and the monitoring period extended.
- v. A conservation covenant may be established which prevents future development or alteration within the mitigation area.

Response: The property owner will monitor the mitigation area and plantings to ensure survivorship of the proposed landscaping.

IV. CONCLUSION

This written statement and the accompanying supporting documents demonstrate compliance with the applicable approval criteria for a Site Plan Review and Shoreline Substantial Development Permit in the City of Stevenson. Therefore, the applicant respectfully requests that the County approve the application.





(2) CONSTRUCT AC PAVEMENT PER SECTIONS ON SHEET C6.00.

(6) INSTALL STOP SIGN AND POST PER DETAIL ON SHEET C6.00.

8 PAINT LOADING ZONE STRIPING AND ACCESSIBLE PARKING SYMBOL PER DETAILS ON SHEET C6.00.

(12) INSTALL MOUNTABLE CURB PER DETAIL ON SHEET 6.00. INSTALL CONSTRUCTION JOINT ON EITHER END.

(13) INSTALL CONCRETE WHEELSTOP PER DETAIL ON SHEET C6.00.

(15) INSTALL REMOVABLE BOLLARD WITH KNOX PADLOCK PER DETAIL ON SHEET C6.00.

(16) PROPOSED STORMWATER FACILITY. REFER TO UTILITY PLANS FOR INFORMATION.

ROCI

NOT FOR CONSTRUCTION

FDM-01A



CRITICAL AREAS AND FWHCA REPORT

Updated May 3, 2023



Rock Creek Cove Hospitality
Stevenson, Washington

Prepared for

FDM Development 5101 NE 82nd Ave, Suite 200 Vancouver, WA 98662 (210) 849-5592

Prepared by

Ecological Land Services, Inc.

1157 3rd Avenue, Suite 220A • Longview, WA 98632 (360) 578-1371 • Project Number 2682.02

SIGNATURES

This report was prepared by the undersigned:

Andrew R. Allison Wetlands Scientist

Ecological Land Services, Inc.

Kate'Lyn (KT) Wills Senior Biologist

Ecological Land Services, Inc.

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WDFW's "Living with Wildlife: Canada Geese"

Introduction

Ecological Land Services (ELS) has prepared the following critical areas report and compensatory mitigation plan for FDM Development (the applicant) as a component of the proposed mixed-use hospitality development adjacent to Rock Creek Cove on parcels 02070100130300, 02070100130400, and 02070100130200 (study area) in the City of Stevenson, Skamania County, Washington. The study area is in the SW ¼ of the NW ¼ of Section 1, Township 2 N, and Range 7 East of the Willamette Meridian, coordinates 45.6890, -121.8992, and is accessed from SW Rock Cove Dr (Figure 1). The study area's zoning is "Commercial" (C1). This report provides a description of existing critical areas on the proposed development site, a summary of proposed impacts from development, and a mitigation proposal for unavoidable impacts.

SMC 18.13.050 - Critical areas report requirements

A. Qualified Professional. When required by this chapter, the applicant shall submit a critical area report prepared by or under the direct supervision of a qualified professional as defined herein.¹

Ecological Land Services Inc. (ELS) is an environmental consulting firm with twenty-four years' experience specializing in natural resources management and land use planning. Andrew Allison has been employed by ELS for 9 years and has a total of 12 years' experience in critical areas analyses that include habitat associated with wetlands, streams, woodlands, and agriculture. He has completed critical areas assessments, prepared critical area determination reports, and designed wetland and habitat mitigation plans in Southeast Alaska, Washington, and Oregon that include urban, rural, and wilderness environments.

B. Best Available Science. The critical area report shall use scientifically valid methods and studies in the analysis of critical area data and field reconnaissance and reference the source of science used. The critical area report shall evaluate the proposal and all probable impacts to critical areas in accordance with the provisions of this chapter.

Stream Assessment:

ELS uses guidance provided by the Washington State Department of Ecology² (Ecology) and the U.S. Environmental Protection Agency³ (EPA) to inform decisions about the location of an ordinary high water mark (OHWM) and to make determinations about stream characteristics, including habitat functions and flow dynamics. The Shoreline Management Act (SMA) of Washington State defines OHWM as a mark "...found by

^{1 &}quot;Qualified professional" means a person with experience and training in accordance with WAC 365-195-905(4).

² Publication No. 16-06-029: "Determining the Ordinary High Water Mark for Shoreline Management Act Compliance in Washington State", revised October 2016.

³ Publication No. 910-K-14-001: "Streamflow Duration Assessment Method for the Pacific Northwest", November 2015.

examining the bed and banks and ascertaining where the presence and action of waters are so common and usual, and so long continued in all ordinary years, as to mark upon the soil a character distinct from that of the abutting upland..." (RCW 90.58.030(2)). ELS, in collaboration with Ecology staff, used principles in this guidance as well as site-specific indicators to identify the OWHM of the Columbia River within the study area boundary. Site specific indicators included transitions in vegetation, wrack lines, scouring under trees and exposed roots, and breaks in topography.

Wetland Assessment:

ELS follows the Routine Determination Method developed by the U.S. Army Corps of Engineers (Corps) for wetland delineation.⁴ The Routine Determination Method examines vegetation, soils, and hydrology to determine if wetland is present. EPA defines wetlands as "...areas that are inundated or saturated by surface or ground water at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions."

- C. Minimum Report Contents. At a minimum, the report shall contain the following:
- 1. The name and contact information of the applicant and landowner (if different).

<u>Applicant:</u> <u>Landowners (represented by the applicant):</u>

FDM Development, Inc. Erwin L&K, LLC 5101 NE 82nd Ave, Suite 200 OPH DBD, LLC

Vancouver, WA 98662 Rawlings Family Investments, LLC

(210) 849-5592

2. The street address and tax lot number of the site proposed for the regulated activity.

Parcel Numbers: 02070100130200, 02070100130300, 02070100130400

Map Number: U-CR-P

Site Address: Rock Creek Dr.
Description: Lot 2 BK T/PG 100

Total Acreage 6.40

Zoning: Commercial Recreation (CR)

3. A description of the proposal and identification of the permit requested.

Rock Creek Cove Hospitality project is a mixed-use development adjacent to Rock Creek Cove on the former Hegewald Lumber Mill Site in Stevenson, WA. The project seeks to

⁴ "Corps of Engineers Wetlands Delineation Manual", Wetlands Research Program Technical Report Y-87-1 (Environmental Laboratory 1987) and the "Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Western Mountains, Valleys and Coast Region (Version 2.0)" (U.S. Army Corps of Engineers, May 2010)

complement the existing tourism industry in Stevenson by offering condo- and studio-sized units available for nightly and weekly rental, totaling 48 available bedrooms. A 15,000 square-foot commercial venue space will anchor the development and provide wide views of Rock Creek Cove and the Columbia River Gorge. The conceptual space planning of the commercial building consists of 5,000 open venue space, supported by 10,000 square feet of service, food preparation, and guest lounging area. The development seeks to attract both local and regional visitors, with venue space available for weddings, company parties, family reunions, and corporate retreats.

The project is proposed in three phases of development: Phase 1 includes condo-style units, operated by a single ownership group. Phase 2 will add the commercial venue space and restore water-side portions of the property for enhanced, publicly-accessible observation and enjoyment. Phase 3 completes the development with the studio-sized units, operated under the same ownership group as the remainder of the property.

The applicant is seeking a Critical Areas Permit from the City of Stevenson for an approximate 0.19-acre impact to the Columbia River's fish and wildlife habitat conservation area (FWHCA) (Figure 3).

4. A detailed plan of the proposal site and all adjoining areas within 100 feet, drawn to a standard engineering scale and submitted on 8 $\frac{1}{2}$ "×11" or 11"×17" paper.

The existing and proposed conditions are included with this report as Figures 2, 3, and 4. Its scale is 1:200 and it is on 11x17 paper. These figures include:

- a. location and description of critical areas and buffers
- b. existing conditions of the property
- c. location, species, and diameter of significant trees
- d. location and extent of proposed regulated activities

Details related to stormwater management are included in the engineer's drawing, included with this report's Appendix.

5. The dates, names, and qualifications of the persons preparing the report and documentation of any fieldwork performed on the site.

ELS (Andrew Allison, qualifications provided above) and Ecology (Rebecca Rothwell, Wetlands and Shorelands Technical and Regulatory Lead) completed fieldwork on December 30, 2019. We assessed critical areas and fish and wildlife habitat in the study area together, and physically demarcated the OHWM of the Columbia River in the study area using consecutively numbered fluorescent tape flagging. S&F Land Services, a

professional surveyor, recorded the flag locations on the same day. ELS and Ecology agreed wetlands were not present in the study area.⁵

6. Identification and characterization of all critical areas, wetlands, water bodies, and buffers adjacent to the proposed project area. For areas off site, estimate conditions within 300 feet of the project boundaries using the best available information.

ELS and Ecology identified one unnamed tributary to the Columbia River at the north end of the study area; the tributary is designated "Type F" by Washington Department of Natural Resources (DNR) (Figure 6). The Columbia River is designated "Type S" and is a shoreline of statewide significance. Rock Cove, a side channel of the Columbia River, surrounds the study area on three sides (Figures 2 and 3). According to SMC 18.13.095(D), the area designated as a fish and wildlife habitat conservation area (FWHCA) for Type F waters is 100 feet and Type S waters is 150 feet. Existing conditions within 300 feet of the study area include SW Rock Creek Drive and single family residences to the west, an assisted living community to the south, and Rock Cove (open water) to the north and east.

Vegetation in the study area's FWHCA consists of mature deciduous and evergreen trees spaced along the north, east, and southwest shoreline with an understory of woody shrubs and herbaceous species. Shrub species are best established between the study area and SW Rock Creek Drive, roughly the northwest and southeast portions of the subject shoreline; elsewhere, shrubs and herbaceous vegetation are sparse or absent due to existing impervious surfaces, riprapped embankments, and historic disturbances from industrial activities that include uses by the timber industry and municipal materials storage. Approximately 70 percent of the study area's shoreline is armored with ripraplike material that consists of loose stones, gravel, fragments of concrete, and large pieces of metal (i.e. rebar, logging cable, nonspecific steel remnants). Derelict pilings are located a few feet offshore near the northeast portion of the study area.

In most places the transition from top-of-bank to the OHWM is relatively steep. Erosion control in the steeper portions of the shoreline has been historically achieved with ripraplike armoring. Approximately 65 percent of the shoreline is armored with material that consists of loose stones, gravel, fragments of concrete, and large pieces of metal (i.e. rebar, logging cable, and non-specific steel remnants). Derelict in-water pilings are located along the southeast shoreline of the study area and formerly supported timber industry infrastructure.

⁵ The National Wetlands Inventory (NWI) maps one wetland in the study area, identified as PEM1/UBFh (Figure 7). ELS and Ecology reviewed the area mapped as wetland by the NWI and determined it is part of Rock Cove, within the OHWM of the Columbia River, and not an independent wetland unit.

⁶ Table 18.13.095-1

7. A statement specifying the accuracy of the report, and all assumptions made and relied upon.

ELS bases this report's determinations on standard scientific methodology and best professional judgment. In our opinion, local, state, and federal regulatory agencies will agree with our determinations; however, the information contained in this report should be considered preliminary and used at your own risk until it has been approved in writing by the appropriate regulatory agencies. ELS is not responsible for the impacts of any changes in environmental standards, practices, or regulations after the date of this report.

8. A discussion of the regulatory standards applicable to the critical areas and proposed activities.

Regulatory standards that apply to the applicant's proposed development include compliance with the City of Stevenson's December 2018 Shoreline Master Program (SMP), and SMC Chapter 18.13 *Critical areas and natural resource lands*.

a. SMP requirements: The standard shoreline management area (or shoreline setback) for all designated shorelines in Washington State is 200 feet, measured landward from the OHWM. The study area is zoned "active waterfront"; according to Stevenson's 2018 SMP, setbacks for development proposed in active waterfront is typically 50 feet. Accordingly, the applicant is keeping all development outside of the 50-foot setback as demonstrated in Figure 3.

Regarding improvements from prior industrial land uses including concrete and gravel surfaces, gravel roads, the graveled boat launch, and riprap embankments, a shoreline use that was lawfully constructed prior to the effective date of the SMA or the December 2018 SMP and that does not conform to the current SMP standards is considered a nonconforming use. For the purposes of the December 2018 SMP, existing roads (whether asphalt, gravel, or dirt) are considered nonconforming uses and do not need a Shoreline Conditional Use Permit to be retained or improved (SMP 2018).

- b. This report is a discussion of all other regulatory standards applicable to SMC Chapter 18.13 *Critical areas and natural resource lands*.
- 9. A description of efforts to apply mitigation sequencing pursuant to SMC 18.13.055 specific to avoidance, minimization, compensation, and preservation measures proposed for the critical areas.

Rock Creek Cove Hospitality is proposed on the former Hegewald Lumber Mill Site which was active from the 1950's through the 1970's. The Natural Resource Conservation

Service (NRCS) describes soils in the study area as Arents 0 to 5 percent slopes. Arents do not have diagnostic horizons because they have been deeply mixed by plowing, spading, or other methods of moving by humans (NRCS 2020). Existing conditions indicate the site's history of disturbance from industrial timber processing; these conditions can be roughly categorized as impervious surface, disturbed/maintained pervious surfaces, and moderately vegetated shoreline.

Impervious surface occupies approximately 1.25 acres and consists of compacted gravel, asphalt, or concrete formerly used for roads, staging pads, or parking areas, and a boat launch. Disturbed/maintained pervious surfaces occupy approximately 4 acres are characterized by areas with little to no plant cover, low plant species diversity, and that have stockpiles of rock or woody materials. Moderately vegetated shoreline occupies approximately 1.22 acres. These areas show signs of prior industrial land use but have not been maintained and, in the absence of maintenance, reestablished trees and shrubs with moderate levels of diversity and interspersion. Critical areas in the study area are the FWHCA for Rock Cove and the unnamed tributary to Rock Cove, and one Oregon white oak tree (Figure 2). The FWHCA is primarily within moderately vegetated shoreline.

In adherence to mitigation sequencing pursuant to SMC 18.13.055 and with specificity to avoidance, the applicant is proposing most development in areas that are either existing impervious surfaces, previously disturbed ground, or that are otherwise prevented from providing buffer functions by shoreline armoring (Figures 2, 3, and 4). Pursuant to minimization, the applicant is proposing a 0.19-acre impact to Rock Cove's FWHCA in the southwest portion of the study area in a part of the buffer that, in addition to prior industrial land use, was used for storing rock, cobbles, and other materials until sometime after 2007 (Figure 4, 2007 aerial base map). The proposed impact area meets the definition of a "degraded" buffer as it is defined in SMC 18.13.010(B)(15).⁷ Proposed mitigation for these impacts is discussed below in the requirements for FWHCA reports, SMC 18.13.095(F) Habitat Mitigation.

10. Any additional information required for the critical area as specified in the corresponding section.

SMC 18.13.095(D)(3) identifies functionally isolated buffer as lawns, pre-existing roads and structures, vertical separation, and other areas that do not protect the FWHCA from adverse impacts. Shoreline armoring meets the description of a preexisting structure that that does afford protection from adverse impacts. It lacks pervious surfacing for detaining and/or filtering sediment loads in surface runoff, an established and diverse native

Rock Cove Creek Hospitality Project Critical Areas and FWHCA Report Ecological Land Services, Inc. May 3, 2023

⁷ Areas of the FWHCA that are dominated by more than 30 percent aerial coverage of invasive vegetation (primarily Himalayan blackberry (*Rubus armeniacus*)) and/or by fill, gravel, debris, asphalt, and other non-native material.

vegetation community able to provide forage, screening, refuge, or denning opportunities for wildlife species, and over-water shading for near-shore aquatic wildlife in the Columbia River. Accordingly, those portions of the study area that contain armoring satisfy the buffer exemption criteria per SMC 18.13.095(B)(3) (Figure 2). ⁸ Additional areas of buffer isolation are located near the entryway to the study area from Rock Creek Drive and consist of maintained vegetation adjacent to impervious surfaces (Figure 2).

SMC 18.13.095 - Fish and wildlife habitat conservation areas

C. FWHCA Reports

1. In order to determine the extent of the appropriate buffers on a site when the nature of the fish and wildlife habitat conservation area is unclear, the applicant may submit a preliminary habitat assessment report as prepared by a qualified professional.

ELS prepared a critical areas' report and conceptual mitigation plan for the preliminary review of this application, dated June 16, 2020. The city's environmental consultant, Olson Environmental LLC (OE), provided review and comments on this report dated June 17, 2020. OE's findings for the preliminary report concur with FWHCA buffer isolation. A copy of ELS's preliminary report and OE's findings letter is included with this report for reference (Appendix).

In addition to the minimum requirements for critical area reports contained in SMC 18.13.050, a preliminary FWHCA report should also contain the following information:

a. Confirmation or correction of the classifications for the FWHCA and/or stream type as defined in this chapter.

ELS confirms there are FWHCAs in the study area for the unnamed tributary to Rock Cove (Type F) and for Rock Cove, a side channel of the Columbia River (Type S). ELS does not recommend revising the stream types or the FWHCA classifications.

b. Characterization of riparian vegetation species, composition, and habitat function.

Vegetation in Rock Cove's FWHCA consists of mature deciduous and evergreen trees spaced along the north, east, and southwest shoreline with an understory of woody shrubs and herbaceous species. Tree species include red alder (*Alnus rubra*), Douglas fir (*Pseudotsuga menziesii*), black cottonwood (*Populus trichocarpa*), and one Oregon white oak (*Quercus garryana*). Shrub species were best established between the study area and SW Rock Creek Drive, roughly the northwest and southeast portions of the subject

Rock Cove Creek Hospitality Project Critical Areas and FWHCA Report Ecological Land Services, Inc. May 3, 2023

⁸ Armoring occupies approximately 65 percent of the shoreline. The remaining 25 percent is moderately well vegetated; moderately well vegetated areas are the northwest and southwest potions of the site (Figure 2).

shoreline; elsewhere, shrubs and herbaceous vegetation are sparse or absent due to existing impervious surfaces, riprapped embankments, and historic disturbances from industrial activities that include uses by the timber industry and municipal materials storage. Shrub species include common snowberry (*Symphoricarpos albus*), beaked hazelnut (*Corylus cornuta*), spirea (*Spiraea douglasii*), and Himalayan blackberry (*Rubus armeniacus*). Himalayan blackberry is common throughout Rock Cove's FWHCA. Herbaceous vegetation was primarily established in the transition zone above and below OHWM with reed canarygrass (*Phalaris arundinacea*), soft rush (*Juncus effuses*), dogwood (*Cornus sericea*), and spirea rooted at or below ordinary high and common grasses rooted above.

A small portion of the study area intersects with the FWHCA for the unnamed tributary in the northwest corner adjacent to SW Rock Creek Dr. Vegetation at this intersection point is characterized by a canopy and understory as discussed above for Rock Cove.

c. Description of the soil types adjacent to and underlying the stream, using the Soil Conservation Service soil classification system.

ELS uses the Natural Resource Conservation Service (NRCS) map unit descriptions to gather baseline soil data. NRCS identifies soils in the study area as Arents 0 to 5 percent slopes. Arents is described by NRCS as a well-drained, terraced soil with more than 80 inches depth to the groundwater table. A typical profile includes gravelly sandy loam from 0 to 24 inches and extremely gravelly sandy loam between 24 and 60 inches. As mentioned previously, Arents do not have diagnostic horizons because they have been deeply mixed by plowing, spading, or other methods of moving by humans (NRCS 2020). ELS did not collect soils data additional to the existing NRCS mapping data due to the prevalence of impervious and disturbed soil conditions, and consensus with Ecology that collecting soils data to demonstrate the absence of wetlands was not necessary for Ecology's purposes in the study area.

d. Identification of the qualities of the area that are essential to maintain feeding, breeding, and nesting, and an assessment of potential project impacts to the use of the site by the species.

Some of the study area's northern and southern FWHCA, as well as the entirety of Rock Cove, is identified by Washington Department of Fish and Wildlife's (WDFW) Priority Habitat and Species (PHS) mapping as a breeding area for Canadian geese (*Branta canadensis*) (Figure 9). Rock Cove is also identified by PHS as providing habitat for resident coastal cutthroat (*Oncorhynchus clarki*), Fall Chinook (*Oncorhynchus tshawytscha*), Winter Steelhead (*Oncorhynchus mykiss*), and rainbow trout (*Oncorhynchus mykiss*) (WDFW 2020) (Figure 9). Canada geese are not sensitive, threatened, or endangered, and FWHCA in the study area does not provide habitat that

is unique, important, or necessary for the species. As stated by WDFW, "...northwest Oregon and southwest Washington are now wintering more Canada geese than at any other time in recorded history" (WDFW 2015). ELS did not observe goose nests in the study area, or evidence that geese use the site (tracks, feathers, and droppings were absent). The applicant is not proposing in-water work or work within 50 feet of the OHWM; accordingly, there are no anticipated impacts to fish or Canada geese from the proposed development.

The study area is in a northern spotted owl management buffer. Spotted owl habitat usually consists of mature and old-growth coniferous forests with high canopy cover, trees of varying sizes, snags, and large downed wood (Buchanan, J.B. 2016). Suitable old-growth forests are approximately 150-200 years old. The study area and surrounding properties are managed for timber production; consequently, they do not meet criteria for spotted owl habitat.

e. A discussion of any federal, state, or local species/habitat management recommendations, including the WDFW habitat management recommendations that have been developed for the identified species or habitat.

There are no specific management recommendations for Canada geese as regards the success of the species. WDFW published a document titled "Living with Wildlife: Canada Geese" in 2005. This document in referenced in WDFW's Priority Habitat and Species List published in 2008 and updated in February 2020. The document provides options for coexisting with geese and resolving conflicts that arise between geese and human land uses, resulting from populations of resident Canada geese "...dramatically increas[ing] over the past 25 years, particularly in urban areas where there are few predators, prohibitions on hunting, and a dependable year-round supply of food and water" (WDFW 2005). A copy of this document in included in the Appendix of the report for reference.

The applicant is not proposing in-water work or work within 50 feet of the OHWM; accordingly, there are no anticipated impacts to fish. The applicant will follow appropriate BMPs during construction and meet the requirements outlined in Ecology's Stormwater Management Manual for Western Washington (Ecology 2019) to further ensure Rock Cove does not receive sediment, surface runoff, or any other input that would potentially affect water quality or fish habitat as a result of project construction.

f. Recent photographs of the property, including detailed photographs of the habitat resource in question.

On-the-ground color photographs of the study area taken by ELS in December 2019 are included with this report (Photoplates, Appendix).

g. An outline of standard buffer widths, available buffer reductions, or potential opportunities for enhancement/mitigation.

SMC 18.13.095(D) identifies the FWHCA for Type F waters as 100 feet and Type S waters as 150 feet. There are no proposed buffer reductions for this project, which is a revision from the preliminary critical areas report and conceptual mitigation plan prepared by ELS and reviewed by OE. After reviewing OE's findings letter with the applicant and OE's representative, and reviewing SMC 18.13, ELS concluded that eliminating buffer reduction from the proposal increases the amount of buffer enhancement available for mitigating the proposed buffer impact onsite. With the standard 150-foot FWHCA applied and adjusted for functional isolating features, the applicant is proposing approximately 0.19-acre buffer impact (Figures 3 and 4). Impacts include permanent development in a portion of the buffer that is already degraded from historic land uses. As mitigation, the applicant is proposing approximately 1.12 acres of buffer enhancement by densely planting the remaining buffer area with native shrubs and removing non-native invasive Himalayan blackberry, a dominant invasive plant in the study area. Blackberry removal and shrub installation will increase native plant diversity, improve habitat opportunities for a variety of native birds and mammals (both water-dependent and terrestrial), increase foraging value, and decrease opportunities for non-native plants and animals to occupy the site. The proposed mitigation is in-kind buffer enhancement at a 5.9:1 ratio, significantly exceeding the 1:1 enhancement ratio requirement for onsite FWHCA mitigation per SMC 18.13.095-3.

SMC 18.13.095(F) - Habitat mitigation

1. Compensatory mitigation for impacts to FWHCA's shall achieve equivalent or greater functions as those affected by the proposed project.

The proposed FWHCA enhancement increases the existing buffer functions and values.

- 2. FWHCA Mitigation Plan. When a project involves FWHCA or FWHCA buffer impacts, enhancements, or reductions, a habitat mitigation plan by a qualified professional shall be required. At a minimum, the habitat mitigation plan must contain the following information:
- a. All the information required in the FWHCA Report prepared under SMC 18.13.095(C).

The report is a fulfillment of this requirement.

b. A copy of the site plan for the development proposal.

The site plan is included in Figures 3 and 4, and in the engineer's drawing in the Appendix.

c. A description of the mitigation sequence developed for the project according to SMC 18.13.055.

Please refer to item 9, page 7 of the report for the discussion of the applicant's mitigation sequencing.

i. Existing conditions of the enhancement area, including location:

The proposed enhancement areas are a combination of moderately vegetated shoreline and previously disturbed ground at the north and south ends of the study area (Figures 3 and 4). These are portions of the FWHCA that have not been regularly maintained or have been minimally maintained during the last 10 to 15 years, and longer in some places. Existing tree species include red alder, Douglas fir, and black cottonwood; cottonwood and alder are closer to the OHWM, fir is mid-slope to top-of-bank. Shrub species are sparsely to moderately interspersed under tree canopy and include common snowberry, beaked hazelnut, and Himalayan blackberry.

ii. Rational for site selection

The study area's history of industrial use provides an opportunity for onsite habitat improvement. The areas selected for improvement have existing canopy cover, lack armored shoreline, and are adjacent to sheltered coves formed by the topographic configuration of the study area. The existing canopy cover provides cooler temperatures and higher, more consistent soil moisture for installed native shrubs, and will help minimize potential regrowth of Himalayan blackberry through shading. The absence of shoreline armoring increases soil availability for installed plants' root establishment, decreases the amount of time and equipment necessary to prepare the site for enhancement, which together improves the overall likelihood that installed plants will succeed quickly. Lastly, sheltered coves provide unique attributes that increase habitat potential: they are more secluded than other parts of the study area's shoreline, both from natural elements such as wind and wave action, as well as minimizing future opportunities for human disturbance through topographic positioning; they have shallower water levels and consequently provide greater accessibility to habitats for birds, terrestrial mammals, and water-dependent species; and they have increased opportunity to provide off-channel salmonid habitat which would be improved by the proposed riparian vegetation enhancement.

iii. Estimated future condition of the enhancement area

Successful riparian vegetation enhancement will include a diverse native shrub understory, the absence or minimal presence of Himalayan blackberry (less than 10 percent cover), and evidence of frequent or ongoing seasonal native wildlife use.

iv. An assessment of all appropriate technical information necessary to assess the compensatory mitigation proposed.

This report is a fulfilment of the requirement.

d. The environmental goals and objectives of the mitigation

SMC 18.13.059 - Performance and monitoring standards

The goal of FWHCA enhancement is to provide high quality riparian habitat functions onsite using the following objectives and performance standards:

Objective 1. Provide high quality riparian habitat functions onsite.

- ▶ Performance Standard 1a. Enhance 1.12 acres of existing, moderately to poorly functioning FWHCA onsite. This performance standard is completed when the enhancement area is documented in the Year 1 Monitoring Report.
- ▶ Performance Standard 1b. Remove non-native invasive Himalayan blackberry from the enhancement site and areas adjacent to the enhancement site.
- ▶ Performance Standard 1c. Plant native shrubs trees and open areas in the understory and in areas formerly occupied by Himalayan blackberry. Plantings will achieve 100 percent survival in Year 1. Dead plants will be replaced if this performance standard is not met.
- ► Performance Standard 1d. Native shrubs will achieve at least 90 percent survival in Year 2. Dead plants will be replaced if this performance standard is not met.
- ► Performance Standard 1e. Native shrubs will achieve at least 80 percent survival in Year 3. Dead plants will be replaced if this performance standard is not met.
- ► Performance Standard 1f. Native shrubs will achieve at least 75 percent survival in Year 5. Dead plants will be replaced if this performance standard is not met.
- ▶ Performance Standard 1g. In all years, non-native invasive plant species will not exceed 10 percent cover in the enhancement area.

- ▶ Performance Standard 1h. In all years, native volunteer plants will be included in the survival calculation. If an installed plant dies and a volunteer plant emerges, the survival standard will be met.
 - Objective 2. Provide signage between the enhancement area and the development.
- ▶ Performance Standard 2a. Install FWHCA signs at a minimum of 50-foot intervals along the perimeter of the enhancement area facing the proposed development. This performance standard is complete when signs are installed and documented in a monitoring report.
- ▶ Performance Standard 2b. Install natural barriers where needed around the perimeters of the enhancement area. The need for barriers may not be apparent until after the development is complete. If needed/required, this performance standard is complete when the natural barriers are installed and documented in a monitoring report or memo.

Objective 3. Provide legally binding protection for the enhancement area.

▶ Performance Standard 3a. A conservation covenant or similar legal mechanism will be established for the enhancement area. The performance standard is complete when the City of Stevenson approves the conservation covenant or similar legal mechanism.

Planting schedule and equipment

Native shrubs will be installed in late winter or early spring when the plants are dormant, and the soil moisture conditions are favorable for planting. The following equipment may be used to prepare and install plants in the enhancement area: tree shovel, garden shovel, and power auger. Heavy equipment is not anticipated to be necessary unless remnants of industrial materials are discovered while planting and removal of such material is determined to be beneficial to enhancement goals.

Table 1: Proposed enhancement plants

Common Name/Botanical Name	Size	Spacing	Quantity
Vine maple, Acer circinatum	1 gallon	6-10 feet	100
Western service berry, Amelanchier alnifolia	1 gallon	6-10 feet	100
Oceanspray, Holodiscus discolor	1 gallon	6-10 feet	100
Tall Oregon grape, Mahonia aquifolium	1 gallon	6-10 feet	100
Common snowberry, Symphoricarpos albus	1 gallon	6-10 feet	100
Sword fern, Polystichum munitum	1 gallon	6-10 feet	100
		Total	600

Specifications for site preparation, planting, and maintenance

Preparing the enhancement area

- Install silt fencing where necessary to control runoff from the development.
- Install temporary construction fencing along the perimeters of the enhancement area bordering the development.
- Remove Himalayan blackberry. Selectively apply herbicide as necessary.

Installing habitat signs

• Install durable signs at a minimum of 50-foot intervals on metal or wood posts where the enhancement area is adjacent to proposed development.

General plant specifications

- Plant the native shrubs during the late winter or early spring at the spacing identified in Table 1.
- Group the plants in uneven patches dominated by a single species or interspersed with one another where no shrubs currently exist.
- All plant materials will be kept cool and moist prior to installation.
- All plant materials will have well developed roots and sturdy stems, with an appropriate root to shoot ratio.
- No damaged or desiccated roots or diseased plants will be accepted.

Planting shrubs

- Dig the receiving hole several inches wider than the size of the root system.
- Position the planted species' root collar so that they are at or slightly above the level of the surrounding soil to allow for settling.
- Back the hole with soil.
- Gently compact the soil around the planted species to eliminate air spaces.
- Install a minimum of 3-inch depth by 4-foot diameter mulch layer around the base of planted species. The mulch will be comprised of clean, chipped wood. Avoid placing mulch directly against plant stems.
- Irrigate all newly installed plants as site and weather conditions warrant.

e. A monitoring and maintenance proposal compliant with SMC 18.13.059

ELS recommends a 5-year monitoring and maintenance schedule in accordance with SMC 18.13.059(E)(1). Monitoring will begin the first growing season after the enhancement area is planted. Annual reports will be submitted to the City of Stevenson by December 31 of each monitoring year.

Monitoring plots

During the first annual monitoring event, monitoring plots will be established as follows:

1. A minimum of two permanent monitoring plots will be established, one in each enhancement area. Monitoring plots will be staked and identified with an aluminum tag, their location will be recorded with GPS, and they will be included on the as-built site map that accompanies each monitoring report. Photo points will be taken at the monitoring plots and elsewhere as needed to accurately document conditions.

Vegetation

Vegetation monitoring will measure the following:

- 1. Percent aerial cover of planted and naturally recruiting native trees and shrubs
- 2. Percent aerial cover of non-native, invasive plants
- 3. Change in the plant community over time (from photo points)

Fauna

Wildlife documentation will include the following:

- 1. Evidence of bird use (nesting, tree excavation, tracks in shoreline sediments, etc.)
- 2. Evidence of mammal use (scat, tracks, shedding hair or antlers, browse, bedding, denning, etc.)

Monitoring reports

The annual monitoring reports will contain at least the following:

- Location map and as-built map, and a revised plant quantity table as needed
- Description of monitoring methods
- Documentation of the presence and legibility of FWHCA signs
- Documentation of plant survival and cover
- Assessment of non-native, invasive plant species and recommendations for management
- Observations of wildlife
- Site photographs
- Summary of maintenance and contingency measures proposed for the next season and completed for the past season.

Enhancement area maintenance

Maintenance includes the following:

- Inspect the plants at least once annually, or more often as appropriate, and maintain to achieve the performance standards.
- Irrigate as-needed.
- Replace mulch as needed.
- Replace dead or failed plants to meet the minimum annual performance standards. Replaced plants will be installed as described for the original installation.
- Implementing a fertilizing schedule.
- Repairing damaged limbs or pruning dead branches.

Responsible parties

The Applicant, their successors, and/or their designee will be responsible for implementing the enhancement plan and its maintenance and monitoring. If the performance standards are not met by Year 5 an adaptive management plan will be developed and implemented. All adaptive management actions will be undertaken only after consulting with and gaining approval from the City of Stevenson. The responsible party will complete an adaptive management plan that describes 1) the need for adaptive management, 2) proposed actions, 3) time-frame for completing actions, and 4) any additional maintenance and monitoring necessary.

f. A bond estimate for the entire enhancement and/or compensatory mitigation project per SMC 18.13.059 - Performance and monitoring standards.

Table 2 on the following page includes the bond estimate for the proposed mitigation project. The estimate assumes initial site preparation and plant installation costs, followed by one maintenance and one monitoring trip each year for the subsequent two years.

Table 2. Bond estimate for the entire enhancement project

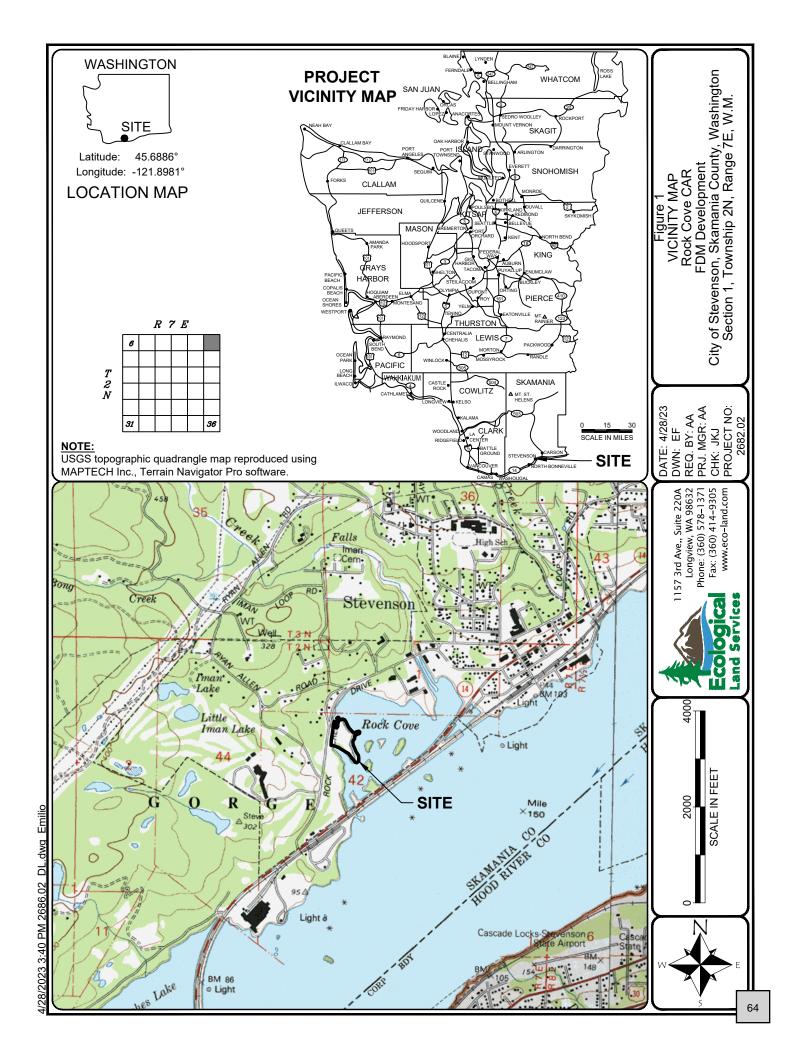
Year 1	Year 2	Year 3	Year 5	Years 1-5
Plant acquisition	Annual	Annual	Annual	
and installation	maintenance	maintenance	maintenance	Total Fatimated
\$2,500	\$750	\$750	\$750	Total Estimated
Monitoring	Monitoring	Monitoring	Monitoring	Mitigation Cost
report \$4,572	report \$4,572	report \$4,572	report \$4,572	
Total = \$7,072	Total = \$5,322	Total = \$5,322	Total = \$5,322	\$23,038

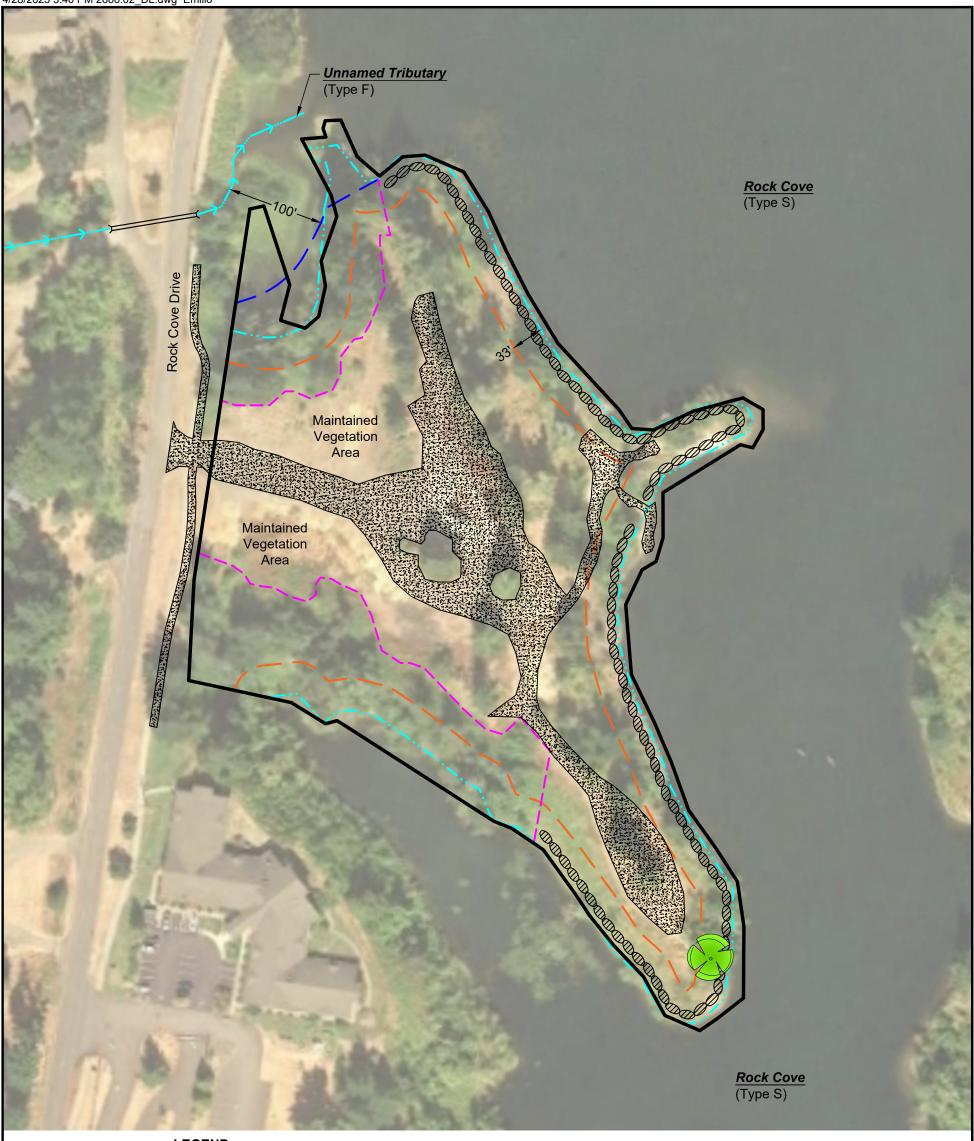
Limitations

ELS bases this report's determinations on standard scientific methodology and best professional judgment. In our opinion, local, state, and federal regulatory agencies will agree with our determinations; however, the information contained in this report should be considered preliminary and used at your own risk until it has been approved in writing by the appropriate regulatory agencies. ELS is not responsible for the impacts of any changes in environmental standards, practices, or regulations after the date of this report. Please contact Kate'Lyn (KT) Wills by email kt@eco-land.com or by phone (360) 578-1371 with any questions regarding the contents of this report.

References

- City of Stevenson, Council Authorized Draft, Shoreline Master Program. December 2018.
- Link, Russel. "Landscaping for Wildlife in the Pacific Northwest". University of Washington Press and the Washington Department of Fish and Wildlife. WDFW 1999.
- Link, Russel. "Living with Wildlife: Canada Geese" Adapted from "Living with Wildlife in the Pacific Northwest". WDFW 2005.
- National Wetlands Inventory (NWI) https://www.fws.gov/wetlands/data/mapper.HTML. Accessed July 2020.
- Natural Resource Conservation Service (NRCS). 2016. Soil Survey of Skamania County, Washington. Online document. http://www.or.nrcs.usda.gov/pnw_soil/wa_reports.html. Accessed July 2020.
- "Pacific Northwest Goose Management: a joint program of the Oregon Department of Fish and Wildlife and the Washington State Department of Fish and Wildlife". August 2015.
- Stevenson Municipal Code (SMC) Chapter 18.13 *Critical Areas and Natural Resource Lands*. November 2008.
- WDFW "PHS on the Web" https://geodataservices.wdfw.wa.gov/hp/phs/. Accessed July 2020.
- WDFW "State of Washington Priority Habitats and Species List". Updated February 2020.
- Washington Department of Natural Resources (DNR) "Forest Practices Application Mapping Tool". https://fpamt.dnr.wa.gov. Accessed July 2020.







Site Boundary

OHWM

Stream with Flow Direction

FWHCA Buffer for Type F

Functionally Isolated FWHCA Buffer for Type S (150')

50' Shoreline Management Plan Setback



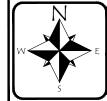
Oak Tree Location

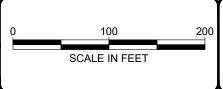
Existing Graveled or Concrete Surfacing

Existing Rip Rap

NOTE(S):

- Aerial from Google Earth™. (2017)
- OHWM line was determined through a joint effort by **Ecological Land Services and Washington Department** of Ecology on December 30, 2019. OHWM flags were professionally surveyed by S&F Land Services December 30-31, 2019.
- SMC 18.13.095(D)(3) identifies functionally isolated buffer as lawns, pre-existing roads and structures, vertical separation, and other areas that do not protect the FWHCA from adverse impacts.







DATE: 4/28/23 DWN: EF REQ. BY: AA PRJ. MGR: AA CHK: JKJ PROJECT NO: 2682.02

Figure 2
EXISTING CONDITIONS SITE MAP
ROCK Cove CAR FDM Development City of Stevenson, Skamania County, Washington Section 1, Township 2N, Range 7E, W.M.



LEGEND:

Site Boundary

Proposed Conrete Surfacing

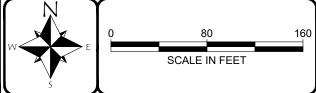
Proposed Asphalt Surfacing

Proposed Storm Pipe

Proposed Building

NOTE(S):

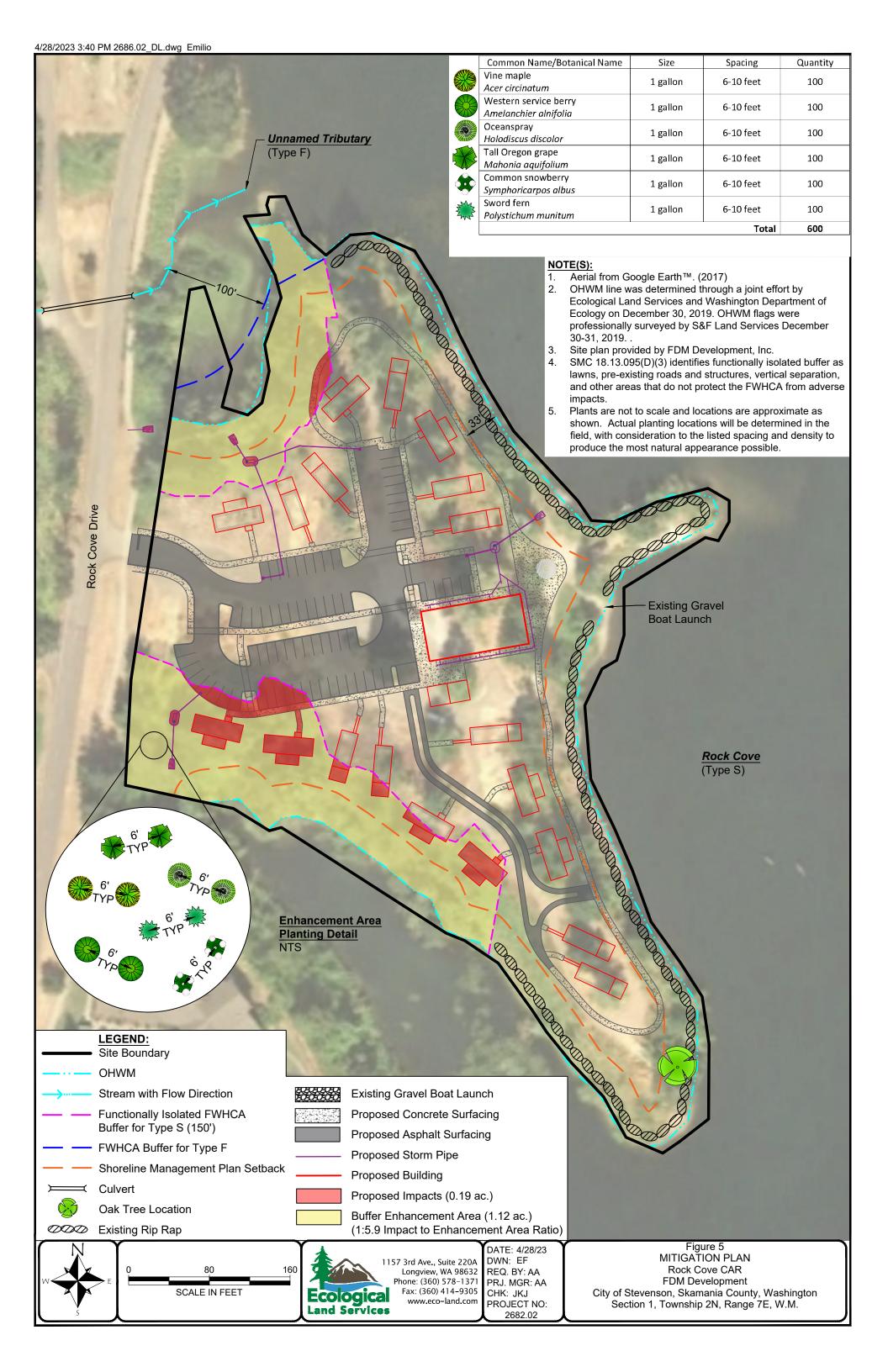
- Aerial from Google Earth™. (2007) Site plan provided by FDM Development, Inc.





DATE: 4/28/23 DWN: EF 1157 3rd Ave., Suite 220A Longview, WA 98632 Phone: (360) 578–1371 Fax: (360) 414–9305 www.eco–land.com REQ. BY: AA PRJ. MGR: AA CHK: JKJ PROJECT NO: 2682.02

Figure 4
PROPOSED CONDITIONS - 2007 AERIAL
Rock Cove CAR
FDM Development
City of Stevenson, Skamania County, Washington
Section 1, Township 2N, Range 7E, W.M.



LEGEND:

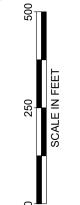
- **2** Arents, 0 to 5 percent slopes. Not hydric.
- 17 Bonneville stony sandy loam. Not hydric.
- **123** Steever stony clay loam, 2 to 30 percent slopes. Not hydric.
- 177 Water.

Figure 6 NRCS SOIL SURVEY Rock Cove CAR FDM Development City of Stevenson, Skamania County, Washington Section 1, Township 2N, Range 7E, W.M.

DATE: 4/28/23
DAWN: EF
32 REQ. BY: AA
71 PRJ. MGR: AA
55 CHK: JKJ
77 PROJECT NO:

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Phone: (360) 578-1371 PI
Fax: (360) 414-9305 CI
www.eco-land.com pi

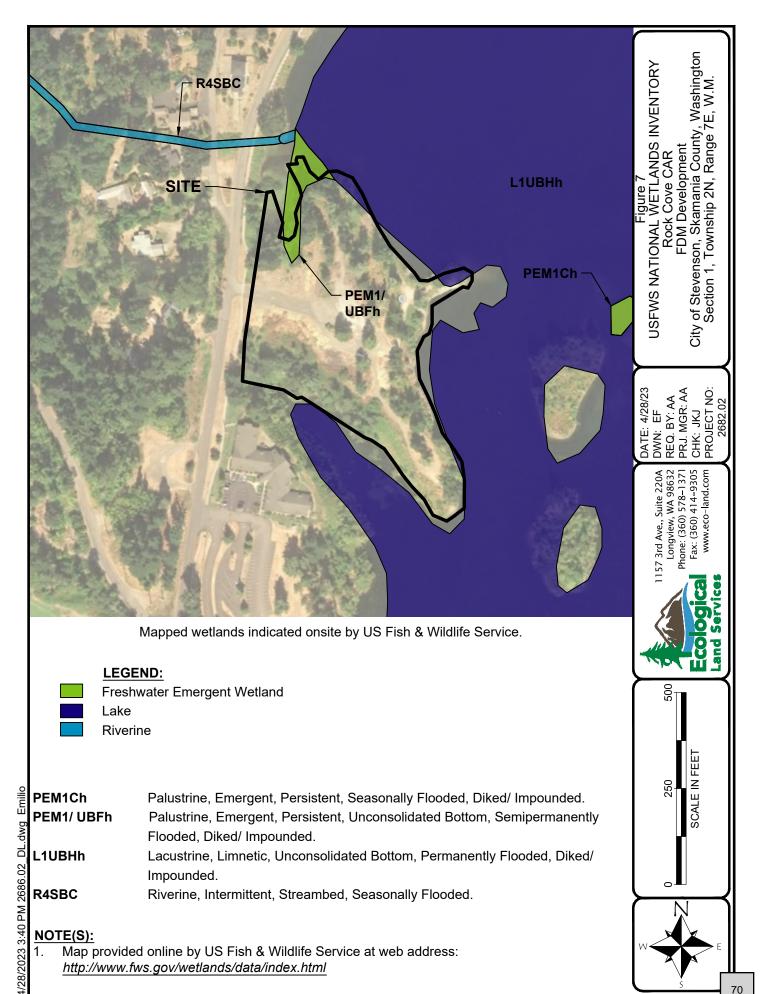


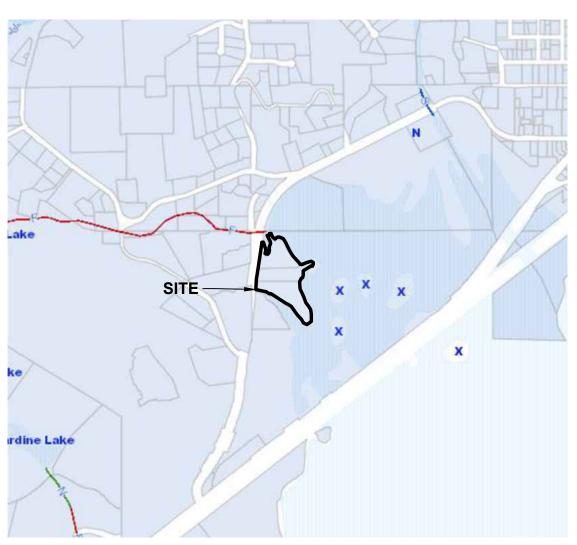




NOTE(S):

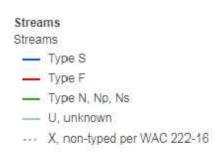
 Map provided online by NRCS at web address: http://websoilsurvey.nrcs.usda.gov/app/





No mapped streams indicated onsite by the Washington State Department of Natural Resources (DNR).

LEGEND:



<u>NOTE:</u> Map provided online by Washington State Department of Natural Resources at web address: https://fortress.wa.gov/dnr/protectiongis/fpamt/index.html

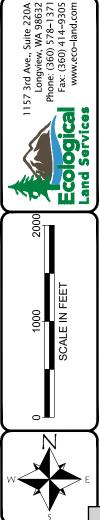


Figure 8

DNR STREAM TYPE MAP
Rock Cove CAR
FDM Development
City of Stevenson, Skamania County, Washington
Section 1, Township 2N, Range 7E, W.M.

DATE: 4/28/23 DWN: EF REQ. BY: AA PRJ. MGR: AA

CHK: JKJ PROJECT NO:



Figure 9

DNR NATURAL HERITAGE MAP
Rock Cove CAR
FDM Development

City of Stevenson, Skamania County, Washington Section 1, Township 2N, Range 7E, W.M.

DATE: 4/28/23 DWN: EF REQ. BY: AA PRJ. MGR: AA

CHK: JKJ PROJECT NO: 1157 3rd Ave., Suite 220A Longview, WA 98632 Phone: (360) 578-1371 Fax: (360) 414-9305 www.eco-land.com

SCALE IN FEET 1500

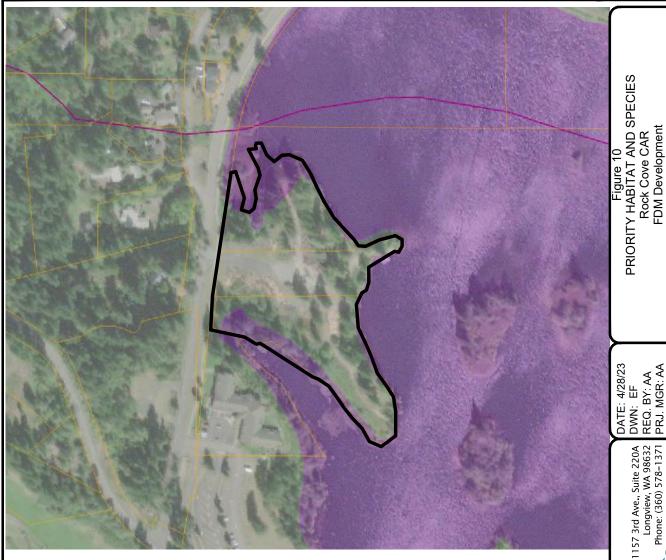
LEGEND:

Site Boundary

Bog Clubmoss (Lycopodiella Inundata)

Low Elevation Sphagnum Bog

NOTE: Map provided online by Washington State Department of Natural Resources at web address: https://fortress.wa.gov/dnr/protectiongis/fpamt/index.html



LEGEND:

Site Boundary

Canada Goose Northern Spotted Owl Waterfowl Concentrations

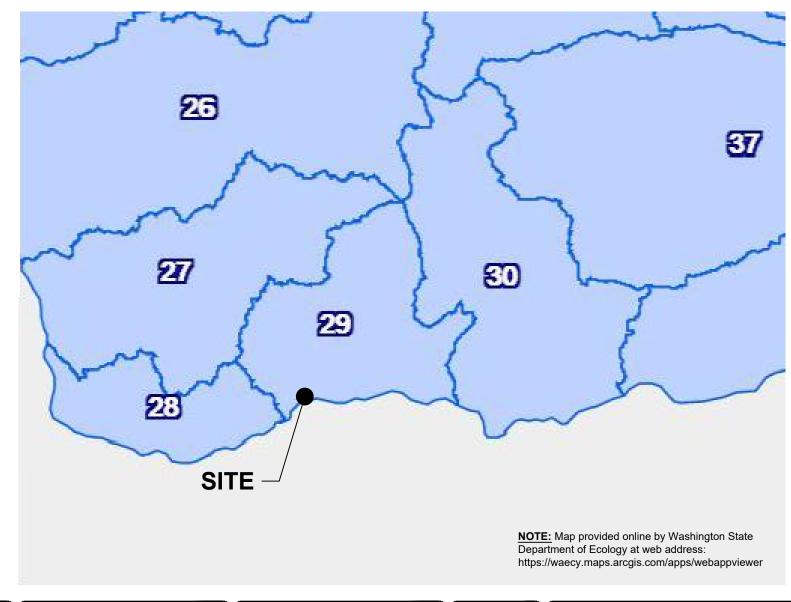
Winter Steelhead - Occurrence/Migration Rainbow Trout - Occurrence/Migration

SCALE IN FEET

1157 3rd Ave., Suite 220A Longview, WA 98632 Phone: (360) 578-1371 Fax: (360) 414-9305 www.eco-land.com

City of Stevenson, Skamania County, Washington Section 1, Township 2N, Range 7E, W.M.

NOTE: Map provided online by Washington State Department of Fish and Wildlife at web address: https://geodataservices.wdfw.wa.gov/hp/phs/





NOT TO SCALE



1157 3rd Ave., Suite 220A Longview, WA 98632 Phone: (360) 578–1371 Fax: (360) 414–9305 www.eco-land.com

DATE: 4/28/23 DWN: EF REQ. BY: AA PRJ. MGR: AA CHK: JKJ PROJECT NO: 2682.02 Figure 11
WRIA MAP
Rock Cove CAR
FDM Development
City of Stevenson, Skamania County, Washington
Section 1, Township 2N, Range 7E, W.M.

ROCK CREEK EVENT CENTER

GENERAL NOTES

I. VERIFY AND COORDINATE SITE CONDITIONS AND DIMENSIONS. BRING INCONSISTENCIES TO ATTENTION OF ARCHITECT BEFORE PROCEEDING WITH WORK.

2. PROVIDE SHORING AND BRACING AS NECESSARY TO ENSURE STRUCTURAL INTEGRITY WHEN WORK INCLUDES STRUCTURAL MEMBERS.

3. IMMEDIATELY BRING ERRORS AND OMISSIONS FOUND IN THESE DRAWINGS TO ATTENTION OF ARCHITECT. WHERE CONTRACTOR IS AWARE OF CONFLICTS OR OMISSIONS AND HAS NOT BROUGHT THEM TO ARCHITECT'S ATTENTION, IT IS UNDERSTOOD THAT CONTRACTOR HAS MADE PROVISIONS FOR MORE COSTLY AND STRINGENT METHOD. WHERE CONFLICTS, ERRORS OR OMISSIONS OCCUR IN DRAWINGS, PROJECT MANUAL, OR OTHER RELATED CONTRACT DOCUMENT PROVISIONS SUCH AS MANUFACTURER'S INSTRUCTIONS, REFERENCE STANDARDS, AND REGULATORY AGENCIES AND CODES, ASSUME MORE STRINGENT REQUIREMENTS AND VERIFY WITH ARCHITECT BEFORE BEGINNING WORK.

4. DO NOT SCALE DRAWINGS. FOLLOW DIMENSIONS SHOWN ON DRAWINGS AND ACTUAL FIELD MEASUREMENTS. NOTIFY ARCHITECT OF DISCREPANCIES.

5. ALL DIMENSIONS ARE TO FACE OF STUD, FACE OF CONCRETE AND MASONRY, FACE OF FRAMING OR CENTERLINE OF COLUMNS, UNLESS NOTED OTHERWISE.

6. COMPLY WITH BUILDING CODES AND OTHER APPLICABLE CODES AND ORDINANCES.

7. WORK INSTALLED IN CONFLICT WITH CONSTRUCTION DOCUMENTS IS NON-CONFORMING WORK AND REQUIRES CORRECTION AT NO ADDITIONAL COST TO OWNER AS DIRECTED BY ARCHITECT.

8. COORDINATE WORK IN COMPLIANCE WITH DRAWINGS AND SPECIFICATIONS TO ACCURATELY LOCATE MECHANICAL, ELECTRICAL, AND OTHER PENETRATIONS AND SYSTEMS WITH RESPECT TO STRUCTURAL FRAMING. DO NOT PENETRATE STRUCTURAL FRAMING EXCEPT AS ACCEPTED BY ARCHITECT.

9. FOLLOW MANUFACTURERS' INSTRUCTIONS EXCEPT WHERE MORE STRINGENT REQUIREMENTS ARE INDICATED OR REQUIRED BY CODE.

10. COORDINATE SIZES AND LOCATIONS OF OPENINGS AND ROUGH-INS FOR MECHANICAL EQUIPMENT WITH MECHANICAL CONTRACTOR, BEFORE PROCEEDING WITH THE WORK.

11. ALL PIPE, CONDUIT, DUCTS AND STRUCTURE TO BE CONCEALED BEHIND FINISHED CONSTRUCTION UNLESS NOTED AS EXPOSED TO VIEW ON DRAWINGS OR APPROVED BY ARCHITECT.

12. OFFSET AND ADJUST NON-LOAD BEARING FRAMING AS NECESSARY TO MAKE FINISHED SURFACES FLUSH AND TRUE TO LINE.

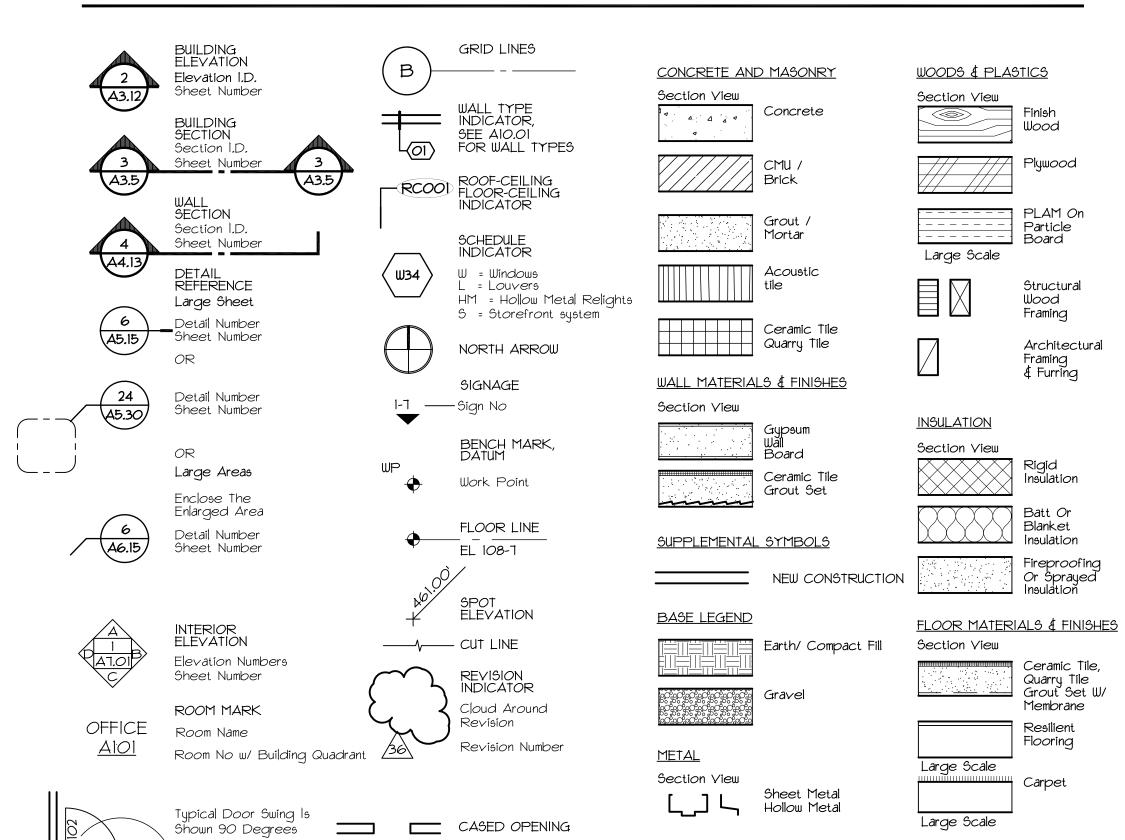
 VERIFY BLOCKING & BACKING REQUIREMENTS WITH EQUIPMENT SUPPLIER & CASEWORK FABRICATOR; INSTALL BLOCKING & BACKING AT ALL WALL-MOUNTED AND WALL-SUPPORTED ITEMS, TYPICAL

14. NOTES INCLUDING "AS REQUIRED" INDICATE AS REQUIRED BY LOCAL JURISDICTION, VERIFY REQUIREMENTS PRIOR TO BID AND BEGINNING THE WORK.

15. EXIT SIGNS AND EMERGENCY ILLUMINATION SHALL INDICATE PATH OF EGRESS TRAVEL.

GRAPHIC SYMBOLS

THESE SYMBOLS ARE FOR STANDARD REFERENCE ONLY. NOT ALL SYSTEMS AND ASSEMBLIES LISTED OCCUR IN THESE DOCUMENTS



AIR BARRIER TESTING

PROVIDE COMPLETE AIR BARRIERS & AIR BARRIER TESTING

Opening Number

THESE ABBREVIATIONS ARE FOR STANDARD REFERENCE ONLY. ABBREVIATIONS NOT ALL SYSTEMS AND ASSEMBLIES LISTED OCCUR IN THESE DOCUMENTS

AB A/C	Anchor Bolt, Acoustical Core Board Asphalt Concrete	Н Н НВ НС	Hinge/ High Hose Bibb	5 5 5 { R	South/ Shelf Shelf and Rod
ACP ACT	Acoustical Ceiling Panel Acoustical Ceiling Tile Area Drain	HCT	Handicap/ Hollow Core Hollow Clay Tile Hot & Cold Water/Hollow Core Wood	SAN	Sanîtary Seat Cover Dispenser Schedule
ADJ	Area Drain Adjustable/ Adjacent	HCW HDR	Hot & Cold Water/Hollow Core Wood Header	SCD SCHED	Scale Cover Dispenser
AFF AHU	Adjustable/ Adjacent Above Finished Floor Air Handling Unit	HDWD HM	Hardwood Hollow Metal	SCW SD	Solid Core Wood Soap Dispenser
L/ALUM	Ajuminum	HORIZ HP	Horizontal	SECT SE	Section Square Feet Special Floor Coating
LT O	Alternate/ Alteration Automatic Door Opener	HR HW	Horsepower/ High Point/ Heat Pump Hour/ Handrai Hot Water	SFC SHT	Sheet
P PPROX	Access Panel/ Apron Panel Approximate	HWH HWR	Hot Water Hot Water Heater Hot Water Return	SHTG SMS	Sheathing Sheet Metal Screws
RCH VE	Architect/ Architectural Avenue	HWS HWS	Hot Water Return Hot Water Supply	SND SNT	Sanitary Napkin Dispenser Sealant
WP	Acoustic Wall Panel	1		SNW SPEC	Sanitary Napkin Waste Receptacle Specification
3		DEZ	Inside Diameter/ Inside Dimension Invert Elevation	5Q	Sauare
OC	Bottom of Curb	iñ Insul	Inches Insulation	55 55T	Solid Surfacing Stainless Steel
BLDG BLKG	Building Blocking	INT IWR	Interior Indirect waste receptor	STC STD	Sound Tranemission Class Standard
SM SOT/BTM		Ĵ	mandet waste receptor	STL STN	Steel Stain
STWN SUR	Between Built-Up Roof	JAN JT	Janitor	STOR STRUCT SUBFL	Storage Structure/ Structural Sub floor
3OW	Bottom of Wall	K	Joint	SUBFL SURF	Sub-floor Surface
:		K	Kip(s) Kiln Dried	SUSP SWC	Suspended Special Wall Coating
3 3B	Catch Basin/ Chalk Board Cementitious Backer Board	KD KIP	Kiin Dried 1000 pounds		Special wall coating
3C O	California Building Code Ceiling Diffuser, Cup Dispenser	KIT/ KITCH	Kitchen_	† †	Ton/ Threshold/ Tread/ Toilet
<i>C</i> -	Control Chared	KO KP	Knock Out Kick Plate	†4B †4G	Top & Bottom Tongue & Groove
HRL HRL I IP	Corner Guard Coat Hook/ Channel Chair Rail Cast kon	L		TBB	Top/ Threshold/ Tread/ Toilet Top & Bottom Tongue & Groove Towel Bar/ Tack Board Tile Backer Board
P	Cast Iron Cast In Place Control Joint / Construction Joint	L LAV	Left/ Length/Ladder Lavatory	TD TD	Terracotta Towel Dispenser & Waste
	Center Line	LAV LB LF	Pound	TEL	rejeptione
.G .R	Ceiling Clear, Clearance	LH	Linear Foot Left Hand Left Hand Royers	TEMP TER	Tembered/ Temporary/ Temperature Terrazzo
MU	Concrete Masonry Units	LHR LL LP	Left Hand Reverse Live Load	TO TOC	Top of Top of Curb/ Top of Concrete
O ONC	Clean Out Concrete	LP	Low Point	TOD	Top of Deck
ONT	Continuous	М		TOF TOIL	Top of Framing Toilet
PT T	Carpet Ceramic Tile/ Changing Table	MAS	Maeonry	TOP TOS	Top of Plate / Pavement Top of Steel
TR TSK	Center	MAX MB	Maximum Machine Bolt / Marker Board	TOW TP	Top of Wall Telephone Panel Board
J	Countersunk Cubic	MDF	Medium Density Fiberboard	TPD TV	Telephone Panel Board Toilet Paper Dispenser Television
Ŋ	Cold Water	MECH MEJ	Mechanical	TVB TYP	Television/ VCR Bracket Typical
		MEMB MTL/MET	Masonry Expansion Joint Membrane Metal	и	
	penny (naile) decibel	MFR MH	Manufacturer / Manufacturing Manhole	u/C	Under Counter
) Д ЗL	Dental Air, Dental Air Compressor Double	MIN MISC	Minimum/ Minute Miscellaneous	UL UNO	Underwriters Laboraties Unless Noted Otherwise
EPT	Department	MNPT	Male National Pipe Thread	UTIL	Utility
A .	Detail Drinking Fountain/ Douglas Fir	MO MTD	Masonry Opening Mounted	Y	
AG	Drinking Fountain/ Douglas Fir Diameter Diagonal Drilled In Concrete Anchor	MUTCD	Manual of Uniform Traffic Control Devices	VCT VERT	Vinyl Composition Tile Vertical
CA M_	Drilled In Concrete Anchor Dimension	N N	North	VP	Veneer Plaster
5P -	Dimension Disposal Dead Load	NA NIC	North Not Applicable Not In Contract	W	
N R	Down Door/ Drain	NO./# NOM	Number Nominal	W	West/ Water/ C othes Washer/ WATT Without
R S SP	Downspout Dry Stand Pipe	NTS	Not To Scale	W/O	
Ŵ	Downspout Dry Stand Pipe Dental Vacuum Dishwasher Drawing Drawer	0		W/ WD	With Wood
WG WR	Drawing Drawer	OI	Over	WF WH	Wīde Flange Water Heater
		OC OD OFD OFF	Over On Center Outside Diameter Overflow Drain	WP WR	Work Point/ Waterproof Waste Receptacle, Waste Receiver (drair Washington State Building Code
	E <i>a</i> st	OFD OFF	Office	WSBC WT	Washington State Building Code
A B F G D	Each	OH	Opposite Hand	WWF WWM	Weight Welded Wire Fabric Welded Wire Mesh
: 1	Expansion Bolt Exhaust Fan Exhaust Grille	OPNG OPP OSB	Opening Opposite Oriented Strand Board		welded wife flesh
JD J	Exhaust Grille Electric Hand Dryer Expansion Joint Elevation Electrical Elevator	OZ	Ounce	Y YD	Yard Drain/ Yard
FC.	Elevation Electrical	_		10	Tara Diani, Tara
EC EV	Elevator	P P	Paint		
TR)	Entrance Electrical Panelboard/ End Panel Equal/ Earth Quake Earth Quake Joint Equipment Existing To Remain Electric Water Cooler	PA	Public Address	SUPPLE	EMENTAL ABBREVIATIONS
Q QJ QUID	Earth Quake Joint Four formers	PC PCD	Precast Paper Cup Dispenser		
	Existing To Remain	PERF PERP	Perforated	a~~~~	N G LIGED
P	Electric Water Cooler Expansion	PH PL	Perpendicular Phase Plate / Plastic Laminate		PLS USED BREVIATIONS
st(E)/ (IST	Existing	PLAM PLAS	Plastic Laminate Plastic Laminate Plaster	43 ADI	And
(T	Exterior	PLYWD PNL	Flaster Plywood Pânel	_	Angle
	Eabranhait	PNT	Paint	a ⊊	At Centerline
, AD	Fahrenheit Fire Alarm/ Forced Air Fire Alarm Angusciator Rand	PREFÍN PSF	Prefinished Pound Per Square Foot	С	Channel
A AAP ACP APB	Fair erine of Fair Fire Alarm / Forced Air Fire Alarm Annunciator Panel Fire Alarm Control Panel Fire Alarm Pull Box Flat Bar	P61		# d	Number Penny
3	rire Ajarm Mulj Box Fjat Bar	PSI PSL PT	Parallel Strand Lumber Preservative Treated/Paint/Point	/	Per Per
))C	Floor Drain Fire Department Connection	PTD PTDW	Pounds per Square Inch Parallel Strand Lumber Preservative Treated/Paint/Point Paper Towel Dispenser PTD & Waste Receptacle	卍	Property Line/ Plate
C	Fire Extinguisher	PVC	Polyvinyl Chloride	<u>.</u> O	Plus or Minus Round
ΞE	Fire Extinguisher Fire Extinguisher Cabinet Finish Floor Elevation Flat Head	Q		Ф	Square/ Square Feet
1 1C 1Mg	Fire Hose Cabinet	QT QTR	Quarry Tíle Quarter		
IC IMS ISMS	Fire Hose Cabínet Fiat Head Machíne Screw Fiat Head Sheet Metal Screw Fiat Head Wood Screw			ARRDE	EVIATED STRUCTURAL SHAPES
1W5 N	Finish	R R	Pieer / Pelocato	С	American Standard Channel
	Floor	R45, R/5 RA	Ríser / Relocate Rod (§ Sheli Roturn Air	HHS L	American Standard Channel Hollow Structural Shape Angle M Shapes Miscellaneous Channel
UOR IPT OB	Fluorescent Female National Pipe Thread Face Of Brick Face Of Concrete Face Of Finish Furn Burgung Postalled by GC	RAD	Return Air Radius	M MC	i i Snapes Miscellaneous Channel
DB DC DF	Face Of Concrete	RB RCP	Rubber Base Reflected Ceiling Plan	PL W	Plate W Shapes
OF OIC OIO	Furn, by Owner Installed by GC	RD	Roof Drain	~	ш о. фо
ΝV	Furn, by Owner Installed by GC Furn, by Owner Installed by Owner Furn, by Owner Installed by Vendor Face of Masonry	REC REF	Recessed Reference/ Refrigerator		
OM OS	Face of Studs	REINF REQD	Reinforced, Reinforcing Bar Required	ABBRF	EVIATED TOILET ACCESSORIES
	Full Size/ Floor Sink Foot or Feet	RES RET	Redilled Refilent Retaining/ Return	CH GB	Coat Hook
IC IT	Furn, by Tenant Installed by GC Furn, by Tenant Installed by Tenant	REV RF	Revision Rubber Flooring, Resilient Flooring	GB PCD PTD	Grab Bar Paper Cup Dispenser Paper Towel Dispenser
II RN (C	turnace/ turnish(ed)	RFG	Robe Hook/ Round Head/ Right Hand	PTDW	Paner Toulel Diabenser & Illaste Recenta
	Furnished by Vendor, Installed by Contractor	RM RO	KOOM	SD SND	Soap Dish/Soap Dispenser Sanitary Napkin Dispenser
: A	Gauge	ROW RP	Rough Opening Right Of Way Radius Point	SNW TB	Soap Dîsh/Soap Dîspenser Sanîtary Napkin Dîspenser Sanîtary Napkin Waste Receptacle Towel Bar
3B	Gauge Gypsum Backer Board General Contractor	RPBA	Reduced Pressure Backflow Assembly	TPD TSCD	Toilet Paper Dispenser Toilet Seat Cover Dispenser
	CASTISTOL COULT OCCUT	RRL	Rub Rail		
iFl iL	Ground Fault Interrupter Glazing / Glue Laminated Wood	RSF RWL	Resilient sheet flooring Rain Water Leader	WR	Waste Receptacle

PROJECT DIRECTORY

ARCHITECT: EL,ARCHITECTS PS CONTACT: ERIC LANCIAULT 3200 SE 164th AVE STE 302 VANCOUVER, WA 98683 TEL: 360.798.3801 EMAIL: eric@elaooa.com

SEPARATE PERMITS

FIRE SPRINKLER FIRE ALARM

DEFERRED SUBMITTALS

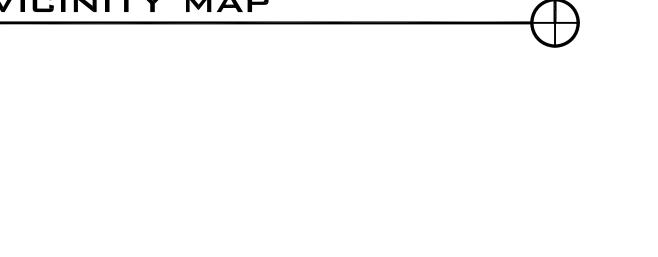
PRE-ENGINEERED METAL BUILDING

Gypsum Wall Board

SPECIAL INSPECTIONS

PER 1704.2. OWNER SHALL EMPLOY ONE OF MORE APPROVED AGENCIES TO PROVIDE SPECIAL INSPECTIONS AND TESTS DURING CONSTRUCTION ON WORK INDICATED IN SPECIAL INSPECTION PROGRAM AND REQUIREMENTS; ALL SPECIAL INSPECTION AND TEST REPORTS SHALL BE SUBMITTED TO THE AHJ AND ARCHITECT

VICINITY MAP



PROJECT DATA

SCOPE OF WORK: CONSTRUCTION OF NEW EVENT CENTER INCLUDING ASSEMBLY SPACE, COMMERCIAL KITCHEN, (2) SHORT-STAY UNITS & SUPPORT SPACES

GOVERNING CODES:

GOVERNING CODES:	
BUILDING CODE WITH WA STATE	2018 INTERNATIONAL BUILDING CODE AMENDMENTS (WAC51-50)
PLUMBING CODE	2018 UNIFORM PLUMBING CODE AND STANDARDS WITH WA STATE AMENDMENTS (WAC 51-56)
MECHANICAL CODE	2018 INTERNATIONAL MECHANICAL COI WITH WA STATE AMENDMENTS (WAC 51-5
ELECTRICAL CODE (NFPA70)	2020 NATIONAL ELECTRIC CODE WITH WA STATE AMENDMENTS
FIRE CODE	2018 INTERNATIONAL FIRE CODE WITH WA STATE AMENDMENTS (51-54A)
ENERGY CODE	2018 INTERNATIONAL ENERGY CONSERVATION CODE WITH WA STATE AMENDMENTS
FIRE ALARM SYSTEM	NFPA 12 NATIONAL FIRE ALARM AND SIGNALING STANDARD CURRENT EDITIO
FIRE SPRINKLER SYSTEM	NFPA 13 STANDARD FOR INSTALLATION OF SPRINKLER SYSTEMS CURRENT EDITION
JURISDICTION:	STEVENSON, WASHINGTON
BUILDING AREA:	xxxx SF
CONSTRUCTION TYPE:	II-B
OCCUPANCY GROUP:	A-3, S-1, B, R-2
NO. OF STORIES:	2
OCCUPANT LOAD:	xx PERSONS
SPRINKLER SYSTEM:	YES
FIRE ALARM SYSTEM:	YES

SHEET INDEX

NO. OF STORIES:

COVER SHEET	A0.01
MAIN LEVEL FLOOR PLAN	A2.01
UPPER LEVEL FLOOR PLAN	A2.02
ROOF PLAN	A2.O3
MAIN LEVEL EQUIPMENT LAYOUT	A2.04
EXTERIOR ELEVATIONS & BUILDING SECTIONS	A2.05
MAIN LEVEL REFLECTED CEILING PLAN	A8.01
UPPER LEVEL REFLECTED CEILING PLAN	A8.02

ARCHITECTS, 3200 SE 164TH

> VANCOUVER, WASHINGTON v. 360.798.3801 **ATHAY &**

AVENUE SUITE 302

ASSOCIATES, INC 411NE 83RD STREET

VANCOUVER, WASHINGTON 98665 v. 360.574.0199

PARACLETE PS, INC

7510 NE VANCOUVER MALL DRIVE VANCOUVER, WASHINGTON 98662 v. 360.254.9234



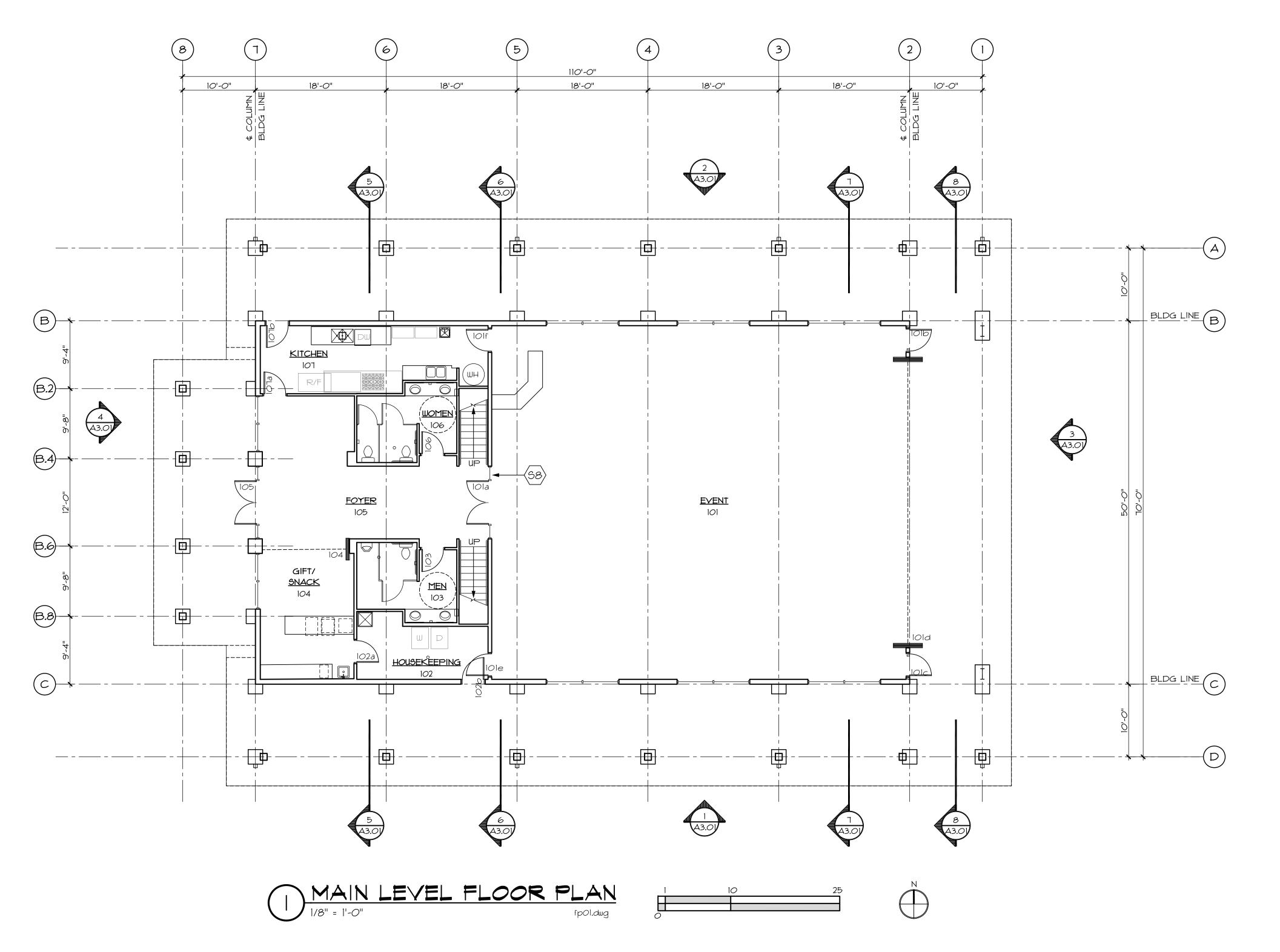
EXPIRES: 01.20.24 SIGNED: xx.xx.xx

REVISIONS DATE

STEVENSON **EVENT CENTER**

ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

COVER SHEET



EL,A

EL, ARCHITECTS, PS

3200 SE 164TH
AVENUE SUITE 302
VANCOUVER,WASHINGTON
98683
V. 360.798.3801

ATHAY & ASSOCIATES, INC

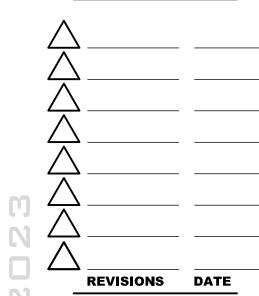
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STEVENSON EVENT CENTER

ISSUE DATE: 04.04.2023
JOB NUMBER: 1036

MAIN LEVEL FLOOR PLAN

A2.01

76

110'-0" 10'-0" 18'-0" 18'-0" 18'-0" 18'-0" A3.01 HVAC DECK BLDG LINE (B) PLENUM 4 A3.01 UPPER LEVEL FLOOR PLAN
1/8" = 1'-0"

fp02.dug

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VANCOUVER,WASHINGTON
98683
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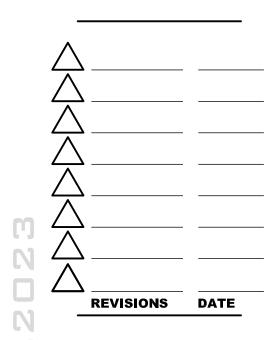
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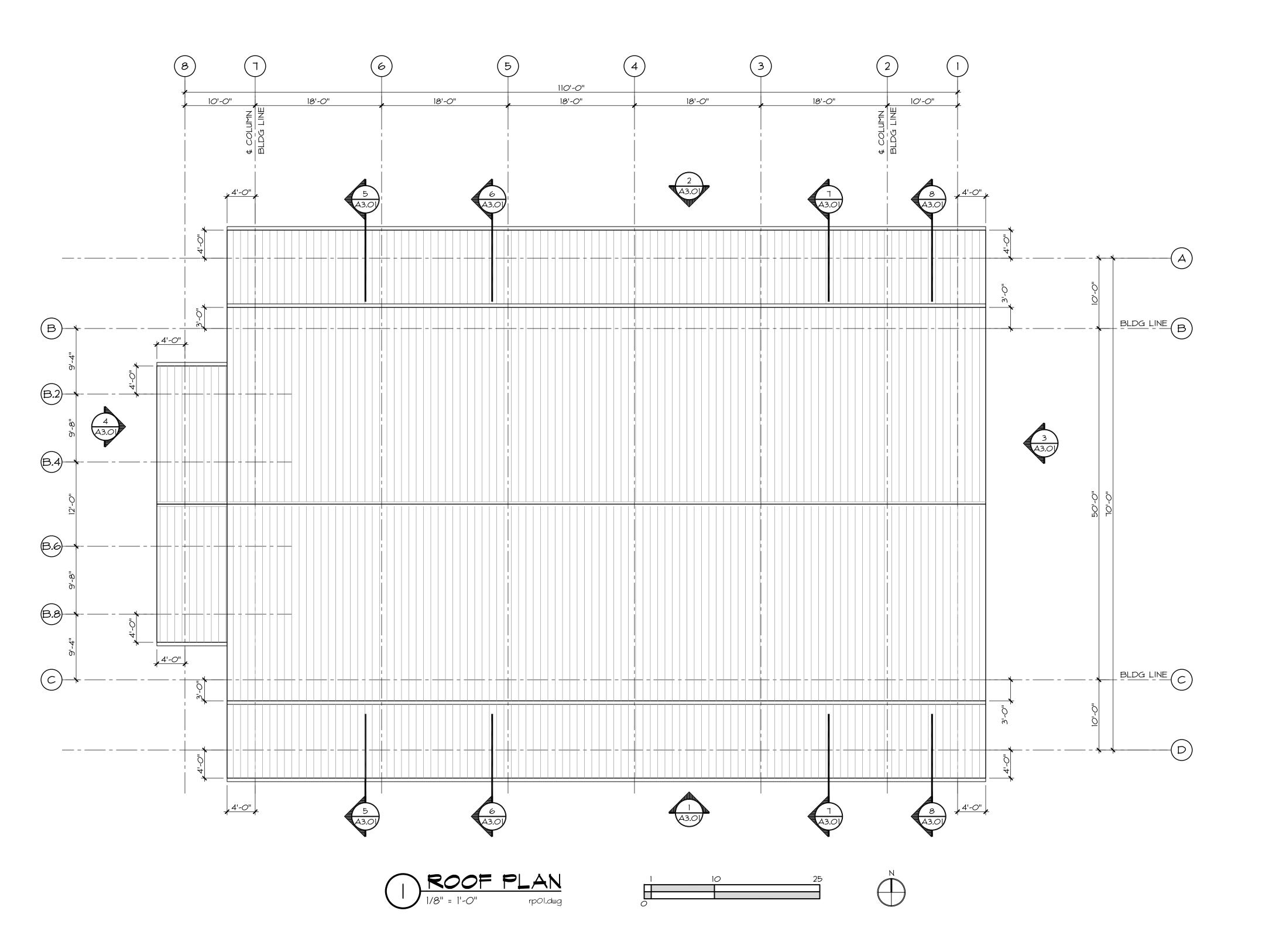
ISSUE DATE: 04.0 JOB NUMBER: 10

ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

LEVEL FLOOR PLAN

[□] A2.02

77





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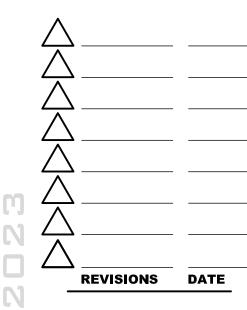
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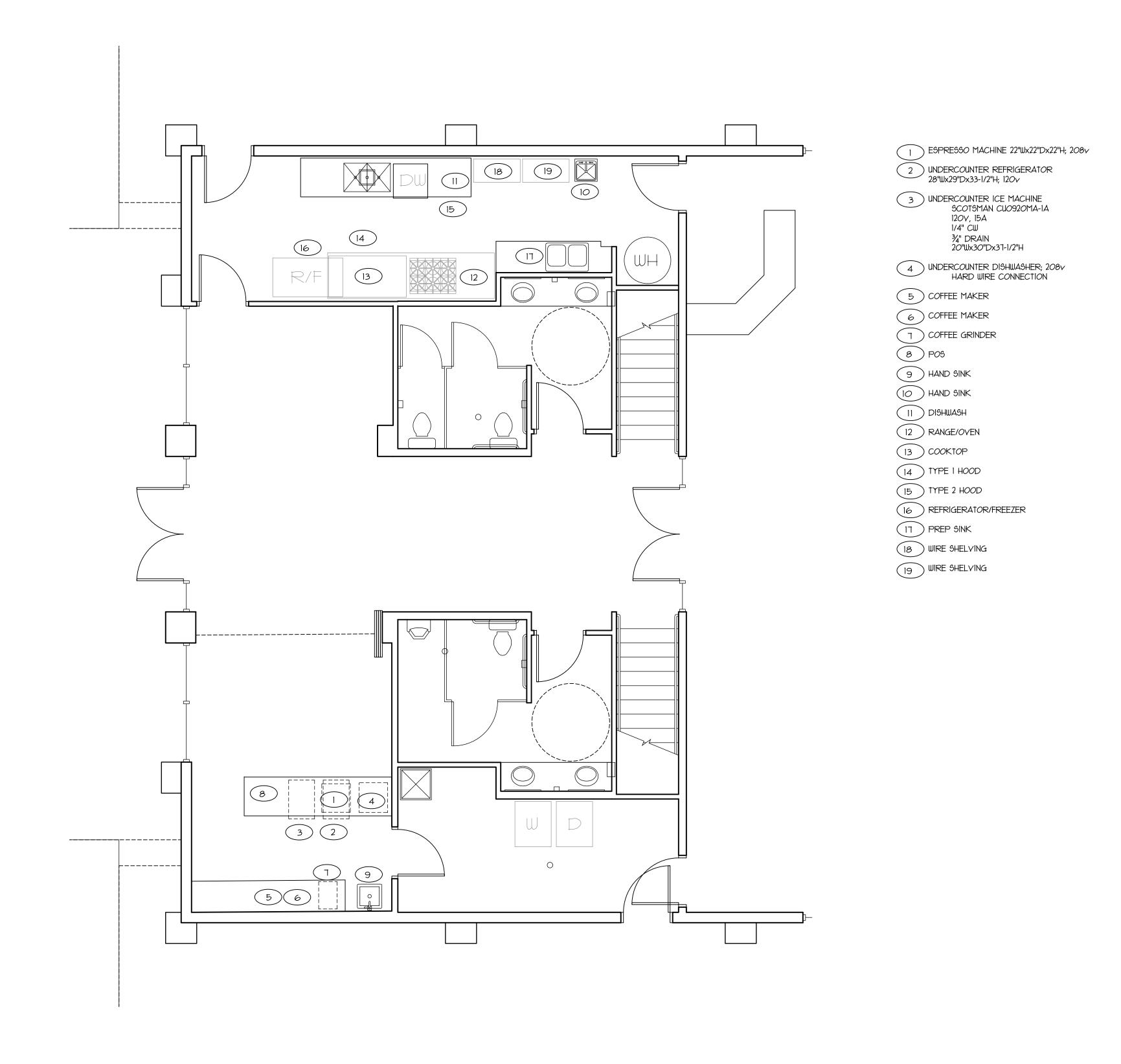


STEVENSON **EVENT CENTER**

ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

ROOF PLAN

A2.03







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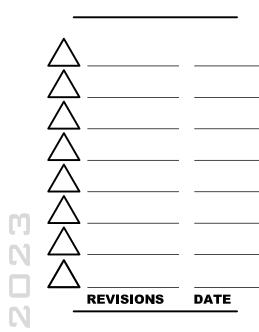
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PARACLETE
PS, INC
7510 NE VANCOUVER

7510 NE VANCOUVER
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WASHINGTON 98662
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STEVENSON EVENT CENTER

ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

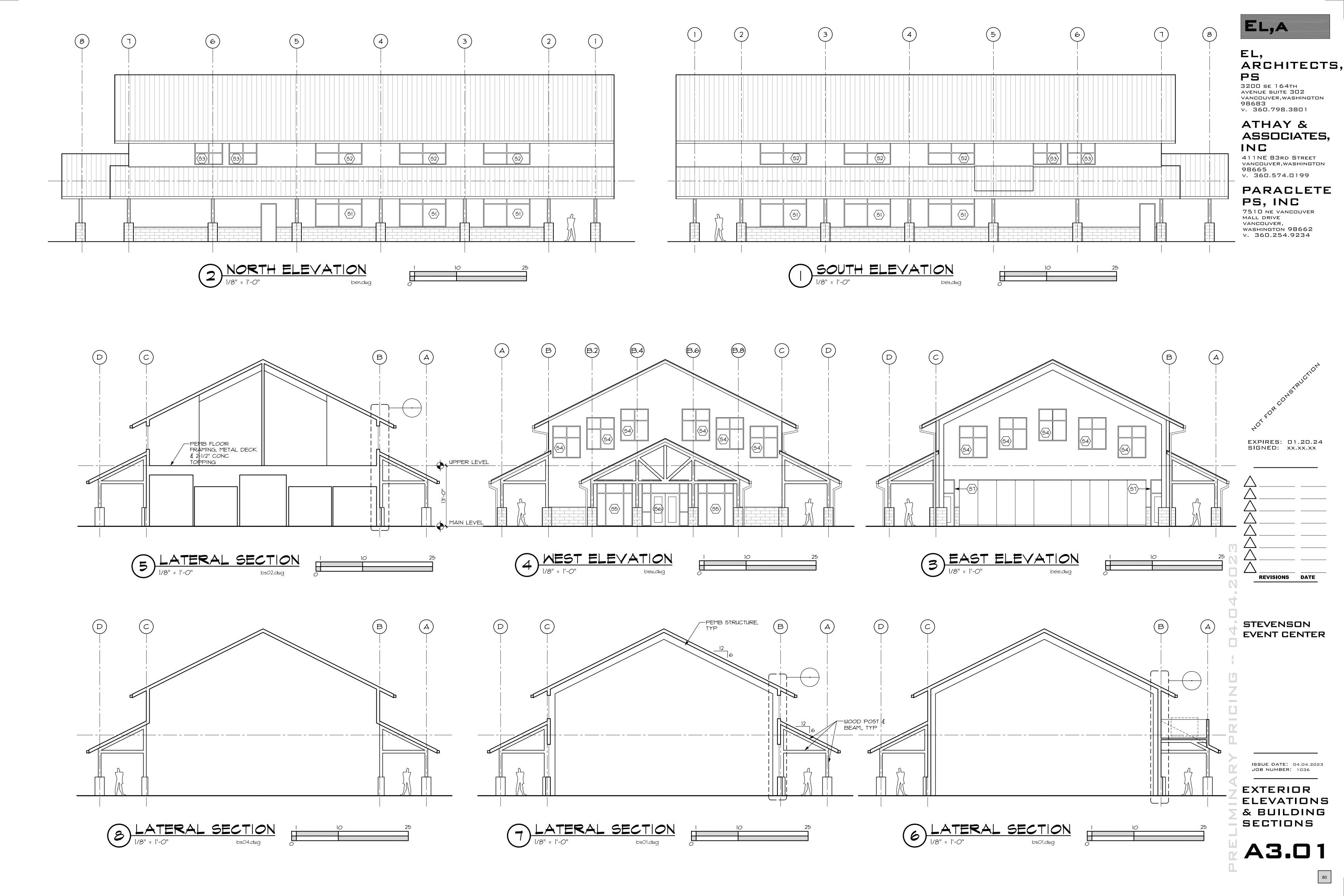
MAIN LEVEL

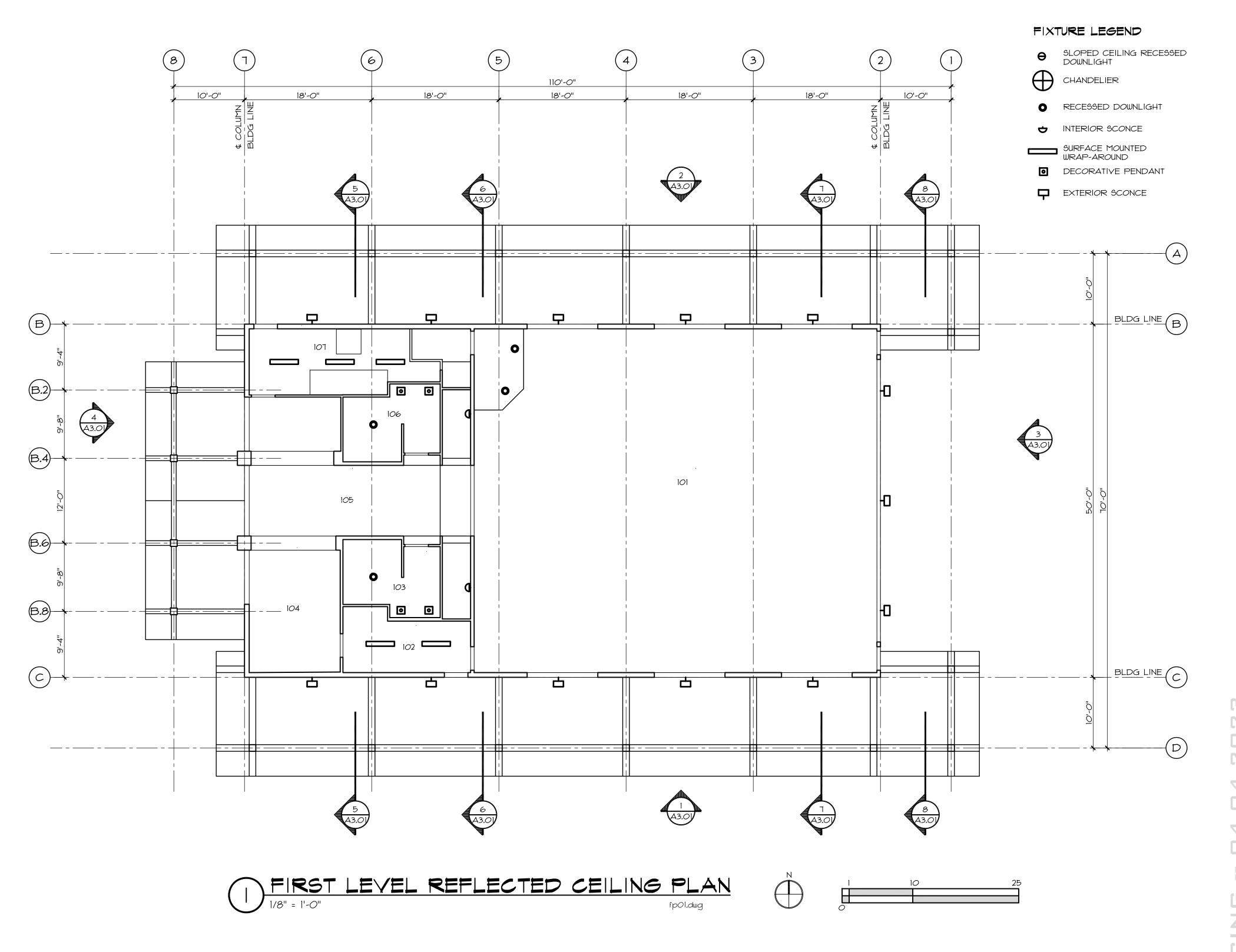
LAYOUT

EQUIPMENT

A2.04

79





EL,A

EL, ARCHITECTS, PS

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98683
V. 360.798.3801

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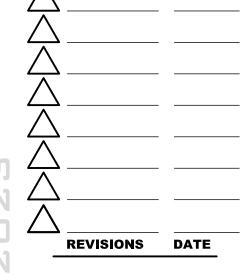
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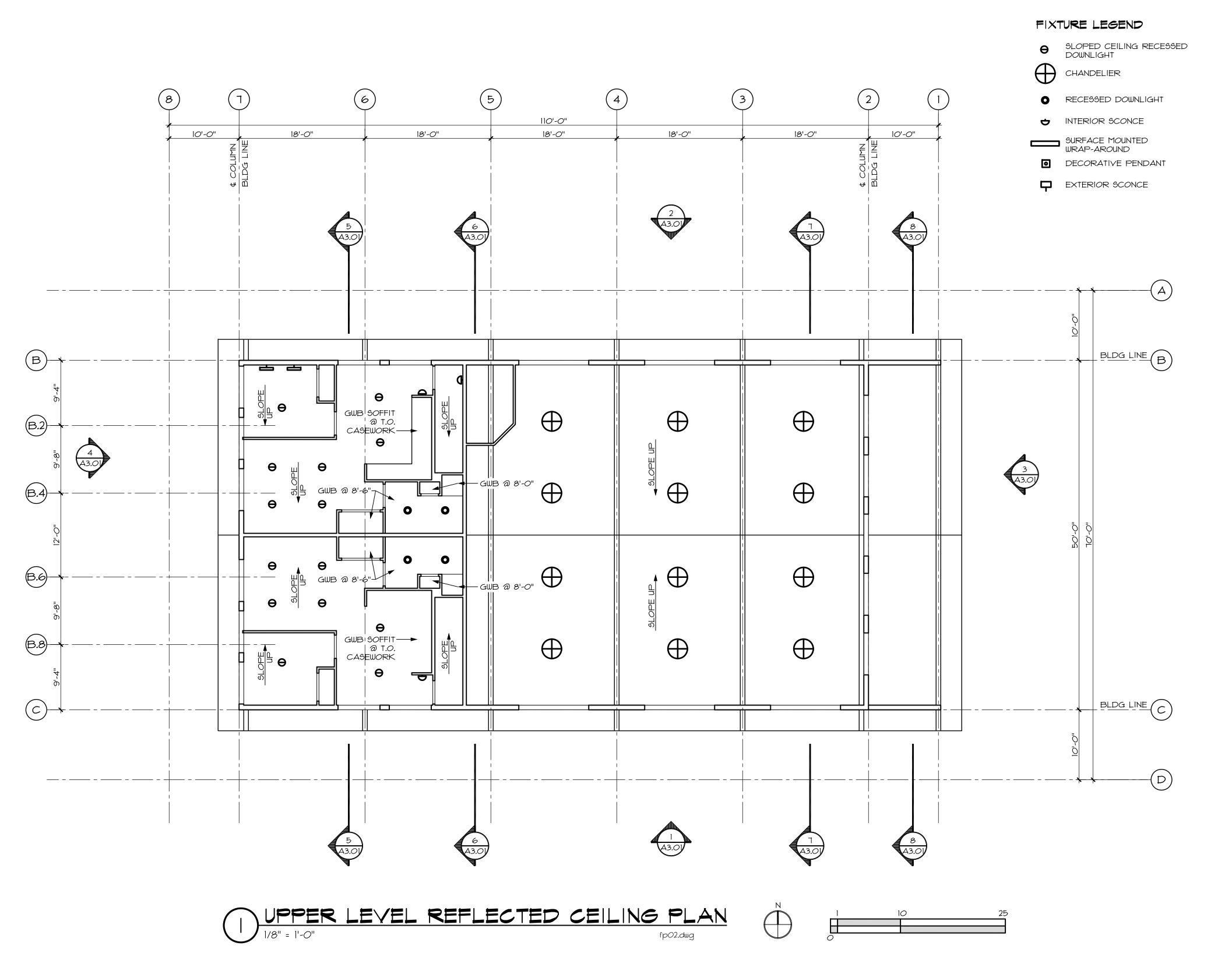


STEVENSON EVENT CENTER

ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

FIRST LEVEL REFLECTED CEILING PLAN

A8.01



EL,A

EL, ARCHITECTS, PS

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ATHAY & ASSOCIATES, INC

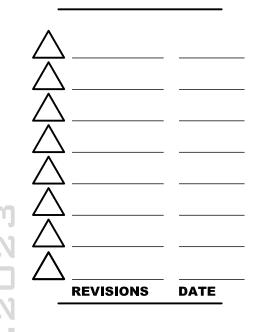
411NE 83RD STREET VANCOUVER,WASHINGTON 98665 V. 360.574.0199

PARACLETE

PS, INC
7510 NE VANCOUVER
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VANCOUVER,
WASHINGTON 98662
V. 360.254.9234



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STEVENSON EVENT CENTER

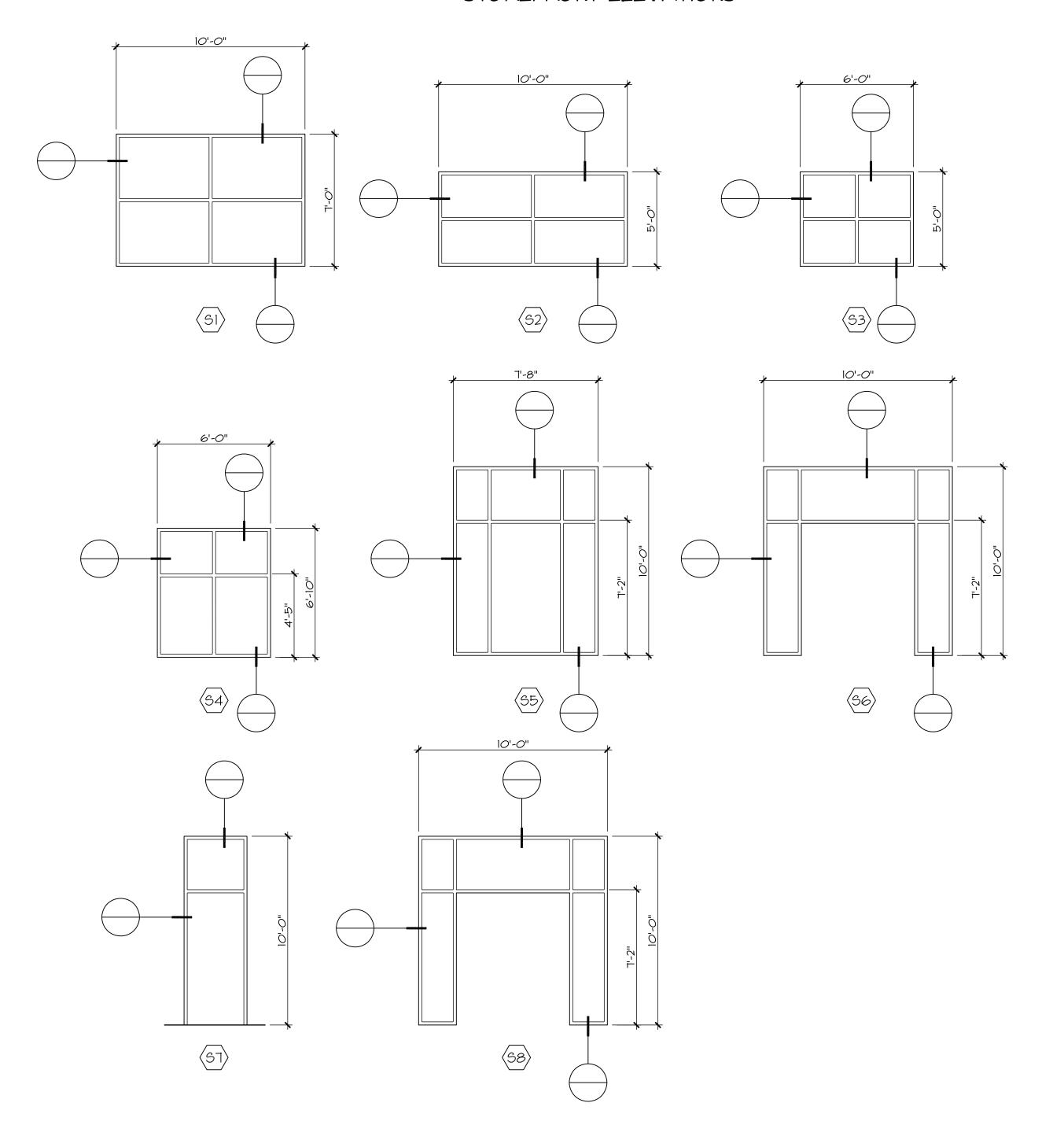
ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

UPPER LEVEL REFLECTED CEILING PLAN

A8.02

82

STOREFRONT ELEVATIONS



ROOM FINISH SCHEDULE

RM			WAL	LS					
NO.	ROOM NAME	N	E	S	W	CLG	FLR	BASE	REMARKS
101	EVENT	-	-	-	-	P-	-	-	-
102	HOUSEKEEPING	-	-	1	-	P-	SC	RB	-
103	MEN	-	-	-	-	P-	-	-	NOTE 2, 3
104	GIFT/SNACK	-	-	-	-	P-	-	-	-
105	FOYER	-	-	-	-	P-	-	-	-
106	WOMEN	-	-	-	-	P-	-	-	NOTE 2, 3
107	KITCHEN	FRP	FRP	FRP	FRP	P-	-	-	-

ROOM FINISH SCHEDULE NOTES:

1. 6" H COVE BASE

2. PLAM WAINSCOT TO 4'-0" AFF,
SEE INTERIOR ELEVATIONS

FRP - FIBER-REINFORCED PLASTIC PANEL LYT - LUXURY VINYL FLOORING =, P- - GWB, PAINT RB - 4" RESILIENT BASE

RB - 4" RESILIENT BASE
RSF - RESILIENT SHEET FLOORING
SC - SEALED CONCRETE

DOOR SCHEDULE

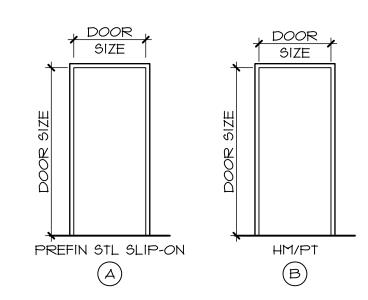
DOOR NO.	DOOR SIZE W x H	FRAME TYPE	DOOR TYPE	HEAD DETA	JAMB AILS A	1	GLAZ- ING	REMARKS
101a	6'-0" x 7'-0"	-	4	-	-	-	GL-1	NOTE 7
1016	3'-0" × 7'-0"	-	4	-	-	-	GL-2	NOTE 7
101c	3'-0" x 7'-0"	-	4	-	-	-	GL-2	NOTE 7
101d	40'-0" × 10'-0"	-	3	-	-	-	GL-2	-
101e	3'-0" x 7'-0"	А	1	-	-	-	-	-
101f	3'-0" × 7'-0"	А	1	-	-	-	-	-
102a	3'-0" × 7'-0"	А	1	-	-	-	-	-
1026	3'-0" × 7'-0"	В	2	-	-	-		-
103	3'-0" x T'-0"	А	1	-	-	-	-	-
104	12'-0" × 10'-0"	-	3	-	-	-	-	-
105	6'-0" x 7'-0"	-	4	-	-	-	-	NOTE 7
106	3'-0" x T'-0"	А	1	-	-	-	-	-
107a	3'-0" x T'-0"	А	1	-	-	-	-	-
1076	3'-0" x 7'-0"	В	2	-	-	-	-	-

GL-I SAFETY GLAZING ALUM ALUMINUM PT PAINT STN STAIN GL-2 INSULATED SAFETY GLAZING HM HOLLOW METAL STL STEEL WD WOOD

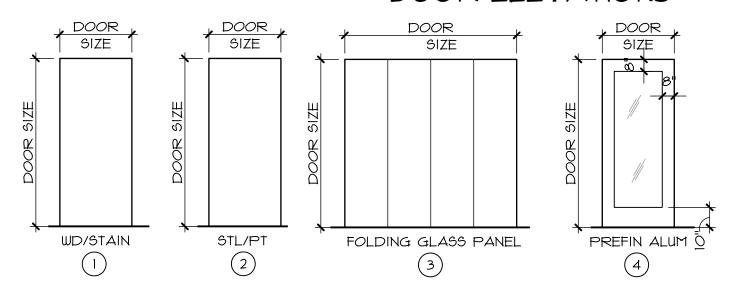
DOOR SCHEDULE NOTES:

- I. SEE SECTION 08 71 00 FOR HARDWARE, TYP
- 2. ALL DOORS: NO MORE THAN 5LBF DOOR OPENING FORCE, TYP
- 3. ALL EXIT DOORS SHALL BE OPENABLE FROM THE INSIDE WITHOUT THE USE OF A KEY OR ANY SPECIAL KNOWLEDGE OR EFFORT; FIELD VERIFY & MODIFY AS REQUIRED
- 4. LOCATE ALL MANUALLY OPERATING HARDWARE WITHIN INDICATED HEIGHT
- 5. ALL DOOR HARDWARE OPERABLE WITH ONE HAND AND DOES NOT REQUIRE TIGHT GRASPING, PINCHING OR TWISTING OF THE WRIST
- 6. ADJUST ALL DOORS WITH CLOSERS SO THAT FROM AN OPEN POSITION OF 90 DEGREES, THE TIME REQUIRED TO MOVE THE DOOR TO A POSITION OF 12 DEGREES FROM THE LATCH IS MINIMUM 5 SECONDS
- 7. SEE STOREFRONT ELEVATIONS

FRAME ELEVATIONS



DOOR ELEVATIONS



EL,A

EL, ARCHITECTS, PS

3200 se 164th
AVENUE SUITE 302
VANCOUVER,WASHINGTON
98683
V. 360.798.3801

ATHAY & ASSOCIATES, INC

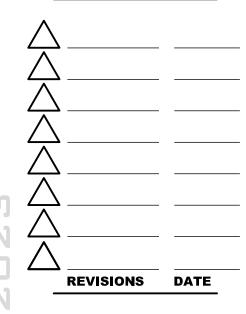
411NE 83RD STREET VANCOUVER,WASHINGTON 98665 V. 360.574.0199

PARACLETE PS, INC

7510 NE VANCOUVER
MALL DRIVE
VANCOUVER,
WASHINGTON 98662
V. 360.254.9234



EXPIRES: 01.20.24 SIGNED: xx.xx.xx



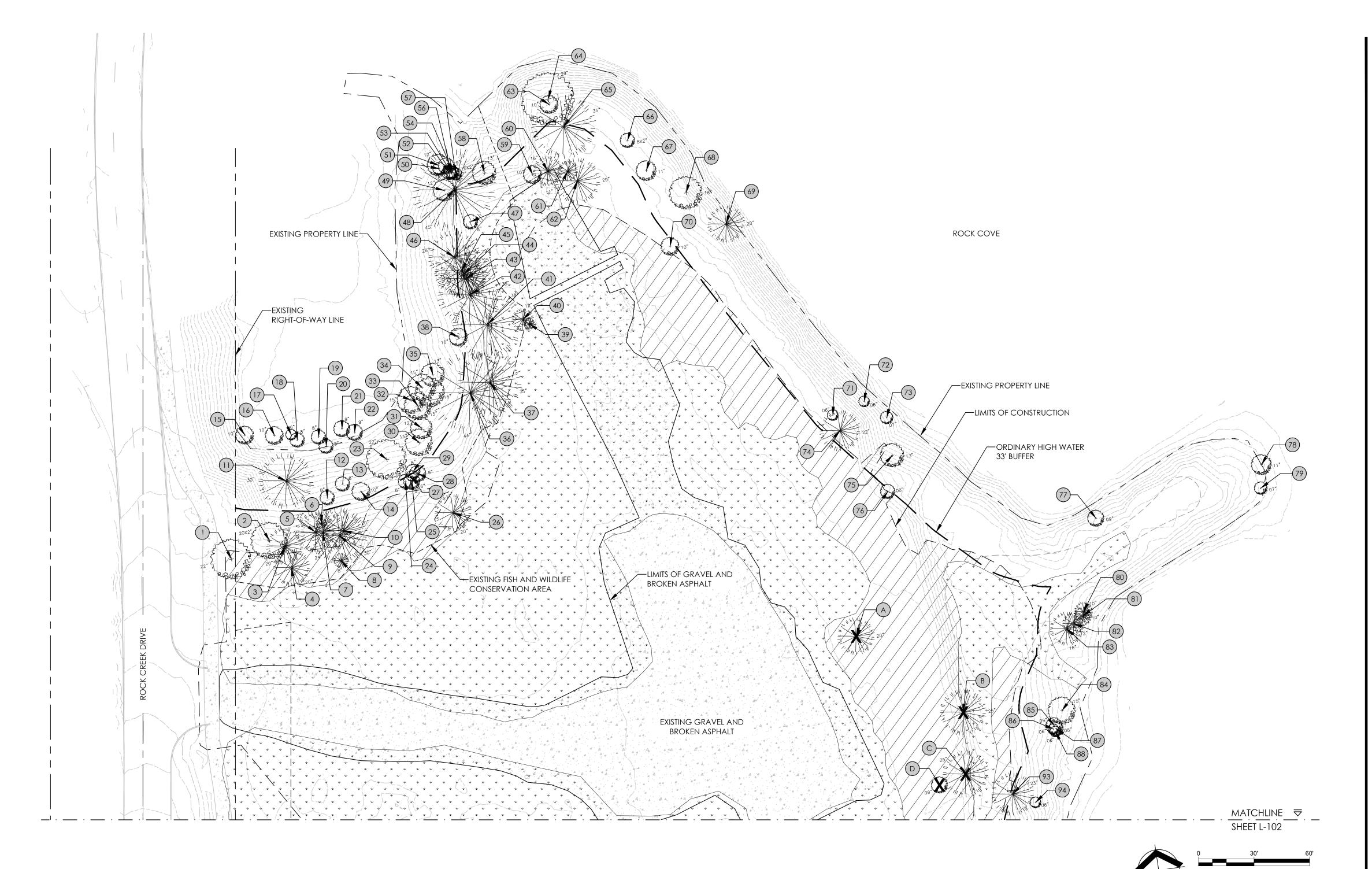
STEVENSON EVENT CENTER

ISSUE DATE: 04.0

ISSUE DATE: 04.04.2023 JOB NUMBER: 1036

SCHEDULES





EXISTING TREES TO REMAIN

TREE NUMBER	SPECIES	SIZE (DBH)
1	NORWAY MAPLE	22"
2	DOUGLAS FIR	20X2''
3	DOUGLAS FIR	20"
4	DOUGLAS FIR	20"
5	DOUGLAS FIR	22"
6	DOUGLAS FIR	6"
7	DOUGLAS FIR	24"
8	DOUGLAS FIR	8"
9	DOUGLAS FIR	30''
10	DOUGLAS FIR	20''
11	DOUGLAS FIR	30''
12	RED ALDER	8"
13	RED ALDER	8"
14	RED ALDER	10"
15	RED ALDER	10"
16	RED ALDER	10"
17	RED ALDER	6"
18	RED ALDER	8"
19	RED ALDER	8"
20	RED ALDER	8"
21	RED ALDER	9"
22	RED ALDER	9"
23	BLACK HAWTHORNE	22"
24	RED ALDER	8"
25	BLACK HAWTHORNE	9"
26	DOUGLAS FIR	20"
27	BLACK HAWTHORNE	6"
28	BLACK HAWTHORNE	9''
29	RED ALDER	8''
30	BIG LEAF MAPLE	15"

TREE NUMBER	SPECIES	SIZE (DBH)
31	RED ALDER	12"
32	RED ALDER	15"
33	RED ALDER	15"
34	RED ALDER	15"
35	RED ALDER	13"
36	DOUGLAS FIR	44''
37	DOUGLAS FIR	35"
38	BIG LEAF MAPLE	10''
39	DOUGLAS FIR	6"
40	DOUGLAS FIR	12"
41	DOUGLAS FIR	38"
42	DOUGLAS FIR	35"
43	DOUGLAS FIR	22"
44	DOUGLAS FIR	25"
45	DOUGLAS FIR	29"
46	DOUGLAS FIR	28"
47	BLACK HAWTHORNE	8"
48	DOUGLAS FIR	45"
49	RED ALDER	12"
50	RED ALDER	7"
51	RED ALDER	12"
52	RED ALDER	8"
53	RED ALDER	6x2"
54	RED ALDER	7''
56	RED ALDER	8"
57	RED ALDER	7''
58	BIG LEAF MAPLE	13"
59	BITTER CHERRY	10"
60	DOUGLAS FIR	16"
61	DOUGLAS FIR	11"

TREE NUMBER	SPECIES	SIZE (DBH)
62	DOUGLAS FIR	25"
63	BIG LEAF MAPLE	10"
64	BIG LEAF MAPLE	29"
65	DOUGLAS FIR	35"
66	RED ALDER	8x2"
67	RED ALDER	11"
68	RED ALDER	18"
69	DOUG FIR	20"
70	RED ALDER	10"
71	CASCARA	6"
72	RED ALDER	6"
73	RED ALDER	7''
74	DOUGLAS FIR	22"
75	BITTER CHERRY	13"
76	RED ALDER	8"
77	RED ALDER	9"
78	RED ALDER	11"
79	RED ALDER	7''
80	DOUGLAS FIR	10"
81	DOUGLAS FIR	10''
82	DOUGLAS FIR	12"
83	DOUGLAS FIR	18"
84	RED ALDER	15"
85	RED ALDER	9"
86	RED ALDER	6"
87	RED ALDER	8"
88	RED ALDER	6"
93	DOUGLAS FIR	23"
94	BIG LEAF MAPLE	6"

TREES PROPOSED FOR REMOVAL

TREE NUMBER	SPECIES	SIZE (DBH
Α	DOUGLAS FIR	20''
В	DOUGLAS FIR	25"
С	DOUGLAS FIR	25"
D	BITTER CHERRY	9"

LEGEND

EXISTING DECIDUOUS TREE

EXISTING CONIFEROUS TREE

X EXISTING TREE TO BE REMOVED EXISTING OPEN GRASSY AREAS

EXISTING HIMALAYAN BLACKBERRY AREAS TO BE REMOVED

NOTES:

1. THE EXISTING VEGETATION AT ROCK CREEK COVE IS PREDOMINANTLY A MIXED

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- 2. THE EXISTING TREE SPECIES PRESENT INCLUDE DOUGLAS FIR, BLACK COTTONWOOD, BIG LEAF MAPLE, RED ALDER, BITTER CHERRY, BLACK HAWTHORNE, BLACK LOCUST, AND A FEW OREGON WHITE OAKS.
- 3. THE RIPARIAN UNDERSTORY CONSISTS OF A MIX OF NOOTKA ROSE, SWAMP ROSE, OCEAN SPRAY, SERVICEBERRY, OREGON GRAPE, AND LADY FERN. THE RIPARIAN GROUNDCOVER IS COMPRISED MOSTLY OF THE INVASIVE HIMALAYAN BLACKBERRY AND COMMON VETCH THAT HAS SPREAD THROUGHOUT NEARLY ALL OF THE VEGETATED AREAS.
- 4. THE OPEN GRASSY AREAS IN THE INTERIOR OF THE SITE INCLUDE A MIX OF NATIVE AND NONNATIVE GRASSES, PATCHES OF YOUNG COTTONWOODS, WHITE CLOVER, COMMON CHICORY, MEADOW HAWKWEED, SCOTCH BROOM, COMMON HORSETAIL, COMMON VETCH, THISTLE, QUEEN ANNE'S LACE, CURLY DOCK, AND HIMALAYAN BLACKBERRY.
- 5. THE VEGETATED AREAS THAT ARE WITHOUT TREE CANOPY AND NOT WITHIN THE GRASSY OPEN AREAS CONSIST OF A DENSE COVER OF COMMON VETCH AND HIMALAYAN BLACKBERRY.
- 6. THE VEGETATED AREAS LOWER IN ELEVATION ADJACENT TO ROCK COVE INCLUDE A MIX OF WILLOWS, COMMON RUSH, SPREADING RUSH, SPIREA, TUFTED HAIRGRASS, YELLOW FLAG IRIS, COMMON VETCH, AND HIMALAYAN BLACKBERRY.
- 7. A TOTAL OF 25 TREES WILL BE IMPACTED AND REMOVED DURING CONSTRUCTION. 15 OF THE TREES TO BE REMOVED HAVE A DBH GREATER THAN OR EQUAL TO 12".
- 8. A TOTAL OF 40 TREES WILL BE REPLANTED. REFER TO SHEETS L201 L203 FOR THEIR SPECIES, SIZE, AND
- 9. THE EXISTING OPEN GRASSY AREAS CONSIST OF 95,485 SF. ALL OF THE GRASSY AREAS WILL BE IMPACTED DURING CONSTRUCTION AND WILL BE REPLANTED WITH A TOTAL OF 500 SHRUBS, 45 ORNAMENTAL GRASSES, 411 PERENNIALS, 209 GROUNDCOVERS, AND 64,677 SF OF RESEEDED GRASS AREAS. REFER TO SHEETS L201 - L203 FOR SPECIES, SIZE, AND LOCATION OF THE PROPOSED PLANTINGS.
- 10. THE PROPOSED CONSTRUCTION WILL REMOVE 61,067 SF OF HIMALAYAN BLACKBERRY AND COMMON

EXISTING	TREES	TO	REMA

TREE NUMBER	SPECIES	SIZE (DBH)
95	RED ALDER	8"
96	RED ALDER	6''
97	DOUGLAS FIR	13"
99	DOUGLAS FIR	14"
100	DOUGLAS FIR	11"
101	DOUGLAS FIR	19"
112	DOUGLAS FIR	13"
114	DOUGLAS FIR	20''
115	DOUGLAS FIR	18"
116	PACIFIC DOGWOOD	9"
117	OREGON ASH	10"
118	RED ALDER	11"
119	DOUGLAS FIR	9"
120	RED ALDER	10"
121	WESTERN SERVICEBERRY	10"
122	WESTERN SERVICEBERRY	12"
123	DOUGLAS FIR	7''
124	DOUGLAS FIR	7''
125	WESTERN SERVICEBERRY	10"
126	OREGON WHITE OAK	15"
127	OREGON WHITE OAK	14"
128	OREGON WHITE OAK	21"
129	DOUGLAS FIR	12"
130	BITTER CHERRY	11"
131	RED ALDER	12"
132	BITTER CHERRY	10''
133	BITTER CHERRY	14''
136	BLACK LOCUST	6"
137	BLACK LOCUST	17''
138	BLACK LOCUST	12"
139	BLACK LOCUST	12"
140	BLACK LOCUST	8"
141	BLACK LOCUST	6"

TREE NUMBER	SPECIES	SIZE (DBH)	
142	BLACK LOCUST	7''	
143	BLACK LOCUST	7''	
144	BLACK LOCUST	13"	
145	BLACK LOCUST	11"	
146	BLACK LOCUST	6"	
147	BLACK LOCUST	6"	
148	BLACK LOCUST	7''	
149	BLACK LOCUST	6"	
150	BLACK LOCUST	12"	
151	BLACK LOCUST	12"	
152	BLACK LOCUST	6"	
153	BLACK LOCUST	6"	
154	BLACK LOCUST	8"	
155	BLACK LOCUST	17"	
156	BLACK LOCUST		
157	DOUGLAS FIR	18"	
158	BITTER CHERRY	12"	
159	BITTER CHERRY	8"	
160	BLACK LOCUST	11"	
161	COTTONWOOD	20"	
162	COTTONWOOD	6"	
163	COTTONWOOD	6"	
164	RED ALDER	18"	
165	RED ALDER	13"	
166	RED ALDER	15"	
167	BLACK LOCUST	8"	
168	BLACK LOCUST	11"	
169	BLACK LOCUST	7''	
170	BLACK LOCUST	10"	
171	BLACK LOCUST	7''	
172	BLACK LOCUST 10"		
173	BLACK LOCUST	8"	
174	BLACK LOCUST	10"	
175	BLACK LOCUST	9''	

TREE NUMBER	SPECIES	SIZE (DBH)
176	BLACK LOCUST	11"
177	BLACK LOCUST	6"
178	BLACK LOCUST	9''
179	BLACK LOCUST	10"
180	BLACK LOCUST	12"
181	BLACK LOCUST	7''
182	BLACK LOCUST	12"
183	BLACK LOCUST	11"
184	BLACK LOCUST	7''
185	BLACK LOCUST	10"
186	BLACK LOCUST	11"
187	RED ALDER	10"
188	RED ALDER	10"
189	RED ALDER	8"
190	DOUGLAS FIR	25"
191	DOUGLAS FIR	6"
192	DOUGLAS FIR	12"
193	DOUGLAS FIR	14"
194	DOUGLAS FIR	14"
195	DOUGLAS FIR	18"
196	DOUGLAS FIR	25"
197	DOUGLAS FIR	14"
198	DOUGLAS FIR	8"
199	DOUGLAS FIR	8"
200	DOUGLAS FIR	13"
201	DOUGLAS FIR	14"
202	DOUGLAS FIR	9''
203	OREGON WHITE OAK	6''
204	DOUGLAS FIR	22"
205	205 DOUGLAS FIR	
206	DOUGLAS FIR	23"
207	Douglas Fir	22"

TREES PROPOSED FOR REMOVAL

TREE NUMBER	SPECIES	SIZE (DBH)
E	COTTONWOOD	28"
F	BIG LEAF MAPLE	9''
G	BIG LEAF MAPLE	10"
Н	BIG LEAF MAPLE	6"
I	DOUGLAS FIR	13"
J	DOUGLAS FIR	12"
K	CASCARA	7''
L	BIG LEAF MAPLE	7''
М	DOUGLAS FIR	18"
N	DOUGLAS FIR	24"
0	DOUGLAS FIR	6"
Р	DOUGLAS FIR	7''
Q	DOUGLAS FIR	12"
R	DOUGLAS FIR	14"
S	DOUGLAS FIR	14"
T	BIG LEAF MAPLE	6"
U	DOUGLAS FIR	19"
V	DOUGLAS FIR	15"
W	COTTONWOOD	12"
Х	COTTONWOOD	17''
Y	COTTONWOOD	9''

LEGEND

EXISTING DECIDUOUS TREE

EXISTING CONIFEROUS TREE

X EXISTING TREE TO BE REMOVED

EXISTING OPEN GRASSY AREAS

EXISTING HIMALAYAN BLACKBERRY AREAS TO BE REMOVED

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- 10. THE PROPOSED CONSTRUCTION WILL REMOVE 61,067 SF OF HIMALAYAN BLACKBERRY AND COMMON VETCH.

SEAL:

EXPIRES 04/09,

STATE OF

WASHINGTON

RESISTERED

LANDSCAPE ARD TELT

LIMITAL LIMITAL

CURTIS ALAN RICEY

CONSULTANT:

HOSPITALITY

PROJECT NUN ROCK CREEK COVE HOSP

T PLAN
SOUTH

NOTE: BAR IS ONE INCH ON ORIGINAL DRAWING. IF NOT ONE INCH ON THIS SHEET, ADJUST SCALE ACCORDINGLY.

SETATION ASSESSMI

L-102



- 1. CORRESPONDING SPECIFICATION SECTION(S) HAVE NOT BEEN DEVELOPED FOR THIS PROJECT. ALL PERTINENT INFORMATION HAS BEEN PROVIDED ON THESE PLAN SHEETS. ENSURE THAT ALL RELATED ARCHITECTURAL AND CIVIL SHEETS ARE THOROUGHLY REVIEWED AS REFERENCE TO INFORMATION ON THE LANDSCAPE AND IRRIGATION SHEETS.
- 2. VERIFY LOCATION OF ALL EXISTING AND PROPOSED UTILITIES EITHER ABOVE OR BELOW GRADE PRIOR TO BEGINNING ANY WORK. COORDINATE WITH ALL OTHER CONTRACTORS TO AVOID CONFLICTS BETWEEN IRRIGATION EQUIPMENT AND TREE/SHRUB PLACEMENT, AND/OR OTHER SITE AMENITIES.
- 3. VERIFY THAT SUB GRADE PREPARATION HAS BEEN COMPLETED TO ACCEPTABLE TOLERANCES PRIOR TO BEGINNING ANY WORK.
- 4. ALL WORK COMPLETED SHALL BE GUARANTEED FOR ONE (1) FULL YEAR.
- 5. LANDSCAPED AREAS TO RECEIVE ?" OF APPROVED TOPSOIL IN LAWN AREAS AND ?" IN ALL SHRUB PLANTING AREAS.
- 6. ACCEPTABLE TOPSOIL SHALL BE EITHER TAKEN FROM 'ON-SITE' STRIPPED AND STOCKPILED TOPSOIL, OR IMPORTED FROM LOCAL SUPPLIERS, AND SHALL BE FRIABLE SURFACE SOIL TAKEN ONLY FROM THE 'A' HORIZON AS DETERMINED BY THE USDA SOIL CONSERVATION SERVICE; FREE FROM TOXIC MATERIAL, NOXIOUS WEED SEEDS, SUBSOIL, STONES AND OTHER DEBRIS GREATER THAN 1" DIAMETER. MAXIMUM ELECTRICAL CONDUCTIVITY: 2.0 MILLIOHMS; MAXIMUM EXCHANGEABLE SODIUM PERCENTAGE: 10%; PH: 5.5 TO 6.5; ORGANIC CONTENT: 5 -7%. PROVIDE THREE SEPARATE SOIL SAMPLES (TAKEN FROM THREE DIFFERENT LOCATIONS WITHIN THE SOIL SOURCE). SUBMIT TEST RESULTS FOR REVIEW AND APPROVAL BY THE OWNER/LANDSCAPE ARCHITECT PRIOR TO INSTALLATION.
- 7. ALL LANDSCAPE AREAS SHALL BE IRRIGATED BY AN AUTOMATIC IRRIGATION SYSTEM SEE IRRIGATION NOTES/SEE SHEET ?.
- 8. COORDINATE WITH GENERAL CONTRACTOR FOR THE PROTECTION AND WATERING OF EXISTING PLANT MATERIAL IDENTIFIED AS 'TO REMAIN' UNTIL THE NEW IRRIGATION SYSTEM IS
- 9. LAWN AREAS SHALL BE EDGED AS INDICATED IN DETAIL?, SHEET?.
- 10. SHRUB PLANTING AREAS SHALL BE MULCHED WITH 3" OF APPROVED ? UNLESS OTHERWISE NOTED. GROUND COVER AREAS SHALL BE DRESSED WITH 1" 2" OF APPROVED ? UNLESS OTHERWISE NOTED. FINISHED GRADE OF MULCH SHALL NOT BE ABOVE OR MORE THAN 1" BELOW ADJOINING SURFACES.
- 11. LAWN AREAS SHALL BE SEEDED/SODDED WITH LOCALLY PRODUCED SEED MIX CONSISTING OF A MINIMUM THREE VARIETIES OF BLUE GRASS SEED, AND AN ANNUAL RYE GRASS. HYDRO-SEED AT A RATE PER MANUFACTURER'S RECOMMENDATIONS.
- 12. PLANT SYMBOLS SHALL DICTATE COUNT.

TREES	CODE	BOTANICAL / COMMON NAME	CONT	CAL.	<u>HEIGHT</u>		<u>Q</u> T
	ACV	ACER CIRCINATUM / VINE MAPLE	MULTI-STEM		5` MIN		6
	AJ	ACER PALMATUM 'JEDDELOH ORANGE' / JEDDELOH ORANGE JAPANESE MAPLE	15 GAL				2
	AB	ACER RUBRUM 'BRANDYWINE' / BRANDYWINE RED MAPLE	B & B	2.0" CAL			7
	AX	ACER TRUNCATUM X PLATANOIDES 'WARRENRED' / PACIFIC SUNSET® MAPLE	B & B	2.0" CAL			2
	CNP	CHAMAECYPARIS NOOTKATENSIS 'PENDULA' / WEEPING NOOTKA FALSE CYPRESS	B & B		8` HT.		1
	CG	CHAMAECYPARIS OBTUSA 'GRACILIS' / SLENDER HINOKI FALSE CYPRESS	15 GAL				7
	CJ	CORNUS CONTROVERSA 'JUNE SNOW-JFS' / JUNE SNOW™ GIANT DOGWOOD	B & B	2.0" CAL			1
	MY	MAGNOLIA ACUMINATA 'YELLOW BIRD' / YELLOW BIRD MAGNOLIA	2" CAL				3
The state of the s	MG	METASEQUOIA GLYPTOSTROBOIDES / DAWN REDWOOD	B & B		7` MIN		6
	РО	PINUS NIGRA 'OREGON GREEN' / OREGON GREEN AUSTRIAN PINE	B & B		6` MIN		3
	SG	SEQUOIADENDRON GIGANTEUM 'PENDULUM' / WEEPING GIANT SEQUOIA	B & B		8` MIN		2
SHRUBS	CODE	BOTANICAL / COMMON NAME	SIZE				
\odot	AN	AZALEA X 'MAISCHNEE' / KIMONO MAY SNOW AZALEA	5 GAL				54
	СК	CORNUS SERICEA 'KELSEYI' / KELSEY'S DWARF RED TWIG DOGWOOD	5 GAL				55
52 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	HD	HAMAMELIS X INTERMEDIA 'DIANE' / DIANE WITCH HAZEL	15 GAL				6
\odot	HR	HYDRANGEA QUERCIFOLIA 'RUBY SLIPPERS' / RUBY SLIPPERS OAKLEAF HYDRANGEA	5 GAL				19
+	HS	HYPERICUM X INODORUM 'KOLSAN' / FLORALBERRY® SANGRIA ST. JOHN'S WORT	5 GAL				52
}	IS	ILEX CRENATA 'SKY PENCIL' / SKY PENCIL JAPANESE HOLLY	5 GAL				34
Marie	PC	PINUS MUGO 'VALLEY CUSHION' / VALLEY CUSHION MUGO PINE	5 GAL				14
AND THE PROPERTY OF THE PROPER	PH	PINUS SYLVESTRIS 'HILLSIDE CREEPER' / HILLSIDE CREEPER SCOTCH PINE	5 GAL				28
\odot	RR	RHAMNUS FRANGULA 'RON WILLIAMS' / FINE LINE® ALDER BUCKTHORN	5 GAL				30
(+)	RP	RHODODENDRON X 'POLARNACHT' / POLARNACHT RHODODENDRON	5 GAL				12
(+)	RS	RHODODENDRON X 'SEPTEMBER SONG' / SEPTEMBER SONG RHODODENDRON	5 GAL				15
\odot	RX	RHODODENDRON X 'YELLOW PETTICOATS' / YELLOW PETTICOATS RHODODENDRON	5 GAL				16
	RG	RHUS AROMATICA 'GRO-LOW' / GRO-LOW FRAGRANT SUMAC	5 GAL				38
\odot	ST	SPIRAEA BETULIFOLIA 'TOR' / TOR BIRCHLEAF SPIREA	5 GAL				80
3 Miles	TD	TAXUS X MEDIA 'DARK GREEN SPREADER' / DARK GREEN SPREADER YEW	5 GAL				39
ORNAMENTAL GRASSES	CODE	BOTANICAL / COMMON NAME	SIZE				
*	PN	PENNISETUM ALOPECUROIDES 'GINGER LOVE' / GINGER LOVE FOUNTAIN GRASS	1 GAL				57
THE SHE	SP	STIPA TENUISSIMA 'PONY TAILS' / PONY TAILS MEXICAN FEATHER GRASS	1 GAL				45
PERENNIALS	CODE AC	BOTANICAL / COMMON NAME ATHYRIUM FILIX-FEMINA 'CRISTATUM' / CRESTED LADY FERN	<u>SIZE</u> 1 GAL				34
2 6 3	AF	ATHYRIUM FILIX-FEMINA 'LADY IN RED' / LADY IN RED FERN	1 GAL				31
~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~~	EG	ECHINACEA PURPUREA 'GREEN EYES' / GREEN EYES CONEFLOWER	1 GAL				81
ر چن	GR	GERANIUM X 'ROZANNE' / ROZANNE CRANESBILL	1 GAL				72
₩ (•)	HH						
		HEMEROCALLIS X 'HAPPY RETURNS' / HAPPY RETURNS DAYLILY	1 GAL				75
*	LM	LIRIOPE MUSCARI 'MONROE'S WHITE' / MONROE'S WHITE LILYTURF	1 GAL				193
<b>*</b>	LS	LIRIOPE MUSCARI 'SILVER DRAGON' / SILVER DRAGON LILYTURF	1 GAL				185
*	PM	POLYSTICHUM MUNITUM / WESTERN SWORD FERN	1 GAL				98
GROUND COVERS	CODE	BOTANICAL / COMMON NAME	CONT				
	AU	ARCTOSTAPHYLOS UVA-URSI / KINNIKINNICK	1 GAL			36" o.c.	335
	GO	GALIUM ODORATUM / SWEET WOODRUFF	1 GAL			36" o.c.	191
KXXXX							

18" HIGH SEAT WALL

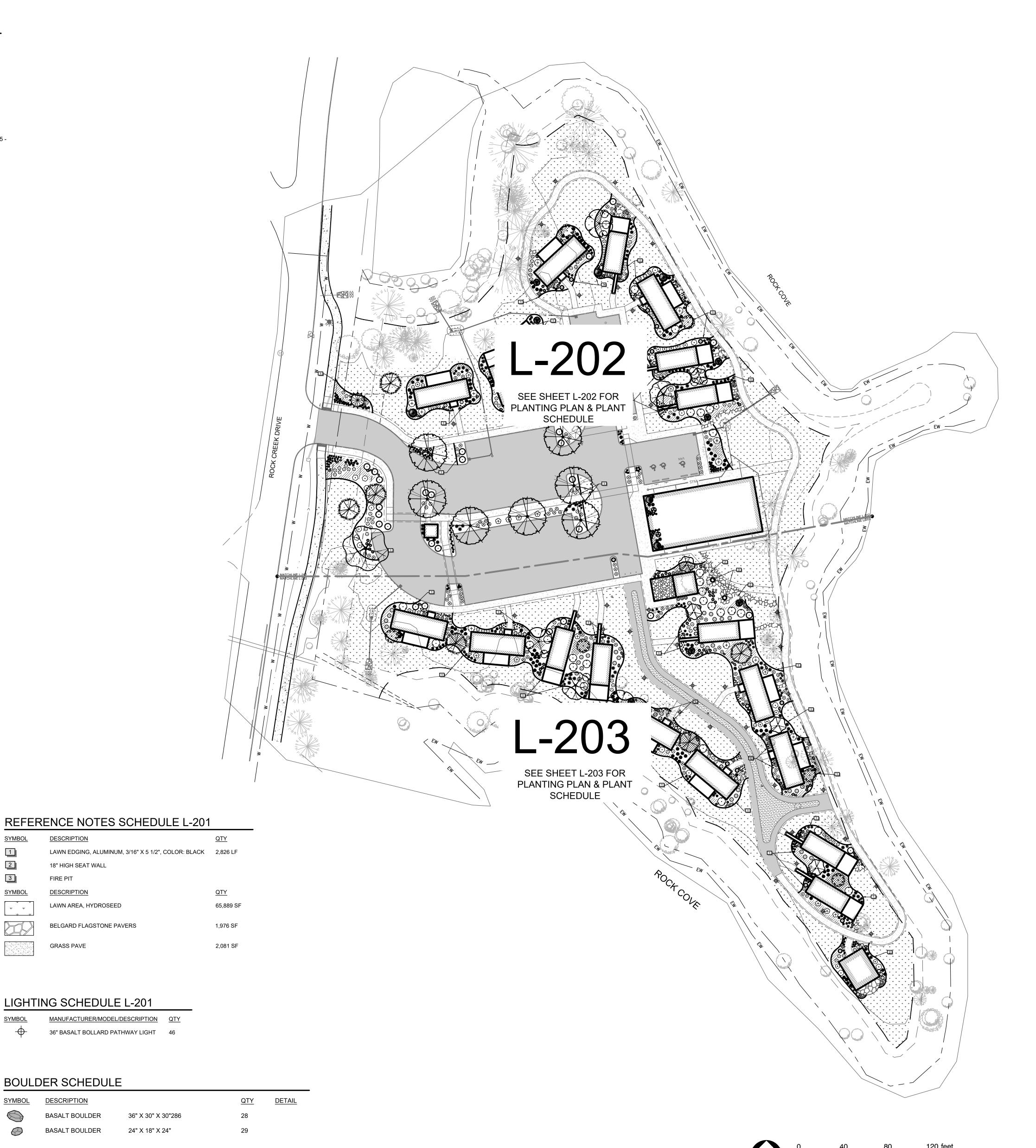
LIGHTING SCHEDULE L-201

**BOULDER SCHEDULE** 

BASALT BOULDER

BASALT BOULDER

SYMBOL DESCRIPTION



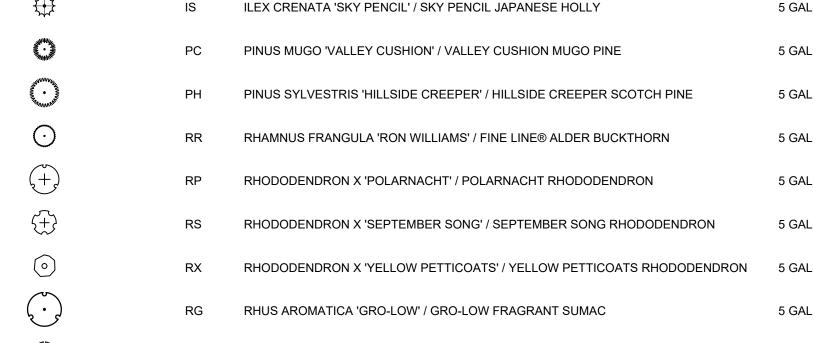
1908 W NORTHWEST BLVD, STE A SPOKANE, WA 99205 509.325.0511

<u>PL</u>	ANT SCHED	ULE	L-202				
TREE	<u> </u>	CODE	BOTANICAL / COMMON NAME	CONT	CAL.	<u>HEIGHT</u>	<u>Q1</u>
		ACV	ACER CIRCINATUM / VINE MAPLE	MULTI-STEM		5` MIN	2
		AJ	ACER PALMATUM 'JEDDELOH ORANGE' / JEDDELOH ORANGE JAPANESE MAPLE	15 GAL			2
	$\supset$	АВ	ACER RUBRUM 'BRANDYWINE' / BRANDYWINE RED MAPLE	В&В	2.0" CAL		7
		AX	ACER TRUNCATUM X PLATANOIDES 'WARRENRED' / PACIFIC SUNSET® MAPLE	B & B	2.0" CAL		1
		CNP	CHAMAECYPARIS NOOTKATENSIS 'PENDULA' / WEEPING NOOTKA FALSE CYPRESS	B & B		8` HT.	1
	•	CG	CHAMAECYPARIS OBTUSA 'GRACILIS' / SLENDER HINOKI FALSE CYPRESS	15 GAL			4
		CJ	CORNUS CONTROVERSA 'JUNE SNOW-JFS' / JUNE SNOW™ GIANT DOGWOOD	B & B	2.0" CAL		1
July Market	upa _{le}	MY	MAGNOLIA ACUMINATA 'YELLOW BIRD' / YELLOW BIRD MAGNOLIA	2" CAL			1

CODE	BOTANICAL / COMMON NAME	CONT	CAL.	<u>HEIGHT</u>	QTY
ACV	ACER CIRCINATUM / VINE MAPLE	MULTI-STEM		5` MIN	2
AJ	ACER PALMATUM 'JEDDELOH ORANGE' / JEDDELOH ORANGE JAPANESE MAPLE	15 GAL			2
AB	ACER RUBRUM 'BRANDYWINE' / BRANDYWINE RED MAPLE	B & B	2.0" CAL		7
AX	ACER TRUNCATUM X PLATANOIDES 'WARRENRED' / PACIFIC SUNSET® MAPLE	B & B	2.0" CAL		1
CNP	CHAMAECYPARIS NOOTKATENSIS 'PENDULA' / WEEPING NOOTKA FALSE CYPRESS	B & B		8` HT.	1
CG	CHAMAECYPARIS OBTUSA 'GRACILIS' / SLENDER HINOKI FALSE CYPRESS	15 GAL			4

CNP	CHAMAECYPARIS NOOTKATENSIS 'PENDULA' / WEEPING NOOTKA FALSE CYPRESS	B & B		8` HT.	
CG	CHAMAECYPARIS OBTUSA 'GRACILIS' / SLENDER HINOKI FALSE CYPRESS	15 GAL			
CJ	CORNUS CONTROVERSA 'JUNE SNOW-JFS' / JUNE SNOW™ GIANT DOGWOOD	B & B	2.0" CAL		
MY	MAGNOLIA ACUMINATA 'YELLOW BIRD' / YELLOW BIRD MAGNOLIA	2" CAL			
MG	METASEQUOIA GLYPTOSTROBOIDES / DAWN REDWOOD	B & B		7` MIN	
РО	PINUS NIGRA 'OREGON GREEN' / OREGON GREEN AUSTRIAN PINE	B & B		6` MIN	
SG	SEQUOIADENDRON GIGANTEUM 'PENDULUM' / WEEPING GIANT SEQUOIA	B & B		8' MIN	
0005	POTANICAL / COMMON NAME	0.175			

			2 4 2
SHRUBS	CODE	BOTANICAL / COMMON NAME	SIZE
$\odot$	AN	AZALEA X 'MAISCHNEE' / KIMONO MAY SNOW AZALEA	5 GAL
	СК	CORNUS SERICEA 'KELSEYI' / KELSEY'S DWARF RED TWIG DOGWOOD	5 GAL
5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5 5	HD	HAMAMELIS X INTERMEDIA 'DIANE' / DIANE WITCH HAZEL	15 GAL
$\odot$	HR	HYDRANGEA QUERCIFOLIA 'RUBY SLIPPERS' / RUBY SLIPPERS OAKLEAF HYDRANGEA	5 GAL
+	HS	HYPERICUM X INODORUM 'KOLSAN' / FLORALBERRY® SANGRIA ST. JOHN'S WORT	5 GAL
<del>**</del>	IS	ILEX CRENATA 'SKY PENCIL' / SKY PENCIL JAPANESE HOLLY	5 GAL



•			
<b>(*)</b>	ST	SPIRAEA BETULIFOLIA 'TOR' / TOR BIRCHLEAF SPIREA	5 G
and the same of th	TD	TAXUS X MEDIA 'DARK GREEN SPREADER' / DARK GREEN SPREADER YEW	5 G
ORNAMENTAL GRASSES	S CODE	BOTANICAL / COMMON NAME	SIZ
*	PN	PENNISETUM ALOPECUROIDES 'GINGER LOVE' / GINGER LOVE FOUNTAIN GRASS	1 G
34.4	SP	STIPA TENUISSIMA 'PONY TAILS' / PONY TAILS MEXICAN FEATHER GRASS	1 G
PERENNIALS	CODE	BOTANICAL / COMMON NAME	SIZ
	AC	ATHYRIUM FILIX-FEMINA 'CRISTATUM' / CRESTED LADY FERN	1 G
233	AF	ATHYRIUM FILIX-FEMINA 'LADY IN RED' / LADY IN RED FERN	1 G
	EG	ECHINACEA PURPUREA 'GREEN EYES' / GREEN EYES CONEFLOWER	1 G
د <del>ر</del> یح	CD	OFDANIUM VIDOZANNE! / DOZANNE ODANECDILI	1.0

PERENNIALS	CODE	BOTANICAL / COMMON NAME	SIZ
	AC	ATHYRIUM FILIX-FEMINA 'CRISTATUM' / CRESTED LADY FERN	1 G
	AF	ATHYRIUM FILIX-FEMINA 'LADY IN RED' / LADY IN RED FERN	1 G
*	EG	ECHINACEA PURPUREA 'GREEN EYES' / GREEN EYES CONEFLOWER	1 G
£;3	GR	GERANIUM X 'ROZANNE' / ROZANNE CRANESBILL	1 G
$\odot$	НН	HEMEROCALLIS X 'HAPPY RETURNS' / HAPPY RETURNS DAYLILY	1 G
*	LM	LIRIOPE MUSCARI 'MONROE'S WHITE' / MONROE'S WHITE LILYTURF	1 G
*	LS	LIRIOPE MUSCARI 'SILVER DRAGON' / SILVER DRAGON LILYTURF	1 G
*	PM	POLYSTICHUM MUNITUM / WESTERN SWORD FERN	1 G

36" o.c. 184[≥]

36" o.c.

405	1 141	T GET GIT GIVE MONTOW, WESTERN GWOR
GROUND COVERS	CODE	BOTANICAL / COMMON NAME
	AU	ARCTOSTAPHYLOS UVA-URSI / KINNIKINNIC
	GO	GALIUM ODORATUM / SWEET WOODRUFF

# REFERENCE NOTES SCHEDULE L-202

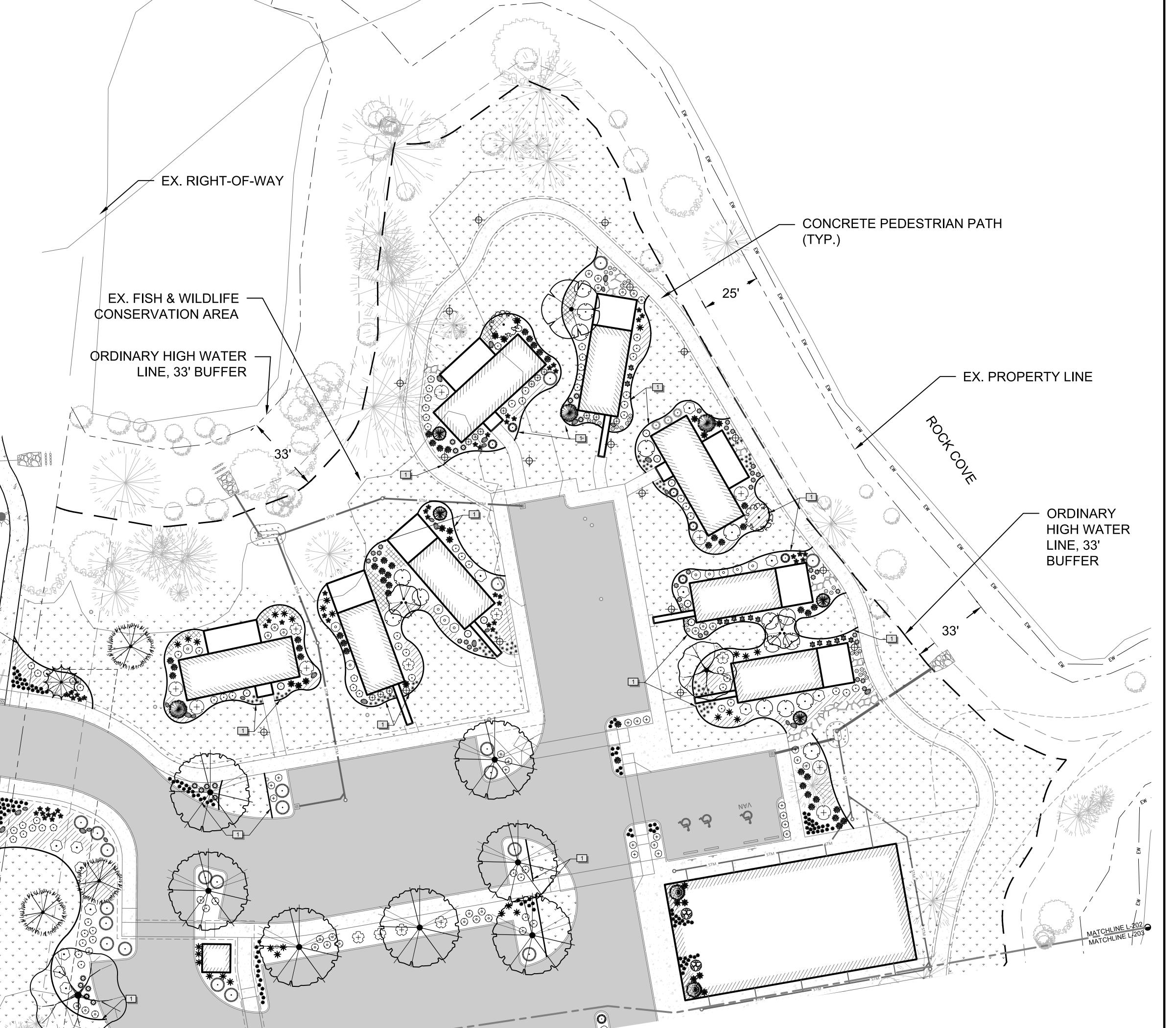
TELLINOL HOTES SOTIED SEE E 202									
SYMBOL	DESCRIPTION	<u>QTY</u>	<u>DETAIL</u>						
1	LAWN EDGING, ALUMINUM, 3/16" X 5 1/2", COLOR: BLACK	1,414 LF							
SYMBOL	DESCRIPTION	QTY	DETAIL						
\ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \ \	LAWN AREA, HYDROSEED	32,540 SF							
	BELGARD FLAGSTONE PAVERS	350 SF							

# LIGHTING SCHEDULE L-202

MANUFACTURER/MODEL/DESCRIPTION QTY 36" BASALT BOLLARD PATHWAY LIGHT 18

# BOULDER SCHEDULE

SYMBOL	DESCRIPTION		QTY	DETAI
	BASALT BOULDER	36" X 30" X 30"286	28	
$\bigcirc$	BASALT BOULDER	24" X 18" X 24"	29	





36" o.c. 151

36" o.c. 152

1 GAL

GO GALIUM ODORATUM / SWEET WOODRUFF

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Rock Creek Cove Hospitality Stevenson, Washington

Prepared For:

FDM Development Inc. 5453 Ridgeline Drive Suite 160 Kennewick, WA 99338

FDM-01

Prepared By:

Harper Houf Peterson Righellis Inc. 1220 Main Street, Suite 150 Vancouver, WA 98660 P: 360-750-1131 F: 360-750-1141

Laura Haunreiter, PE

May 18, 2023



ENGINEERS ◆ PLANNERS LANDSCAPE ARCHITECTS ◆ SURVEYORS

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#### **TECHNICAL APPENDICES**

- 1. Maps
- 2. Project Plans
- 3. Stormwater Calculations and Design Information
- 4. Geotechnical Report
- 5. Critical Areas Assessment
- 6. Operations and Maintenance Manual

#### **REFERENCES**

- 1. Department of Ecology Stormwater Management Manual for the Puget Sound Basin, 1992
- 2. GN Northern, Inc. Geotechnical Site Investigation Report. Proposed Rock Creek Cove Development. Stevenson, Washington. January 2020.
- 3. Ecological Land Services. *Rock Cove Preliminary Critical Areas Assessment*. Stevenson, Washington. January 21, 2020.

#### SECTION A - PROJECT OVERVIEW

#### Site Location and Description

FDM Development, Inc. (FDM) proposes to develop a vacant lot located at Rock Creek Cove into a resort that would include 19 vacation rental units, event space, a massage hut, associated utilities and paved parking areas.

The 6.4-acre site is in the town of Stevenson in Skamania County, Section 1, Township 2 North, Range 7 East, Willamette Meridian. The site consists of three contiguous parcels (02070100130300, 02070100130400, and 02070100130200), on an irregularly shaped peninsula projecting into Rock Creek Cove on the northern bank of the Columbia River. It is bordered on the west by Rock Creek Drive, and by the cove on the north, east and south. Historically, the site was developed as an industrial lumber mill facility, the Hegewald Veneer Mill.

#### **Topography**

The subject site is generally characterized as an irregular shaped peninsula with several fingers extending east from Rock Creek Drive into Rock Cove. The elevation of the site is approximately 102 ft above mean sea level. Most of the upper surface of the site is relatively flat, while the irregular shaped peninsula fingers typically include steep slopes along the perimeter down to the shoreline. Surface conditions across the site include a variety of gravel covered and paved areas (asphalt and concrete), as well as areas with a dense growth of mature Douglas-fir, red alder, and maple trees, grasses and Himalayan blackberry, with selected areas across shoreline slope faces that include riprap.

The surface contains artificial fill likely related to the development of the Hegewald Veneer Mill in the early 1950s. Two concrete pads remain onsite (28,982 sf and 2,655 sf) marking the former location of the mill buildings.

#### Critical Areas

A Critical Areas Assessment of the site was completed by Ecological Land Services. ELS and Ecology identified one unnamed tributary north of the study area identified as a Type F (fishbearing) water by Washington Department of Natural Resources (DNR). Rock Creek is east of the study area and is designated as Type S, a shoreline of the state. Rock Cove surrounds the study area on three sides. The Columbia River is designated Type S and is a shoreline of statewide significance. There were no wetlands or other surface waters in the study area, and no priority habitat for terrestrial wildlife. Additionally, portions of the study area are exempt from the designation as a fish and wildlife habitat conservation area (FWHCA) for Rock Cove due to areas of maintained vegetation and the presence of riprap which is both structural and vertical separation from Rock Cove. (See attached Critical Areas Assessment in Appendix 5)

#### Existing Stormwater System

No stormwater system presently exists on-site. Stormwater either infiltrates on-site or flows overland to Rock Cove.

#### Site Soils

The National Resources Conservation Service (NRCS) map identifies the site soils as Arents (0 to 5 percent slopes) with typical profile described as gravelly sandy loam grading to extremely gravelly sandy loam; units generally consisting of well drained materials. The geotechnical investigation of the site determined that the soils include a variably thick layer of artificial fill soils likely associated with historic site development, atop the native silty gravel with sand stratum

(mass wasting deposits). Fill soils were generally classified as silty gravel with sand and variable amounts of cobbles and boulders, and with some areas also including organics, wood debris and miscellaneous trash. The fill soils at the site are likely to be related to the previous historic development at the site. The apparent native underlying soils were classified as Silty Gravel with Sand and included varying amounts of cobbles and boulders. The native soil stratum typically appeared medium dense.

The Geotechnical Engineer tested the infiltration rate at one location near the site entrance. At a depth of 5.5 feet below ground surface (bgs), the infiltration test pit yielded a result of 4 in/hour. The infiltration rate will be confirmed during construction. (See attached Geotechnical Report in Appendix 4.)

#### Drainage to and from Adjacent Properties

The site is relatively flat, with steep slopes along the perimeter of the peninsula down to the shoreline. No stormwater flows onto the site from adjacent parcels.

#### Compliance with Standards

This project is designed to meet the requirements of the City of Stevenson, the Department of Ecology's 1992 Stormwater Management Manual for the Puget Sound Basin (portions adopted by the City of Stevenson), and the Uniform Plumbing Code.

#### **SECTION B - MINIMUM REQUIREMENTS**

There is an existing gravel driveway (pervious) and existing concrete building pads (impervious) on-site. A summary of the project's surface impacts to the site is provided in the table below.

**TABLE B-1. PROJECT IMPACT AREA VALUES** 

Existing Impervious (acres)	Replaced	New	Total Land-
	Impervious	Impervious	Disturbing Activity
	(acres)	(acres)	(acres)
0.72	0.72	0.51	2.00

This project is considered a "New Development" project for stormwater thresholds, as the development is greater than 5000 square feet, with greater than 1 acre of land disturbing activity. Minimum Requirements 1-11 apply.

A summary of how the project meets each of the minimum requirements is described below. See additional sections of this report for more detailed information. See the project plans in Appendix 2 for grading, stormwater and erosion control information.

#### MR#1 – Erosion and Sediment Control

See the project plans in Appendix 2 for temporary erosion control information. The contractor is responsible for conforming to the City of Stevenson and Department of Ecology (DOE) erosion control standards. A Construction Stormwater Pollution Prevention Plan (SWPPP) will be prepared prior to construction.

#### MR#2 – Preservation of Natural Drainage Systems

The majority of the upper surface of the site is relatively flat, while the irregular shaped peninsula fingers typically include steep slopes along the perimeter down to the shoreline. No stormwater flows onto the site from adjacent parcels. Stormwater collected on site will be conveyed to proposed bioretention facilities, and then discharged to Rock Cove. The proposed

outfall will be placed above the Ordinary High Water Mark (OHWM) and stabilized with a riprap dispersion pad.

#### MR#3 – Source Control of Pollution

The primary source of pollutants for this project will be vehicle traffic. The main permanent structural BMPs incorporated on this project will be:

- 1. Sumped catch basins.
- 2. Bioretention facilities located in the north, south and east portions of the site.

The operational BMP will be the continual maintenance of the storm system by the property owner.

#### MR#4 – Runoff Treatment BMPs

Three bioretention facilities designed using WWHM2012 are proposed to treat the collected runoff from the new pollution generating and non-pollution generating impervious surfaces. See attached WWHM Report in Appendix 3.

#### MR#5 - Streambank Erosion Control

Stormwater runoff treated by the proposed bioretention facilities will be discharged to Rock Cove through a proposed outfall, placed above the OHWM and stabilized with a riprap dispersion pad.

#### MR#6 – Wetlands

There are no existing wetlands on the project site.

#### MR#7 – Water Quality Sensitive Areas

There are no known sensitive areas on or immediately downstream of the project site where stormwater will flow, and therefore, this minimum requirement does not apply.

#### MR#8 – Off-Site Analysis and Mitigation

The proposed bioretention facilities will treat stormwater runoff, which will be discharged to Rock Cove, a large water body along the north shore of the Columbia River. There are no negative water quality impacts anticipated downstream of the project site.

#### MR#9 – Basin Planning

There are no impacts to any regional drainage basins or watersheds associated with this development, nor are there any known regional plans that would affect the minimum requirements for this project.

#### MR#10 – Operation and Maintenance

The new stormwater facilities associated with this project will be maintained by the property owner.

#### MR#11 – Financial Liability

These facilities will be constructed by the property owner, and financial guarantee is not necessary.

#### **SECTION C - OFFSITE ANALYSIS**

This project will provide water quality treatment to all proposed runoff from the project site prior to discharge into Rock Cove, and ultimately the Columbia River. See Appendix 2 for project plans and Appendix 3 for all stormwater calculations.

### SECTION D - FLOW CONTROL ANALYSIS AND DESIGN

The stormwater runoff from this site will discharge directly to Rock Cove and the Columbia River. Per the 2019 Stormwater Management Manual for Western Washington, flow control is not required for Threshold Discharge Areas that discharge to a Flow Control Exempt Receiving waterbody. Table I-A.1 in Appendix I-A of the SWMMWW lists the Columbia River downstream of the Canadian border as a Flow Control Exempt Receiving Waters. Therefore, the project is exempt from the requirement to control the off-site flow to match the existing conditions for the 2-year, 10-year and 100-year 24-hour design storms. (A design exemption memo is attached.) Stormwater runoff from the site will be collected and conveyed to proposed bioretention facilities, and discharged to Rock Cove through proposed outfalls, placed above the OHWM and stabilized with a riprap dispersion pad. See project plans in Appendix 2.

#### SECTION E - CONVEYANCE SYSTEMS ANALYSIS AND DESIGN

For conveyance calculations, the capacity of the pipe based on size, slope and pipe type was calculated using the Rational Method and a 25-year storm event. The flow was calculated for the immediate and upstream basins conveyed to each pipe segment. The actual flow for each pipe was then compared to the capacity of the pipe to ensure that the pipe was free flowing.

- 1. The conveyance system for the project consists of overland flow, inlets and underground pipes to convey stormwater to the bioretention facilities and the proposed outfalls.
- 2. The conveyance system has not been significantly adjusted since the preliminary plan set.
- 3. Refer to Appendix 3 Stormwater Calculations where peak flows and pipe capacities are tabulated.
- 4. The existing site consisted of onsite infiltration and overland flow to Rock Creek Cove. The design has kept this same flow regime.
- 5. The assumptions utilized in the conveyance system design are:
  - The rainfall data in the storm calculations accurately reflects the rainfall.
  - b. The proposed conveyance system will be well maintained by the owner.
- 6. Refer to Appendix 2 Project Plans for details regarding the stormwater piping and outfall protection.
- 7. Refer to Appendix 3 Stormwater Calculations where peak flows and pipe capacities are tabulated and verified.

#### SECTION F – WATER QUALITY DESIGN

The proposed BMPs, bioretention facilities, were designed using WWHM2012 per the DOE Stormwater Manual. They are designed to infiltrate at least 91% of the runoff through the treatment soil. The infiltration rate utilized for design of the bioretention facility soil was 12 in/hour with a factor of safety of four (design rate = 3 in/hour). Infiltration tests conducted onsite in the vicinity of the proposed bioretention facilities yielded results of 4 in/hour (5.5 feet bgs). The non-pollution generating roof drain runoff will flow to roof drain dispersion systems. See Appendix 3 for all stormwater calculations and Appendix 4 for the Geotechnical Report.

**TABLE F-1. Pollution Generating Surface Summary** 

Post-Developed

Pollution Generating Impervious Surface (acre)	Pollution Generating Pervious Surface (acre)
0.78	0

The asphalt parking area and the drive aisles are considered pollution generating. The roof areas and pedestrian pathways are considered non-pollution generating. The runoff from the pollution generating and the non-pollution generating pedestrian areas will be treated in bioretention facilities and discharged to proposed outfalls with riprap dispersion pads located above the OHWM of Rock Cove. As shown in the WWHM printouts, the bioretention facilities treat at least 91% of the water quality runoff through the treatment soils. There are three bioretention facilities within the project:

Bioretention Facility 1 (North): The northern bioretention facility is located north of the north parking area and will treat pollution generating runoff from the Basin A parking area and drive aisles. The facility has a bottom area of approximately 24 sf, 3:1 side slopes, 6 inches of storage volume, 18 inches BSM depth, and 12 inches of drain rock below the BSM.

Bioretention Facility 2 (South): The southern bioretention facility is located southwest of the south parking area and will treat pollution generating runoff from the Basin B parking area and drive aisles. The facility has a bottom area of approximately 24 sf, 3:1 side slopes, 6 inches of storage volume, 18 inches BSM depth, and 12 inches of drain rock below the BSM.

Bioretention Facility 3 (East): The eastern bioretention facility is located east of the east parking area and will treat pollution generating runoff from the Basin C parking area and drive aisles, as well as non-pollution generating roof drain runoff from the event center. The facility has a bottom area of approximately 12 sf, 3:1 side slopes, 6 inches of storage volume, 18 inches BSM depth, and 12 inches of drain rock below the BSM.

- 1. The water quality will be mitigated by sumped catch basins and bioretention facilities.
- 2. The water quality system has not been significantly adjusted since the preliminary plan set.
- 3. Refer to Appendix 4 for the geotechnical report.
- 4. The Best Management Practices (BMPs) utilized in design are bioretention facilities. BMP T5.13 Post-Construction Soil Quality and Depth will be completed on all disturbed soils.
- 5. Groundwater was found on-site ranging from 12' 14' below existing ground surface in the month of December. See Geotechnical Report, Appendix 4.
- 6. The assumptions utilized in the water quality system design are:
  - a. The rainfall data in WWHM2012 accurately reflects the rainfall.
  - b. The proposed bioretention facilities will be well maintained by the owner.
- 7. Refer to the project plans for details regarding the stormwater piping, outfall protection, and bioretention facilities.
- 8. Refer to Appendix 3 Stormwater Calculations for all storm water treatment calculations.

#### **SECTION G - SOILS EVALUATION**

- 1. The National Resources Conservation Service (NRCS) map identifies the site soils as Arents (0 to 5 percent slopes) with typical profile described as gravelly sandy loam grading to extremely gravelly sandy loam; units generally consisting of well drained materials. The geotechnical investigation of the site determined that the soils include a variably thick layer of artificial fill soils likely associated with historic site development, atop the native silty gravel with sand stratum (mass wasting deposits). Fill soils were generally classified as silty gravel with sand and variable amounts of cobbles and boulders, and with some areas also including organics, wood debris and miscellaneous trash. The fill soils at the site are likely to be related to the previous historic development at the site. The apparent native underlying soils were classified as Silty Gravel with Sand and included varying amounts of cobbles and boulders. The native soil stratum typically appeared medium dense. (See attached Geotechnical Report in Appendix 4.)
- 2. The Geotechnical Engineer tested the infiltration rate at one location near the site entrance. At a depth of 5.5 feet below ground surface (bgs), the infiltration test pit yielded a result of 4 in/hour. The infiltration rate will be confirmed during construction. (See attached Geotechnical Report in Appendix 4.)
- 3. The soil parameters that affected the stormwater design are the infiltration rates, as well as the proximity to a large receiving water body.

#### SECTION H - SPECIAL REPORTS AND STUDIES

- 1. Geotechnical Report See Appendix 4.
- 2. Critical Areas Assessment See Appendix 5.

#### **SECTION I – OTHER PERMITS**

1. N/A

#### **SECTION J – GROUNDWATER MONITORING PROGRAM**

1. N/A

### **SECTION K - MAINTENANCE AND OPERATIONS MANUAL**

1. An Operations and Maintenance manual is provided in Appendix 6.

#### **SECTION L - TECHNICAL APPENDIX**

The Technical Appendices include all computations, drawings, maps, referenced data, software printouts, specials studies, and all other information used in the preparation of this report.

- 1. Maps
- 2. Project Plans
- 3. Stormwater Calculations and Design Information
- 4. Geotechnical Report
- 5. Critical Areas Assessment
- 6. Operations and Maintenance Manuals

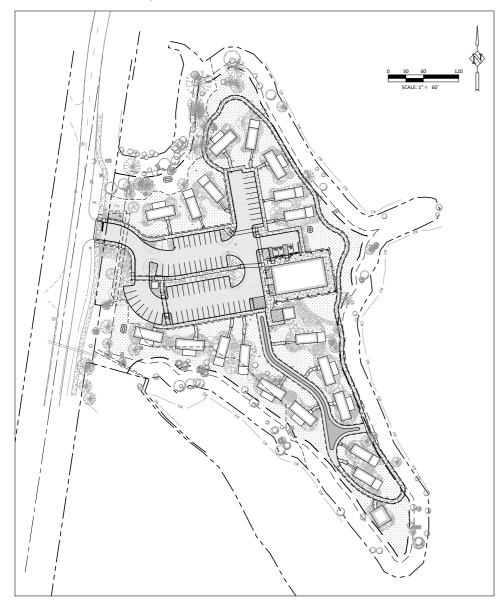
## APPENDIX 1 - Maps



## <u>APPENDIX 2</u> – Project Plans

# **ROCK CREEK COVE HOSPITALITY**

## STEVENSON, WASHINGTON





**VICINITY MAP** 

#### **PROJECT TEAM**

OWNER / DEVELOPER:

#### CIVIL ENGINEER:

HARPER HOUF PETERSON RIGHELLIS INC 1220 MAIN STREET, SUITE 150 VANCOLIVER WASHINGTON 98660

#### SURVEYOR:

#### SITE INFORMATION:

LOTS 2, 3, AND 4 OF ROCK CREEK COVE

SHEET LIST TABLE					
Sheet Number	Sheet Title				
C1.00	COVER				
C1.01	GENERAL NOTES				
C2.00	OVERALL EXISTING CONDITIONS & DEMO PLAN				
C3.00	OVERALL SITE PLAN				
C4.00	OVERALL GRADING, DRAINAGE & EROSION CONTROL PLAN				
C5.00	OVERALL UTILITY PLAN				
C6.00	DETAILS				
C6.01	DETAILS				
C6.02	DETAILS				

#### LEGEND:

PROPOSED _____ EASEMENT · /-/-/-/- UTILITY DEMOLTION

TC = TOP OF CURB
BC = BOTTOM OF CURB
FS = FINISH SURFACE
FG = FINISH GRADE
TW = TOP OF WALL
BW = BOTTOM OF WALL
FF = FINISHED FLOOR
TG = TOP OF GRATE

#### EXISTING

BUILDING OVERHANG BUILDING DECK PARKING STRIPES FLOW LINE CURB EXTRUDED CURB STANDARD CURB EDGE OF PAVEMENT EDGE OF CONCRETE FIGE OF GRAVE GUARD RAIL WALL-TOP WALL-TOE COMMUNICATIONS - CABLE TV

STORM SEWER SANITARY SEWER SANITARY SEWER-PRESSURE TRAFFIC SIGNAL

#### LIST OF ABBREVIATIONS

UNKNOWN UNDERGROUND UTILITY

EDGE OF LANDSCAPING EDGE OF WATER FENCE - MISC*** FENCE - BARBED WIRE FENCE - WOOD FENCE - CHAINLINK BOUNDARY LINE

TELEPHONE COMMUNICATION

IRRIGATION

EASEMENT LOT/PARCEL LINE TREELINE DONATION LAND CLAIM SECTION LINE NATURAL CAS LINE BUILDING HATCH

GRAVEL HATCH

SET MONUMENT SET BERNSTEN BRASS DISC CALCULATED POSITION FOUND MONUMENT-ALUMINUM C FOUND MONUMENT-BRASS CAP FOUND MONUMENT-IRON PIPE FOUND MONUMENT NO. FOUND MONUMENT - 1/2" IRON ROD FOLIND MONLIMENT - MAG NAIL FOUND MONUMENT - 5/8" IRON ROD FOUND MONUMENT - SCREW BENCHMARK CONTROL ALUM CAP/BRASS CAP CONTROL HUB & TACK CONTROL MAGNAIL/PK NAIL CONTROL 5/8" IR W/ RPC

CONTROL SCRIBE CONTROL NAIL & WASHER CONTROL POINT HANDICAP PARKING FLAG POLE MAILBOX ROCK-BOULDER

TEST PIT / BOREHOLE A/C UNIT STOP SIGN MONITORING WELL BOLLARD BUILDING COLUMN RAILROAD X-ING FENCE GATE POST WETLAND FLAG TREE - STUMP

TREE - DECIDUOUS

TREE - CONIFER

SANITARY SEWER CLEANOUT SANITARY SEWER MANHOLE STORM AREA DRAIN STORM CATCH BASIN STORM MANHOLE STORM ROOF DRAIN STORM CLEANOUT STORM CULVERT STORM DITCH INLET/SLANTED CATCH BASIN STORM TRAPPED INLET STORM CATCH BASIN - ROUND STORM CURB INLET STORM COMBINATION CURB INLET WATER MANHOLE WATER AIR RELEASE VALVE

POWER POLE
POWER TRANSFORMER

POWER POLE W/ LIGHT POWER RISER

POWER JUNCTION BOX

LIGHT-LAMP POST

CLIY ANCHOR

POWER CABINET

POWER MANHOLE POWER GUY POLE

POWER VAULT

WATER FIRE DEPT, CONNECT WATER POST INDICATOR VALVE WATER SPIGOT - SHUTOFF WATER VAULT FIRE HYDRANT WATER METER WATER VALVE WATER STANDPIPE WELL GAS VALVE GAS METER GAS RISER GAS FINK

TRAFFIC SIGNAL CROSSWALK TRAFFIC SIGNAL JUNCTION BOX TRAFFIC SIGNAL POLE TELEPHONE RISER TELEPHONE MANHOLE
TELEPHONE VAULT COMMUNICATION MANHOLE COMMUNICATION VAULT

SPRINKLER HEAD CATV RISER UR UR UNKNOWN VAULT UNKNOWN RISER

UNKNOWN JUNCTION BOX

DEVELOPMENT INC.

HOSPITALITY CREEK COVE ROCK

NOT FOR CONSTRUCTION

C1.00

#### **GENERAL NOTES:**

- WORK SHALL CONFORM WITH CITY OF STEVENSON STANDARDS, THE INTERNATIONAL BUILDING CODE AND THE LATEST EDITION OF THE TANDARD SPECIFICATIONS FOR ROAD, BRIDGE, AND MUNICIPAL CONSTRUCTION" AS PREPARED BY WSDOT AND APWA.
- 2. AS-BUILT INFORMATION SHOULD BE FIELD VERIFIED PRIOR TO CONSTRUCTION. THIS INCLUDES POTHOLING EXISTING UTILITIES AT PROPOSED NECTION POINTS PRIOR TO CONSTRUCTION TO ENSURE LOCATIONS AND ELEVATIONS ARE ACCURATE. NOTIFY ENGINEER IMM WITH POTHOLE RESULTS FOR DISCREPANCIES.
- PROVISIONS SHALL BE MADE BY THE CONTRACTOR TO KEEP ALL EXISTING UTILITIES NOT SHOWN FOR REMOVAL IN SERVICE AND PROTECT THEM 18. ALL PIPE SHALL HAVE A MINIMUM OF 36" OF COVER MEASURED FROM FINISH GRADE. DURING CONSTRUCTION.
- EXISTING MONUMENTS, PROPERTY CORNERS, AND SURVEY MARKERS SHALL BE PROTECTED. REPLACEMENT SHALL BE AT THE CONTRACTOR'S EXPENSE.
- CONSTRUCTION STAGING IS NOT PERMITTED IN THE PUBLIC RIGHT-OF-WAY
- 6. EXISTING UTILITIES SHOWN ON THE PLANS ARE PER SURFACE LOCATIONS AND AS-BUILT DRAWINGS. ADDITIONAL UNDERGROUND UTILITIES MAY EXIST. THE CONTRACTOR SHALL VERIFY LOCATIONS OF ALL EXISTING UTILITIES PRIOR TO CONSTRUCTION AND REPORT ANY CONFLICTS TO THE ENGINEER. THE CONTRACTOR SHALL COORDINATE THE RELOCATION OF ANY UTILITY IN CONFLICT WITH THE PROPOSED CONSTRUCTION.
- CONTRACTOR SHALL CONFIRM ALL REQUIRED PERMITS AND LICENSES HAVE BEEN ISSUED BEFORE STARTING CONSTRUCTION
- 8. CONTRACTOR SHALL NOTIFY THE PROJECT ENGINEER AND THE CITY OF STEVENSON INSPECTOR 48 HOURS BEFORE INSPECTION. CONSTRUCTION VEHICLES ARE NOT ALLOWED TO BE STAGED IN THE PUBLIC RIGHT-OF-WAY WITHOUT PRIOR APPROVAL
- 10. CONTRACTOR SHALL KEEP AND MAINTAIN A CURRENT SET OF DRAWINGS FOR THE PROJECT ENGINEER SHOWING AS-CONSTRUCTED DATA CONTRACTOR SHALL KEEP AN APPROVED SET OF PLANS ON THE PROJECT SITE AT ALL TIMES.
- 11. CONTRACTOR SHALL PERFORM ALL WORK NECESSARY TO COMPLETE THIS PROJECT IN ACCORDANCE WITH THE PLANS INCLUDING SUCH INCIDENTALS AS MAY BE NECESSARY TO MEET APPLICABLE AGENCY REQUIREMENTS AND OTHERS AS NECESSARY TO PROVIDE A COMPLETE
- 12. ANY ALTERATION OR VARIANCE FROM THESE PLANS, EXCEPT MINOR FIELD ADJUSTMENTS NEEDED TO MEET EXISTING FIELD CONDITIONS, SHALL 24. FIRST BE APPROVED BY THE APPLICABLE AGENCY REPRESENTATIVE. ANY ALTERATION OR VARIANCE FROM THESE PLANS SHALL BE DOCUMENTED ON CONSTRUCTION FIELD PRINTS AND TRANSMITTED TO THE PROJECT ENGINEER.
- 13. CONTRACTOR SHALL PROVIDE THE NECESSARY FROSION PROTECTION TO MINIMIZE FROSION AND IMPACT TO ADJACENT PROPERTIES
- 14. OPEN TRENCHES SHALL BE STRICTLY LIMITED TO A MAXIMUM OF 100 FEET UNLESS LIMITED TO A LESSER AMOUNT BY PERMIT. NO TRENCHES WILL BE ALLOWED TO REMAIN OPEN AT NIGHT.
- 15. AT THE END OF EACH WORK DAY THE CONTRACTOR SHALL CLEAN UP THE PROJECT AREA AND LEAVE IT IN A NEAT AND SECURED MANNER. UPON COMPLETION. THE CONTRACTOR SHALL LEAVE THE PROJECT AREA FREE OF DEBRIS AND UNUSED MATERIAL
- 16. THE CONTRACTOR SHALL PRUNE ALL VEGETATION. AS NECESSARY, AWAY AND UP FROM THE AREA OF WORK. THE CONTRACTOR SHALL PROTECT ALL EXISTING LANDSCAPING THAT IS TO REMAIN. ARBORIST SHALL BE CONTACTED IF SIGNIFICANT ROOTS ARE UNCOVERED.
- 17. ALL MATERIAL SUPPLIERS SHALL SUBMIT TO THE ENGINEER PROOF OF MATERIAL(S) TESTED IN ACCORDANCE WITH SPECIFICATIONS. BY ACCEPTANCE OF THE CONTRACT WITH THE OWNER/DEVELOPER, THE CONTRACTOR CERTIFIES THAT ALL MATERIALS DELIVERED TO THE JOB SITE

  4. WILL MEET OR EXCEED THOSE SPECIFICATIONS. ANY MATERIAL NOT CONFORMING SHALL BE REMOVED FROM THE SITE AT NO ADDITIONAL

#### TRAFFIC CONTROL:

- TRAFFIC CONTROL TO BE PERFORMED IN ACCORDANCE WITH THE MANUAL FOR UNIFORM TRAFFIC CONTROL DEVICES. THE CITY OR COUNTY CAN REQUIRE ADDITIONAL TRAFFIC CONTROL MEASURES AS NEEDED TO PROVIDE FOR PUBLIC SAFETY.
- 2 A TRAFFIC CONTROL PLAN SHALL RE PREPARED AND SLIBMITTED FOR REVIEW BY CITY FOR BOTH CONSTRUCTION OPERATIONS AND
- 3. ALL TRAFFIC CONTROL MEASURES NEED TO BE SUBMITTED TO THE CITY FOR REVIEW AND APPROVAL

#### PRIVATE UTILITIES NOTES:

- 1. PRIVATE STORM DRAINAGE CONSTRUCTION SHALL BE IN ACCORDANCE WITH CITY OF STEVENSON, THE INTERNATIONAL BUILDING CODE (IBC) AND THE UNIFORM PLUMBING CODE (UPC).
- STORM SEWER PIPE MATERIAL SHALL BE HDPE (ADS N-12), PVC ASTM D-3034, OR ENGINEER APPROVED EQUAL
- THE CONTRACTOR SHALL FLUSH THE ENTIRE STORM SYSTEM AT PROJECT COMPLETION
- 4. CATCH BASINS SHALL BE INSTALLED TO FINISH GRADE.
- ADJUST MANHOLES, CLEAN OUT AND AREA DRAIN RIMS TO FINISH GRADI
- 6. HORIZONTAL LINES CONNECTING WITH OTHER HORIZONTAL LINES SHALL ENTER THROUGH 45 DEGREE WYE BRANCH. TEE BRANCH IS NOT
- 7. ALL RAIN DRAIN PIPING INSTALLED WITHIN 5.0 FEET OF A BUILDING TO BE SCHEDULE 40 PVC-D.W.V. PIPING OR APPROVED EQUAL. COORDINATE LOCATION OF RAIN DRAINS WITH PLUMBING PLANS.
- 8. FOLINDATION DRAIN PIPE SHALL BE COORDINATED WITH STRUCTURAL AND ARCHITECTURAL PLANS AND DETAILS. AT FOLINDATION DRAIN CONNECTION INSTALL CLEANOUT AND ACCESSIBLE FLAPPER TYPE BACKWATER VALVE. SET RIM TO FINISH GRADE. COORDINATE FOUNDATION DRAIN CONNECTION POINTS WITH ARCHITECTURAL AND STRUCTURAL PLANS.
- 9. TRACER WIRE 12-GAUGE STRANDED OR SOLID COPPER INSULATED HIGH MOLECULAR WEIGHT POLYETHYLENE (HMW-PE) TRACER WIRE. THE HMW-PE INSULATED COVER SHALL BE GREEN AND A MINIMUM 45 MIL THICK. THE WIRE SHALL BE RATED FOR 140 DEGREES FAHRENHEIT.
  INSTALL TRACER WIRE IN ALL TRENCHES FOR STORM SEWERS. PLACE THE TRACER WIRE DIRECTLY OVER THE PIPE CENTERLINE AND ON TOP OF THE PIPE ZONE MATERIAL PARALLEL TO AND ALONG THE ENTIRE LENGTH OF ALL NONMETALLIC PIPE
- 10. ALL STORMWATER FACILITIES SHALL REMAIN IN PRIVATE OWNERSHIP AND SHALL BE PRIVATELY MAINTAINED IN ACCORDANCE WITH THE O&M MANUAL AND THE PROJECT TIR.

#### SANITARY SEWER

- 11. ALL SANITARY SEWER CONSTRUCTION TO WITHIN THREE (3) FEET OF THE BUILDING SHALL BE PVC ASTM D3034 SDR 35 AND PVC C900 WHER PIPE COVER IS LESS THAN 3' IN VEHICULAR AREAS AND IN ACCORDANCE WITH CITY OF STEVENSON, THE INTERNATIONAL BUILDING CODE (IBC) AND UNIFORM PLUMBING CODE (UPC).
- 12. PRIVATE SANITARY SEWER PIPE WITHIN THREE (3) FEET OF THE BUILDING SHALL BE DRAIN WASTE VENT (DWV), IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE (IBC) AND UNIFORM PLUMBING CODE (UPC).
- 13. HORIZONTAL LINES CONNECTING WITH OTHER HORIZONTAL LINES SHALL ENTER THROUGH 45 DEGREE WYE BRANCH. TEE BRANCH IS NOT
- 14. WHERE SANITARY LINES CROSS WATER LINES, THE SYSTEMS NEED TO BE CONSTRUCTED SUCH THAT THE CROSSING WILL OCCUR AT THE CENTER E
- 15. PRIOR TO TESTING AND INSPECTION OF THE SANITARY PIPELINE ALL PARTS OF THE SYSTEM SHALL BE CLEANED OF ALL DERRIS

- 16. TESTING OF PRIVATE SANITARY PIPELINE SHALL BE IN ACCORDANCE WITH INTERNATIONAL BUILDING CODE AND UNIFORM PLUMBING CODE
- TRACER WIRE 12-GAUGE STRANDED OR SOLID COPPER INSULATED HIGH MOLECULAR WEIGHT POLYETHYLENE (HMW-PE) TRACER WIRE. THE HMW-PE INSULATED COVER SHALL BE GREEN AND A MINIMUM 45 MIL THICK. THE WIRE SHALL BE RATED FOR 140 DEGREES FAHRENHEIT ISTALL TRACER WIRE IN ALL TRENCHES FOR SANITARY SEWERS. PLACE THE TRACER WIRE DIRECTLY OVER THE PIPE CENTERLINE AND ON TOP OF THE PIPE ZONE MATERIAL. PARALLEL TO, AND ALONG THE ENTIRE LENGTH OF ALL NONMETALLIC PIPE.

- WATER PIPE MATERIAL SHALL BE SCHEDULE 80 PVC OR ENGINEER APPROVED EQUAL.
- FIRE SERVICE PIPE MATERIAL SHALL BE C900, D.I.P. OR ENGINEER APPROVED EQUAL
- THE CONTRACTOR SHALL CALL FOR ALL INSPECTIONS AND PERFORM THE NECESSARY TESTING REQUIRED BY THE SPECIFICATIONS AND THE PRIVATE UTILITIES PERMIT. UPON COMPLETION OF THE INSTALLATION OF THE WATER SYSTEM ALL LINES SHALL BE FLUSHED AND DISINFECTED IN CONFORMANCE WITH HEALTH DIVISION GUIDELINES.
- ALL WATERLINES, JOINTS, TEES, BENDS (HORIZ. & VERT.), REDUCERS AND VALVES SHALL BE MECHANICALLY RESTRI
- 23. ALL WATER PIPE SHALL COMPLY WITH AWWA STANDARDS AND UL APPROVED.
- TRACER WIRE 12-GAUGE STRANDED OR SOLID COPPER INSULATED HIGH MOLECULAR WEIGHT POLYETHYLENE (HMW-PE) TRACER WIRE. THE HMW.PE INSULATED COVER SHALL BE GREEN AND A MINIMUM 4S MILLTHICK. THE WIRE SHALL BE RATED FOR 140 DEGREES FAHRENHEIT INSTALL TRACER WIRE IN ALL TRENCHES FOR WATER LINE. PLACE THE TRACER WIRE DIRECTLY OVER THE PIPE CENTERLINE AND ON TOP OF THE PIPE ZONE MATERIAL, PARALLEL TO, AND ALONG THE ENTIRE LENGTH OF ALL NONMETALLIC PIPE

#### MISC LITILITIES

- ELECTRICAL, TELEPHONE, GAS, AND TV INSTALLATION SHALL BE COORDINATED BY THE CONTRACTOR WITH THE APPROPRIATE UTILITY OMPANY INCLUDING REQUIREMENTS FOR UTILITY CROSSING SLEEVES.
- ALL PROPOSED POWER, TELEPHONE, GAS, AND TV SERVICES ON SITE SHALL BE PLACED UNDERGROUNI
- INSTALLATIONS: STORM, SANITARY, WATER, IRRIGATION CROSSINGS, PRIVATE UTILITIES.
- PLACE DETECTABLE MARKING TAPE AND TRACER WIRE IN THE TRENCH DIRECTLY ABOVE. PARALLEL TO, AND ALONG THE ENTIRE LENGTH OF ALL NONMETALLIC PIPE AND CONDUIT.

#### SITE GRADING, PREPARATION AND FILL NOTES:

- REFER TO FINAL GEOTECHNICAL REPORT FOR ADDITIONAL EARTHWORK AND GEOTECHNICAL RECOMMENDATIONS.
- BUILDING SLAB AND FOUNDATION DESIGN SHALL BE PER STRUCTURAL DRAWINGS AND GEOTECHNICAL ENGINEERING REPORT
- THE PERIMETER GROUND SURFACE AND HARDSCAPE SHOULD BE SLOPED TO DRAIN AWAY FROM ALL STRUCTURES AND AWAY FROM ADJACENT CONSTRUCTION OF THE PROPOSED DEVELOPMENT WILL INVOLVE CLEARING AND GRUBBING OF THE EXISTING VEGETATION AND DEMOLITION
- OF EXISTING STRUCTURES. DEMOLITION SHALL INCLUDE REMOVAL OF EXISTING PAVEMENT, SLABS, UTILITIES, ETC., THROUGHOUT T PROPOSED NEW DEVELOPMENT. VEGETATION, ROOTS, ORGANIC LADEN SOILS, AND ANY OTHER DELETERIOUS SOILS SHALL BE REMOVED. UNDERGROUND UTILITY LINES OR OTHER ABANDONED STRUCTURAL ELEMENTS SHALL BE REMOVED. THE VOIDS RESULTING FROM REMOVAL O FOUNDATIONS OR LOOSE SOIL IN UTILITY LINES SHALL BE BACKFILLED WITH COMPACTED STRUCTURAL FILL. THE BASE OF THESE EXCAVATIONS SHOULD BE EXCAVATED TO FIRM NATIVE SUBGRADE BEFORE FILLING, WITH SIDES SLOPED AT A MINIMUM OF 1H:1V TO ALLOW FOR UNIFORM COMPACTION. MATERIALS GENERATED DURING DEMOLITION SHOULD BE TRANSPORTED OFF SITE OR STOCKPILED IN AREAS DESIGNATED BY TIME OF CONSTRUCTION.
- FILL SHOULD BE PLACED IN RELATIVELY UNIFORM HORIZONTAL LIFTS ON THE PREPARED SUBGRADE. EACH LOOSE LIFT SHOULD BE ABOUT 10 INCHES THICK. THE TYPE OF COMPACTION EQUIPMENT USED WILL ULTIMATELY DETERMINE THE MAXIMUM LIFT THICKNESS. STRUCTURAL FILL HALL BE COMPACTED TO AT LEAST 92 PERCENT OF MODIFIED PROCTOR MAXIMUM DRY DENSITY AS DETERMINED BY ASTM DESIGNATION D 1557 (MODIFIED PROCTOR).
- ANY STRUCTURAL FILL PLACED ON SLOPES AT OR GREATER THAN 5H:1V SHOULD BE PROPERLY BENCHED. LEVEL BENCHES EXCAVATED INTO THE FYISTING SLOPE SHOULD BE A MINIMUM OF 10 FEFT WIDE LATERALLY AND SHOULD BE CLIT INTO THE SLOPE FOR EVERY FIVE FEFT OF MAXIMUM VERTICAL RISE. THE PLACEMENT OF FILL SHOULD BEGIN AT THE BASE OF THE FILL. ALL BENCHES SHOULD BE INSPECTED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER AND APPROVED PRIOR TO PLACEMENT OF STRUCTURAL FILL LIFTS. IF EVIDENCE OF SEEPAGE IS OBSERVED IN THE BENCH EXCAVATIONS, A SUPPLEMENTAL DRAINAGE SYSTEM MAY NEED TO BE DESIGNED AND INSTALLED TO REVENT HYDROSTATIC PRESSURE BUILDUP BEHIND THE FILL. FINAL FILL AND/OR CUT SLOPES SHOULD BE KEPT AT OR BELOW 2H:1V
- EACH LIFT OF COMPACTED ENGINEERED FILL SHOULD BE TESTED BY A REPRESENTATIVE OF THE GEOTECHNICAL ENGINEER PRIOR TO
- FOLLOWING SITE PREPARATION AND PRIOR TO PLACING AGGREGATE BASE FOR SHALLOW FOUNDATIONS. BUILDING PAD. SLAB SUBGRADE SECTIONS, OR PAVEMENT SECTIONS, THE EXPOSED SUBGRADE SHOULD BE EVALUATED EITHER BY PROOFROLLING OR ANOTHER METHOD OF SUBGRADE VERIFICATION. THE SUBGRADE SHOULD BE PROOFFOLLED WITH A FULLY LOADED DUMP TRUCK OR SIMILAR HEAVY, RUBBER-TIRE CONSTRUCTION EQUIPMENT TO IDENTIFY UNSUITABLE AREAS. IF EVALUATION OF THE SUBGRADES OCCURS DURING WET CONDITIONS, OR IF PROOFROLLING THE SUBGRADES WILL RESULT IN DISTURBANCE. THEY SHOULD BE EVALUATED BY THE GEOTECHNICAL ENGINEER.
- SITE EARTHWORK AND SUBGRADE PREPARATION SHOULD NOT BE COMPLETED DURING FREEZING CONDITIONS, EXCEPT FOR MASS EXCAVATION TO THE SUBGRADE DESIGN ELEVATIONS.
- ALL EXCAVATIONS SHOULD BE MADE IN ACCORDANCE WITH APPLICABLE OCCUPATIONAL SAFETY AND HEALTH ADMINISTRATION (OSHA) AND
- IF DEWATERING IS REQUIRED, THE TYPE AND DESIGN OF THE DEWATERING SYSTEM SHALL BE THE RESPONSIBILITY OF THE CONTRACTOR. CUT AND FILL SLOPES SHALL BE PROTECTED FROM EROSION. SUCH CONTROL MAY CONSIST OF APPROPRIATE REVEGETATION OR OTHER
- ACCEPTABLE MEANS AND METHODS. EROSION CONTROL MEASURES SHALL BE IN PLACE PRIOR TO EARTHWORK OR SITE STRIPPING.

#### **EROSION AND SEDIMENT CONTROL NOTES:**

(E.G., WETLANDS), AND OTHER AREAS TO BE PRESERVED, ESPECIALLY IN PERIMETER AREAS.

- THE ESCP MEASURES SHOWN ON THIS PLAN ARE MINIMUM REQUIREMENTS FOR ANTICIPATED SITE CONDITIONS. DURING THE CONSTRUCTION PERIOD
- UPGRADE THESE MEASURES AS NEEDED TO COMPLY WITH ALL APPLICABLE LOCAL, STATE, AND FEDERAL EROSION AND SEDIMENT CONTROL REGULATION SUBMISSION OF ALL ESCP REVISIONS IS NOT REQUIRED. SUBMISTIAL OF THE ESCP REVISIONS IS ONLY UNDER SPECIFIC CONDITIONS.
- PHASE CLEARING AND GRADING TO THE MAXIMUM EXTENT PRACTICAL TO PREVENT EXPOSED INACTIVE AREAS FROM BECOMING A SOURCE OF EROSION.
- IDENTIFY, MARK, AND PROTECT (BY FENCING OFF OR OTHER MEANS) CRITICAL RIPARIAN AREAS AND VEGETATION INCLUDING IMPORTANT I ASSOCIATED ROOTING ZONES, AND VEGETATION AREAS TO BE PRESERVED. IDENTIFY VEGETATIVE BUFFER ZONES BETWEEN THE SITE AND SENSITIVE AREA
- PRESERVE EXISTING VEGETATION WHEN PRACTICAL AND RE-VEGETATE OPEN AREAS. RE-VEGETATE OPEN AREAS WHEN PRACTICABLE BEFORE AND AFTE GRADING OR CONSTRUCTION. IDENTIFY THE TYPE OF VEGETATIVE SEED MIX USED.
- FROSION AND SENIMENT CONTROL MEASURES INCLUDING DEPIMETER SENIMENT CONTROL MILIST RE IN DLACE REFORE VEGETATION IS DISTURBED AND

- MUST REMAIN IN PLACE AND BE MAINTAINED. REPAIRED. AND PROMPTLY IMPLEMENTED FO CONSTRUCTION. INCLUDING PROTECTION FOR ACTIVE STORM DRAIN INLETS AND CATCH BASINS AND APPROPRIATE NON-STORMWATER POLLUTION
- PIT OR LEAK-PROOF CONTAINER. HANDLE WASH WATER AS WASTE, CONCRETE DISCHARGE TO WATER OF THE STATE IS PROHIBITED.
- ROADWAYS INCLUDING GRAVEL ROADWAYS.
- PREVENT TRACKING OF SEDIMENT ONTO PUBLIC OR PRIVATE ROADS USING BMPS SUCH AS: GRAVELED (OR PAVED) EXITS AND PARKING AREAS, GRAVEL ALL
- WHEN TRUCKING SATURATED SOILS FROM THE SITE, EITHER USE WATER-TIGHT TRUCKS OR DRAIN LOADS ON SITE.
- STORAGE; OTHER CLEANING AND MAINTENANCE ACTIVITIES; AND WASTE HANDLING ACTIVITIES. THESE POLLUTANTS INCLUDE FUEL, HYDRAULIC FLUID, AND OTHER OILS FROM VEHICLES AND MACHINERY, AS WELL AS DEBRIS, LEFTOVER PAINTS, SOLVENTS, AND GLUES FROM CONSTRUCTION OPERATIONS
- PREVENTION AND PROPER DISPOSAL PROCEDURES, SPILL KITS IN ALL VEHICLES, REGULAR MAINTENANCE SCHEDULE FOR VEHICLES AND MACHINE
- USE WATER, SOIL-BINDING AGENT OR OTHER DUST CONTROL TECHNIQUE AS NEEDED TO AVOID WIND-BLOWN SOIL.
- AT THE END OF EACH WORKDAY SOIL STOCKPILES MUST BE STABILIZED OR COVERED, OR OTHER BMPS MUST BE IMPLEMENTED TO PREVENT DISC
- CONSTRUCTION ACTIVITIES MUST AVOID OR MINIMIZE EXCAVATION AND CREATION OF BARE GROUND DURING WET WEATHER OCTOBER 01 MAY 31.
- SPOIMENT FRACE- REMOVE TRADDED SENIMENT REFORE IT REACHES ONE THIRD OF THE AROVE GROUND FRACE HEIGHT AND REFORE FRACE 19. OTHER SEDIMENT BARRIERS (SUCH AS BIOBAGS): REMOVE SEDIMENT BEFORE IT REACHES TWO INCHES DEPTH ABOVE GROUND HEIGHT. AND BEFORE BM
- TRENCH BACKFILL WITHIN THE PUBLIC RIGHT OF WAY TO BE CRUSHED ROCK PER CITY OF STEVENSON STANDARDS. THIS APPLIES TO ALL UTILITY 20. CATCH BASINS: CLEAN BEFORE RETENTION CAPACITY HAS BEEN REDUCED BY FIFTY PERCENT. SEDIMENT BASINS AND SEDIMENT TRAPS: REMOVI
  - MENTS BEFORE DESIGN CAPACITY HAS BEEN REDUCED BY FIFTY PERCENT AND AT COMPLETION OF PROJECT. WITHIN 24 HOURS, SIGNIFICANT SEDIMENT THAT HAS LEFT THE CONSTRUCTION SITE, MUST BE REMEDIATED. INVESTIGATE THE CAUSE OF THE SEDIMEN'
  - THE INTENTIONAL WASHING OF SEDIMENT INTO STORM SEWERS OR DRAINAGE WAYS MUST NOT OCCUR. VACUUMING OR DRY SWEEPING AND MATERIA
  - 23. PROVIDE PERMANENT EROSION CONTROL MEASURES ON ALL EXPOSED AREAS DO NOT REMOVE TEMPORARY SEDIMENT CONTROL PRACTICES LINTH ANENT VEGETATION OR OTHER COVER OF EXPOSED AREAS IS ESTABLISHED. HOWEVER, DO REMOVE ALL TEMPORARY EROSION CONTROL MEA AS EXPOSED AREAS BECOME STABILIZED, UNLESS DOING SO CONFLICTS WITH LOCAL REQUIREMENTS. PROPERLY DISPOSE OF CONSTRUCTION MATERIALS
  - IF VEGETATIVE SEED MIXES ARE SPECIFIED. SEEDING MUST TAKE PLACE NO LATER THAT SEPTEMBER 1: THE TYPE AND PERCENTAGES OF SEED IN THE MIX
  - ALL PUMPING OF SEDIMENT LADEN WATER SHALL BE DISCHARGED OVER AN UNDISTURBED, PREFERABLY VEGETATED AREA, AND THROUGH A SEDIMEN'
  - ALL EXPOSED SOILS MUST BE COVERED DURING THE WET WEATHER PERIOD. OCTOBER 01 MAY 31.
  - THE 50-FOOT ZONE FOR THE DURATION OF THE PERMIT COVERAGE, OR MAINTAIN LESS THAN THE ENTIRE EXISTING NATURAL BUFFER AND PROVIDE ADDITIONAL EROSION AND SEDIMENT CONTROL BMPS.
  - ALL LAND AREA PROPOSED FOR EXCAVATION, VEGETATION REMOVAL, SOIL STOCKPILING, OR WHICH WILL HAVE EXPOSED SOIL SHALL BE

  - REMAINS EXPOSED FOR 21 DAYS OR MORE DURING CONSTRUCTION SHALL BE TREATED WITH AN EROSION CONTROL COVER (I.E., PLASTIC. SEEDING OR MULCHING), FOLLOWING GRADING OR CONSTRUCTION, UNTIL SOILS ARE RE-VEGETATED OR OTHERWISE STABILIZED.
  - REMAINS EXPOSED FOR 7 DAYS OR MORE DURING CONSTRUCTION SHALL BE TREATED WITH AN EROSION CONTROL COVER (I.E., PLASTIC,

  - CITY STAFF PRIOR TO ANY DEVELOPMENT OR SITE PREPARATION. NO OTHER VEHICULAR ENTRANCE OR EXIT BY USED TO ACCESS THE DEVELOPMENT SITE.
  - VEHICLE WHEELS, SUCH THAT MUD TRACKING IS EVIDENT OFF SITE, ADDITIONAL MEASURES MUST BE TAKEN.
  - TOPSOIL REMOVED FOR DEVELOPMENT SHALL BE STOCKPILED AND REUSED TO THE DEGREE NECESSARY TO RESTORE DISTURBED AREAS TO
  - THE OWNER SHALL BE RESPONSIBLE FOR THE PROMPT CLEAN-UP OF ALL SEDIMENTS THAT ARE CARRIED ONTO ANY PUBLIC OR PRIVATE STREETS. OR ONTO ADJACENT PROPERTY AS SOON AS THE OWNER BECOMES AWARE OF SUCH PROBLEMS OR WITHIN THE TIME REQUIRED BY THE CITY. THE OWNER SHALL BE RESPONSIBLE FOR CLEANING AND REPAIRING STREETS, CATCH BASINS, DRAINAGE WAYS, STORM WATER DRAINAGE FACILITIES, AND ADJACENT PROPERTIES CONTAMINATED OR DAMAGED BY SEDIMENT, FAILURE TO DO SO WILL BE IN VIOLATION OF

ESTARLISH MATERIAL AND WASTE STORAGE AREAS, AND OTHER NON-STORMWATER CONTROLS

UNPAVED ROADS LOCATED ONSITE, OR USE AN EXIT TIRE WASH. THESE BMPS MUST BE IN PLACE PRIOR TO LAND-DISTURBING ACTIVITIES

USE BMPS TO PREVENT OR MINIMIZE STORMWATER EXPOSURE TO POLLUTANTS FROM SPILLS; VEHICLE AND EQUIPMENT FUELING, MAINTENANCE, AND

ENT THE FOLLOWING BMPS WHEN APPLICABLE: WRITTEN SPILL PREVENTION AND RESPONSE PROCEDURES, EMPLOYEE TRAIL

MATERIAL DELIVERY AND STORAGE CONTROLS. TRAINING AND SIGNAGE, AND COVERED STORAGE AREAS FOR WASTE AND SUPPLIES.

THE APPLICATION RATE OF FERTILIZERS USED TO REESTABLISH VEGETATION MUST FOLLOW MANUFACTURER'S RECOMMENDATIONS TO MINIMIZE NUTRIEN RELEASES TO SURFACE WATERS. EXERCISE CAUTION WHEN USING TIME-RELEASE FERTILIZERS WITHIN ANY WATERWAY RIPARIAN ZONE.

SURFACE WATERS OR CONVEYANCE SYSTEMS LEADING TO SURFACE WATERS.

- REMOVAL.
- RELEASE AND IMPLEMENT STEPS TO PREVENT A RECURRENCE OF THE DISCHARGE WITHIN THE SAME 24 HOURS.
- PICKUP MUST BE USED TO CLEANUP RELEASED SEDIMENTS.
- AND WASTE, INCLUDING SEDIMENT RETAINED BY TEMPORARY BMPS MUST BE IDENTIFIED ON THE PLANS.
- CONTROL BMP I.E. (FILTER BAG).
- IF WATERS OF THE STATE IS WITHIN THE PROJECT SITE OR WITHIN 50 FEET OF THE PROJECT BOUNDARY, MAINTAIN THE EXISTING NATURAL BUFFER WITHIN
- DISCHARGE OF SEDIMENT LADEN WATER FROM SITE TO PUBLIC STORM SYSTEM IS NOT ALLOWED. TREATMENT REQUIRED PRIOR TO DISCHARGE
- CONSIDERED PART OF THE DEVELOPMENT SITE. MAY 1 THROUGH SEPTEMBER 30. THE DURATION OF SOIL EXPOSURE SHALL BE KEPT TO A MAXIMUM OF 21 DAYS. ALL DISTURBED SOIL THAT
- OCTOBER 1 THROUGH APRIL 30, THE DURATION OF SOIL EXPOSURE SHALL BE KEPT TO A MAXIMUM OF 7 DAYS. ALL DISTURBED SOIL THAT
- SEEDING OR MULCHING), FOLLOWING GRADING OR CONSTRUCTION, UNTIL SOILS ARE REVEGETATED OR OTHERWISE STABILIZED 32. DURING CONSTRUCTION, RUNOFF FROM THE DEVELOPMENT SITE SHALL BE CONTROLLED, AND RUNOFF AND SEDIMENT RESULTING
- 33 A STABILIZED PAD OF GRAVEL SHALL BE LAID AND MAINTAINED AT ALL ENTRANCES AND EXITS TO ANY DEVELOPMENT SITE FROM WHICH VEHICULAR TRAFFIC MAY TRACK SOIL OR DEBRIS ONTO PUBLIC RIGHT-OF-WAY. THE GRAVEL PAD(S) SHALL BE INSTALLED AND INSPECTED BY
- GRAVEL PADS SHALL BE MAINTAINED TO FUNCTION PROPERLY. IF THE GRAVEL PAD DOES NOT ADEQUATELY REMOVE DIRT AND MUD FROM THE
- THEIR ORIGINAL OR ENHANCED CONDITION, OR TO ASSURE A MINIMUM OF SIX INCHES OF STABLE TOPSOIL FOR REVEGETATION.

HOSPITALITY

COVE

CREEK

OCK

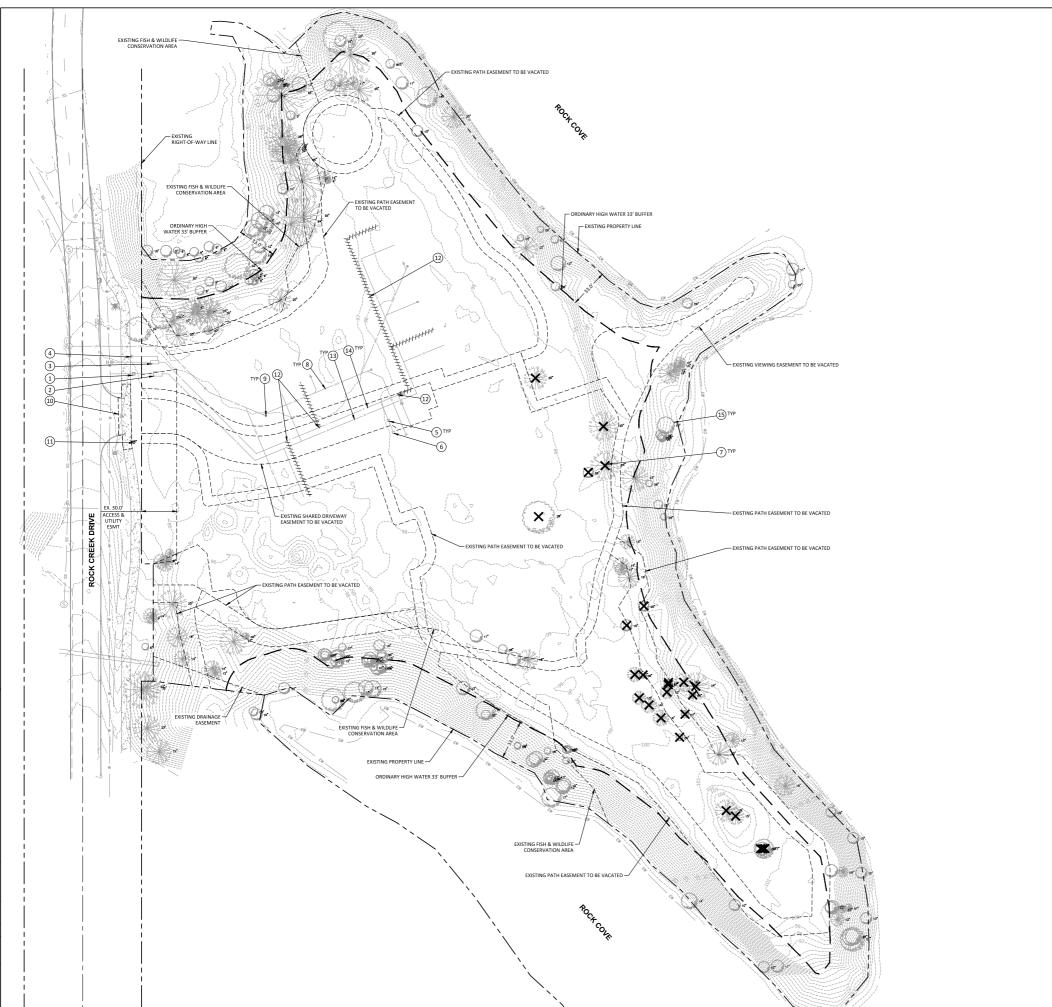
GENERAL NOTES

STEVENSON, WASHINGTON

NOT FOR CONSTRUCTION







#### **CONSTRUCTION NOTES:**

- 2 EXISTING WATER DCVA TO REMAIN. PROTECT IN PLACE.
- 3 EXISTING FIRE SERVICE AND DCDA TO REMAIN. PROTECT IN PLACE
- (4) EXISTING FDC TO REMAIN. PROTECT IN PLACE.
- 5 EXISTING WATER VALVE TO REMAIN AND BE PROTECTED. LID TO BE ADJUSTED TO FINISH GRADE AS NECESSARY.
- 6) EXISTING PRIVATE FIRE HYDRANT TO REMAIN. PROTECT IN PLACE.
- 7) EXISTING TREE TO BE REMOVED.
- 8 EXISTING SANITARY SEWER LINE TO REMAIN. PROTECT IN PLACE.
- (9) EXISTING SANITARY CLEANOUT TO REMAIN AND BE PROTECTED. LID TO BE ADJUSTED TO FINISH GRADE AS
- 11 REMOVE EXISTING SIGN AND POST.
- $\widetilde{\overline{\bf 12}}$  existing fire service to be capped at existing tee and removed or abandoned
- (13) EXISTING PRIVATE WATER LINE TO REMAIN. PROTECT IN PLACE.  $\stackrel{\smile}{14}$  existing private fire line to remain. Protect in place.
- 15) EXISTING TREE TO REMAIN AND BE PROTECTED.

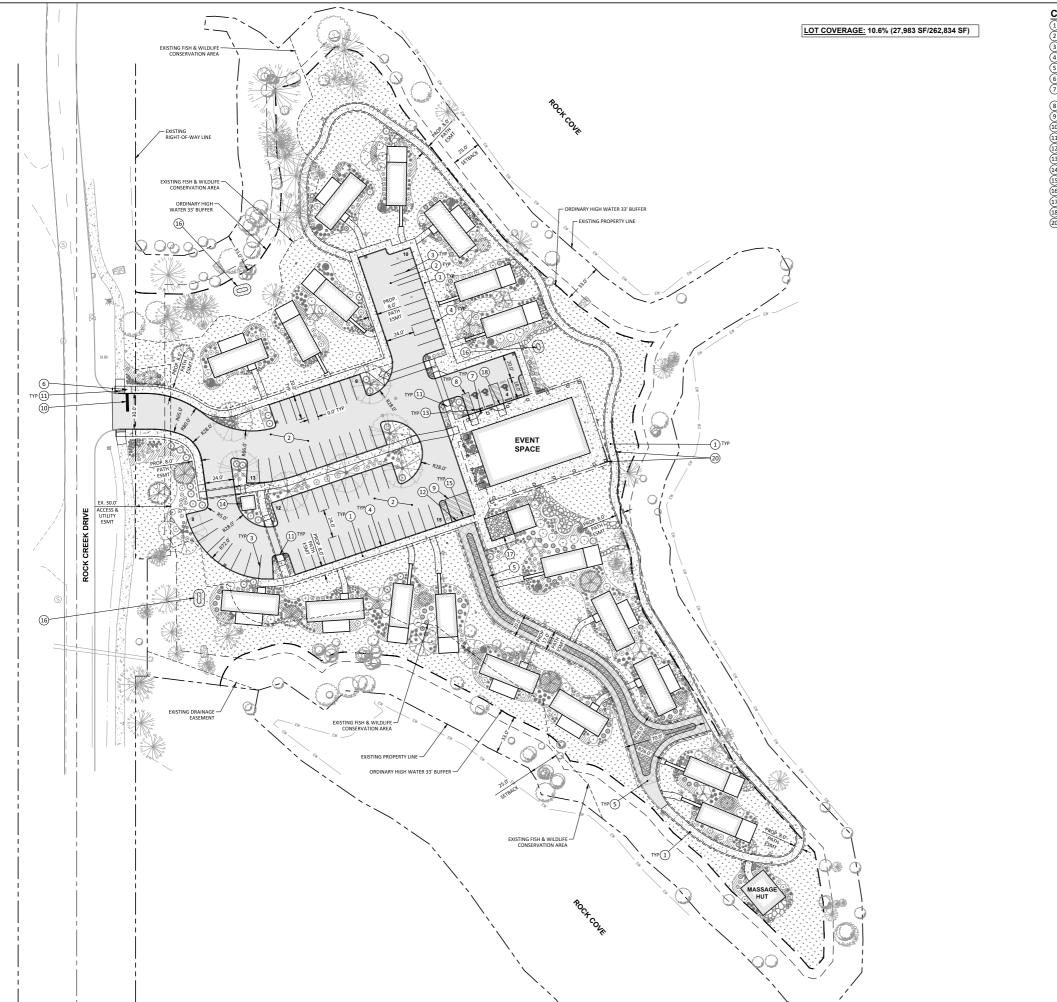
OVERALL EXISTING CONDITIONS & DEMO PLAN ROCK CREEK COVE HOSPITALITY STEVENSON, WASHINGTON

NOT FOR CONSTRUCTION

	DESIGNED: HHPR	DRAWN: HHPR	снескер: ННРR		DATE: 05/18/2023	
				DESCRIPTION	R E V I S I O N S	
DESCRIPTION R E V I S I O N S				NO.		
				DATE		







#### **CONSTRUCTION NOTES:**

- ) CONSTRUCT AC PAVEMENT PER SECTIONS ON SHEET C6.00.
  - 3) PAINT 4" WHITE PARKING STRIPE PER DETAILS ON SHEET C6.00.
  - CONSTRUCT STANDARD CONCRETE 6" CURB PER DETAIL ON SHEET C6.00. 5) CONSTRUCT FIRE ACCESS AND PEDESTRIAN PATH PER SECTION ON SHEET C6.00.
- (7) INSTALL ACCESSIBLE PARKING SIGN PER DETAILS ON SHEET C6.00. VAN ACCESSIBLE SIGN TO BE INSTALLED WHERE SHOWN ON PLANS.
- PAINT "NO PARKING" STRIPING PER DETAIL ON SHEET C6.00.

- INSTALL CONCRETE WHEELSTOP PER DETAIL ON SHEET C6.00.
- 15) INSTALL REMOVABLE BOLLARD WITH KNOX PADLOCK PER DETAIL ON SHEET C6.00.
- 16) PROPOSED STORMWATER FACILITY. REFER TO UTILITY PLANS FOR INFORMATION.
- ) PROPOSED STORAGE AREA AND GAZEBO. REFER TO ARCHITECTURAL PLANS FOR INFORMATION (18) CONSTRUCT FLUSH CURB PER DETAIL ON SHEET C6.00.
- (20) PROPOSED SIDEWALK THICKENED EDGE PER DETAIL ON SHEET C6.00.

OVERALL SITE PLAN
ROCK CREEK COVE HOSPITALITY
STEVENSON, WASHINGTON

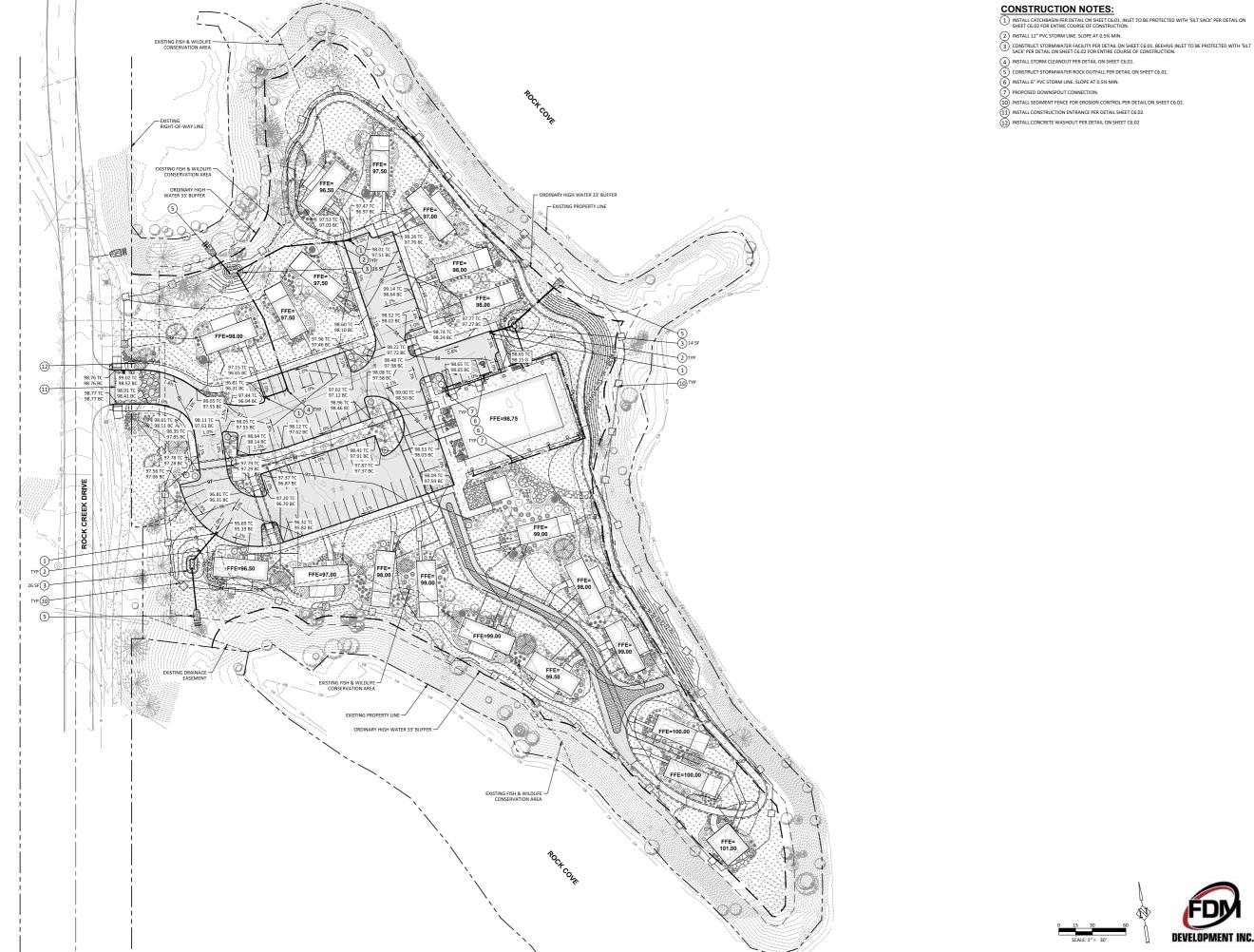
NOT FOR CONSTRUCTION

DESIGNED: HHPR	DRAWN:	HHPR	CHECKED:	HHPR	-	DATE: 05/18/2023	
					DESCRIPTION	REVISIONS	
			L		Ñ.		
					DATE		





FDM-01A



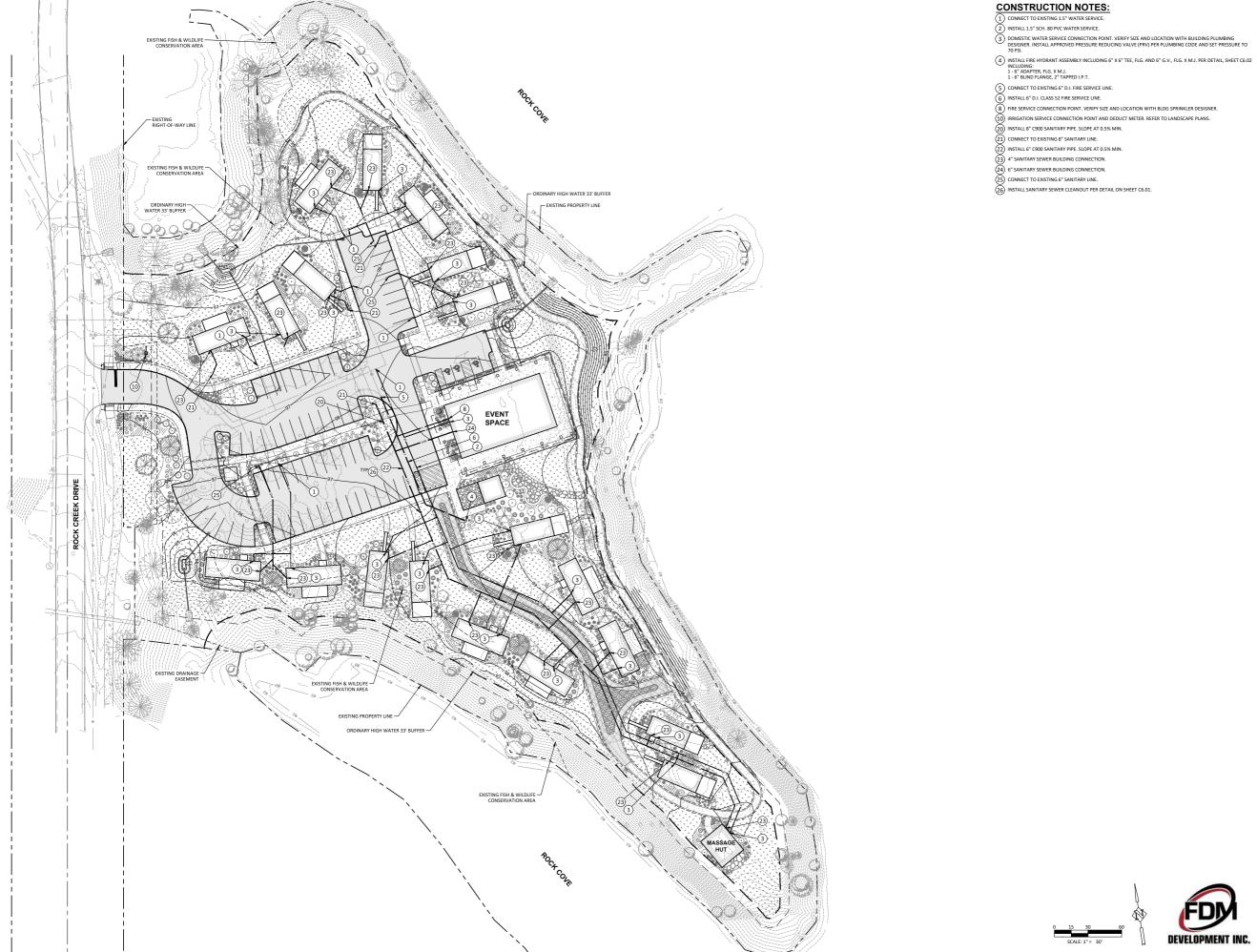
OVERALL GRADING, DRAINAGE & EROSION CONTROL PLAN
ROCK CREEK COVE HOSPITALITY
STEVENSON, WASHINGTON

NOT FOR CONSTRUCTION





C4.00 DB NO. FDM-01A



RRIGATION SERVICE CONNECTION POINT AND DEDUCT METER. REFER TO LANDSCAPE PLANS.

OVERALL UTILITY PLAN
ROCK CREEK COVE HOSPITALITY
STEVENSON, WASHINGTON

NOT FOR CONSTRUCTION



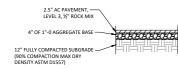


DB NO. FDM-01A

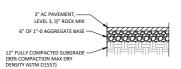
- LOTES:

  CONCRETE SHALL BE COMMERCIAL MIX, MIN. COMPRESSIVE STRENGTH OF 3300 PSI @ 28 DAYS, WITH A SLUMP RANGE OF 1½" MIN. TO 3" MAX.
  SIDEWALK PARIELS TO BE SQUARE (\$ LONG x 5" WIDE TYP.).
  EVANASION, DIVINITY TO BE PALECE OF A TISBLE OF ROWNEWAY APPROACHES, UTILITY VAULTS, CURB RAMMS, AND/OR POINTS OF FANCENCY IN CURB AS SHOWN ON THE STANDARD DRAWINGS FOR SIDEWALK PARMS, AND AT SPACHES ON TO DEVECTE HE STANDARD DRAWINGS FOR SIDEWALK PARMS, AND AT SPACHES ON TO DEVECTE HE STANDARD PROMINGS FOR SIDEWALK PARMS, AND AT SPACHES ON TO TO EXCELL HE STANDARD THE STANDARD SHOWN OF THE STANDARD PARMS OF THE SIDEWALK STANDARD SHOWN OF THE STANDARD PARMS OF THE STANDARD SHOWN OF THE ST

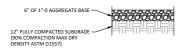
### STANDARD SIDEWALK



#### AC PAVEMENT SECTION FOR PARKING LOT STALLS



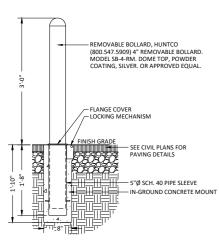
#### AC PAVEMENT SECTION FOR PARKING LOT AISLES



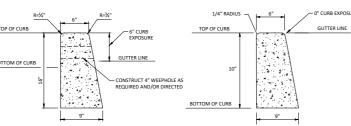
#### **GRAVEL FIRE TURNAROUND SECTION**



#### SIGN LEGEND



#### REMOVABLE BOLLARD DETAIL

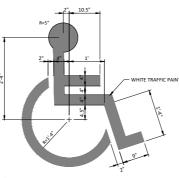


#### STANDARD CURB

**FLUSH CURB** 

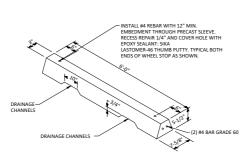
- CURBS ADJACENT TO PAVEMENT OR SIDEWALK IO HAVE EXPANSION ARMY ON COMMISSION OF THE ACTION OF THE ACT

#### **ACCESSIBLE PARKING SIGN**

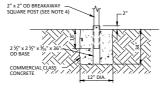


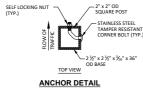
#### ACCESSIBLE PARKING SYMBOL

STOP BAR

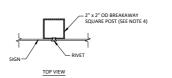


#### PRECAST CONCRETE WHEEL STOP

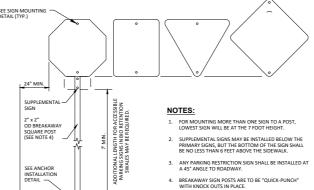




#### ANCHOR INSTALLATION DETAIL



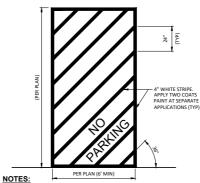
### SIGN MOUNTING DETAIL



#### **GROUND MOUNTED SIGN DETAILS**

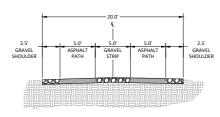


#### SIDEWALK THICKENED EDGE

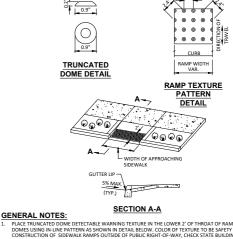


ACCEPTABLE TRAFFIC PAINTS:
FULLER O'BRIEN TRAFFIC LINE PAINT, 382-12
GENERAL. TRU-L'EST SUPREME ZONE MARKING PAINT, 1010 WHITE AND 1012 YELLOW
POE INDUSTRIES: PITSBURGH TRAFFIC AND ZONE MARKING PAINT 22 LINE, WHITE AND YELLOW
RODDA: TRAFFIC PAINT, WHITE 671 AND YELLOW 870
SHEWIN WILLIAMS: WHITE 287 US AND YELLOW 829 Y2

### ACCESSIBLE AISLE & NO PARKING AREA STRIPING



#### ASPHALT TRAIL/FIRE ACCESS LANE SECTION



GENERAL NOTES:

1. PLACE TRUNCATED DOME DETECTABLE WARNING TEXTURE IN THE LOWER 2' OF THROAT OF RAMP ONLY. ABRANGE DOMES USING IN-LINE PATTERN AS SHOWN IN DETAIL BELOW. COLOR OF TEXTURE TO BE SAFETY YELLOW. FOR CONSTRUCTION OF SIDEWALK RAMPS OUTSIDE OF PUBLIC RIGHTO-O-PWAY, CHECK STATE BUILDING CODE REQUIREMENTS.

2. SIDEWALK CURB RAMP SLOPES SHOWN ARE RELATIVE TO THE TRUE LEVEL HORIZON (ZERO BUBBLE).

3. IN ALTERATIONS, CURB RAMP SLOPES SHOWN ARE RELATIVE TO THE TRUE LEVEL HORIZON (ZERO BUBBLE).

4. SIDEWALK CURB RAMP SLOPES SHOWN ARE RELATIVE TO THE TRUE LEVEL HORIZON (ZERO BUBBLE).

5. IN ALTERATIONS, CURB RAMP SLOPES SHOWN ARE RELATIVE TO THE TRUE LEVEL HORIZON (ZERO BUBBLE).

6. SIDEWALK STATE ARE NOT PART OF THE PATH OF TRAYEL MAY BE ANY SLOPE.

7. FOR PATHS INTERSECTING A ROADWAY TO A SIDEWALK, THE RAMP SHOULD BE 8' WIDE WITH NO TEXTURING.

7. FOR THE PURPOSE OF THIS DRAWING, A CURB SRAMP IS CONSIDERED "PERPENDICULAR" IF HE ANGE BETWEEN THE CONSIDERAL ANS OF THE RAMP CAN AND SCHOLE TO THE CURB CONTRICTION AS OF THE SAMP CONTRINE AS SOFT THE TRAYER CONTRINENTS. TO GREATER.

9. SIDEWALK FLARE IS NOT NECESSARY WHERE THE RAMP IS PROTECTED FROM PEDESTRIAN CROSS-TRAVEL.

- PER PLAN (8.5' MIN)

STANDARD PARKING STALL STRIPING

LOW PROFILE MOUNTABLE CONCRETE CURB

#### SIDEWALK RAMP

Harper Houf Peterson Righellis Inc.

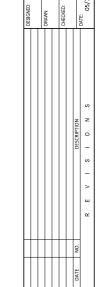
CREEK COVE HOSPITALITY

ROCK

DETAILS









C6.00

#### NOTES:

DO NOT EXCAVATE THE BIORETENTION FACILITY DURING WET OR SATURATED CONDITIONS.

EXCAVATION SHOULD BE PERFORMED BY MACHINERY OPERATING ADJACENT TO THE BIORETENTION FACILITY AND NO HEAVY EQUIPMENT WITH NARROW TRACKS, NARROW TO OR LARGE LUGGED, HIGH PRESSURE TIRES SHOULD BE ALLOWED ON THE BOTTOM OF THE BIORETENTION FACILITY.

- PRIOR TO PLACEMENT OF THE BSM, THE FINISHED SUBGRADE SHALL:

  BE SCARIFIED TO A MINIMUM DEPTH OF 3 INCHES.
  HAVE ANY SEDIMENT DEPOSITIED FROM CONSTRUCTION RUNOFF REMOVED. TO REMOVE ALL INTRODUCED SEDIMENT, SUBGRADE SOIL SHOULD BE REMOVED TO A DEPTH OF 3-6 INCHES AND REPLACED WITH BSM.

  BE INSPECTED BY THE RESPONSIBLE ENGINEER TO VERIFY REQUIRED SUBGRADE CONDITIONS.

COMPACT THE BIORETENTION SOIL MIX TO A RELATIVE COMPACTION OF 85 PERCENT OF MODIFIED MAXIMUM BRY DENSITY (ASTM D 1557). COMPACTION CAN BE ACHIEVED BY BOOT PACKING (SIMPLY WALKING OVER ALL AREAS OF FEACH LIFT), AND THEN APPLYO 2. INCHES (0.5 CM) OF WATER PER 3 INCH (2.5 CM) OF BIORETENTION SOIL MIX DEPTH. WATER FOR SETTLING SHOULD BE APPLIED BY SRAVING ON SETRINGLING.

CONSTRUCTION ON BIORETENTION FACILITIES SHOULD NOT BEGIN UNTIL ALL CONTRIBUTING DRAINAGE AREAS ARE STABILIZED ACCORDING TO EROSION AND SEDIMENT CONTROL BMP'S AND TO THE SATISFACTION OF THE ENGINEER.

AT INLET. WARP GUTTER TO MEET NORMAL GUTTER SLOPE 36" AT EACH END OF INLET

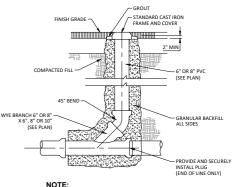
NOTES:

NOTES:

1 1/2" OPENING

SECTION A-A

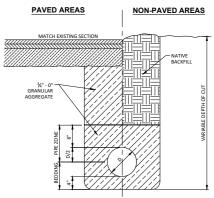
**CONCRETE CATCH BASIN GB-2** 



NOTE:

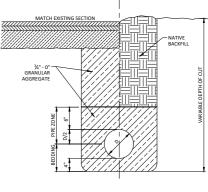
NOTE: CAST IRON FRAME AND COVER SHALL
BE VALLEY IRON & STEEL CO. NO.'S 202 (6") OR
203 (8") OR APPROVED EQUAL.

### STANDARD CLEANOUT





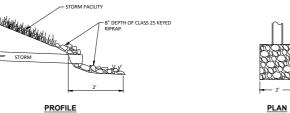
### PIPE BEDDING AND BACKFILL TYPICAL SECTION





## **BEEHIVE OVERFLOW INLET DETAIL**

24" NYLOPLAST CAST IRON DOME LOCKING GRATE



100 Jan 1990

NYLOPLAST PVC AREA DRAIN BASIN (OR APPROVED EQUAL)

NYLOPLAST ENVIROHOOD (OR APPROVED EQUAL)



C6.01

FDM-01A

NOT FOR

CONSTRUCTION

ROCK CREEK COVE HOSPITALITY

DETAILS

STEVENSON, WASHINGTON

**ROCK OUTFALL** D = PIPE DIAMETER

W = BOTTOM WIDTH OF CHANNEL

MACHINE PLACE 200-300lb BOULDERS OVER WOVEN GEOTEXTILE BLANKET OR

P = WETTED PERIMETER OF CHANNEL

#### NOTES:

- 1. SEE PLAN FOR CLASS OF RIP RAP
  REQUIRED AT OUTLET.
  2. MINIMUM DEPTH OF RIP RAP SHALL
  BE 11/2 TIMES AVERAGE STONE SIZE.
  A ROCKS SHALL BE PLACED TO PROVIDE
  A MINIMUM OF VOIDS.
  4. SURFAGE ROCKE SUMMANDER.

  SURFAGE ROCKE SUMMANDER.

  SURFAGE ROCKE SUMMANDER.

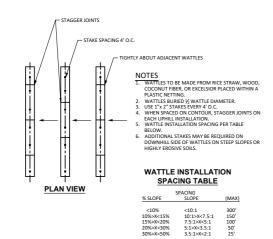
  **TOTAL PROVINCE SU
- A MINIMUM OF VOIDS. SURFACE ROCKS SHALL PROTRUDE AT LEAST 1/2 THEIR VERTICAL
- DIMENSION.
  RIP RAP SHALL BE PLACED OVER A
  GEOTEXTILE FABRIC ON A
  NATURAL BEDDING, OR IT MAY
  GROUTED OR PLACED OVER A GRAVEL
  BEDDING AS REQUIRED BY THE

#### RIPRAP AT STORM PIPE OUTLET



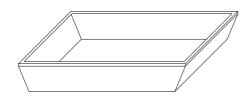
**BIOFILTRATION SWALE INLET PIPE OUTFALL** 

PROFILE

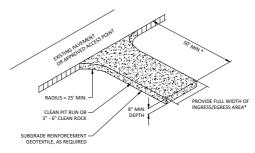


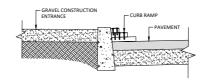
SECTION

#### **EROSION AND SEDIMENT CONTROL WATTLE DETAIL**



#### **CONCRETE TRUCK WASHOUT BASIN**





#### NOTES:

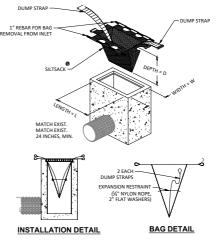
- IN LEST.

  THE ENTRANCE SHALL BE MAINTAINED IN A CONDITION THAT WILL PREVENT TRACKING OR FLOWING OF SEDIMENT ONTO PUBLIC RIGHT-OF-WAYS. THIS MAY REQUIRE TOP DRESSING, REPAIR AND/OR CLEAN OLD TOF ANY MEASURES USED TO TRAP SEDIMENT.

  WHEN NECESSARY, WHEELS SHALL BE CLEANED PRIOR TO ENTRANCE ONTO PUBLIC RIGHT-OF-WAY.
  WHEN NASHING IS REQUIRED, IT SHALL BE DONE ON AN AREA STRAILEDE WITH CRUSHED STONE
  THAT DRAINS INTO AN APPROVED SEDIMENT TRAP OR SEDIMENT BASIN.
  WHERE RUNDOFF CONTAINING SEDIMENT LODGE WHERE IS LEVEN'DE THE SITE VIA THE CONSTRUCTION
  ENTRANCE, OTHER MESSURES SHALL BE IMPLEMENTED TO DIVERT RUNDOFF THROUGH AN
  DIMENSIONS.

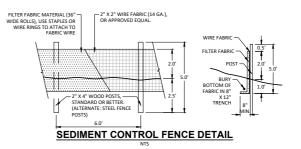
- APPROVED HILERINGS ...
  DIMENSIONS
  SINGLE FAMILY
  20 LONG SP 20' WIDE 8" DEEP OF 3/4" MINUS CLEAN ROCK.
  COMMERCIAL
  50' LONG SP 20' WIDE 3-6" CLEAN ROCK,
  GOVERNING AUTHORITY MAY REQUIRE GEOTEXTILE FABRIC TO PREVENT SUB-SOIL PUMPING.

#### **CONSTRUCTION ENTRANCE DETAIL**



NOTE: INSTALL SILTSACK AND ACCESSORIES AS SUPPLIED BY ACF WEST, INC., PH. 771-5115, OR APPROVED EQUAL.

#### "SILT SACK" INLET PROTECTION



ROCK CREEK COVE HOSPITALITY STEVENSON, WASHINGTON

DETAILS

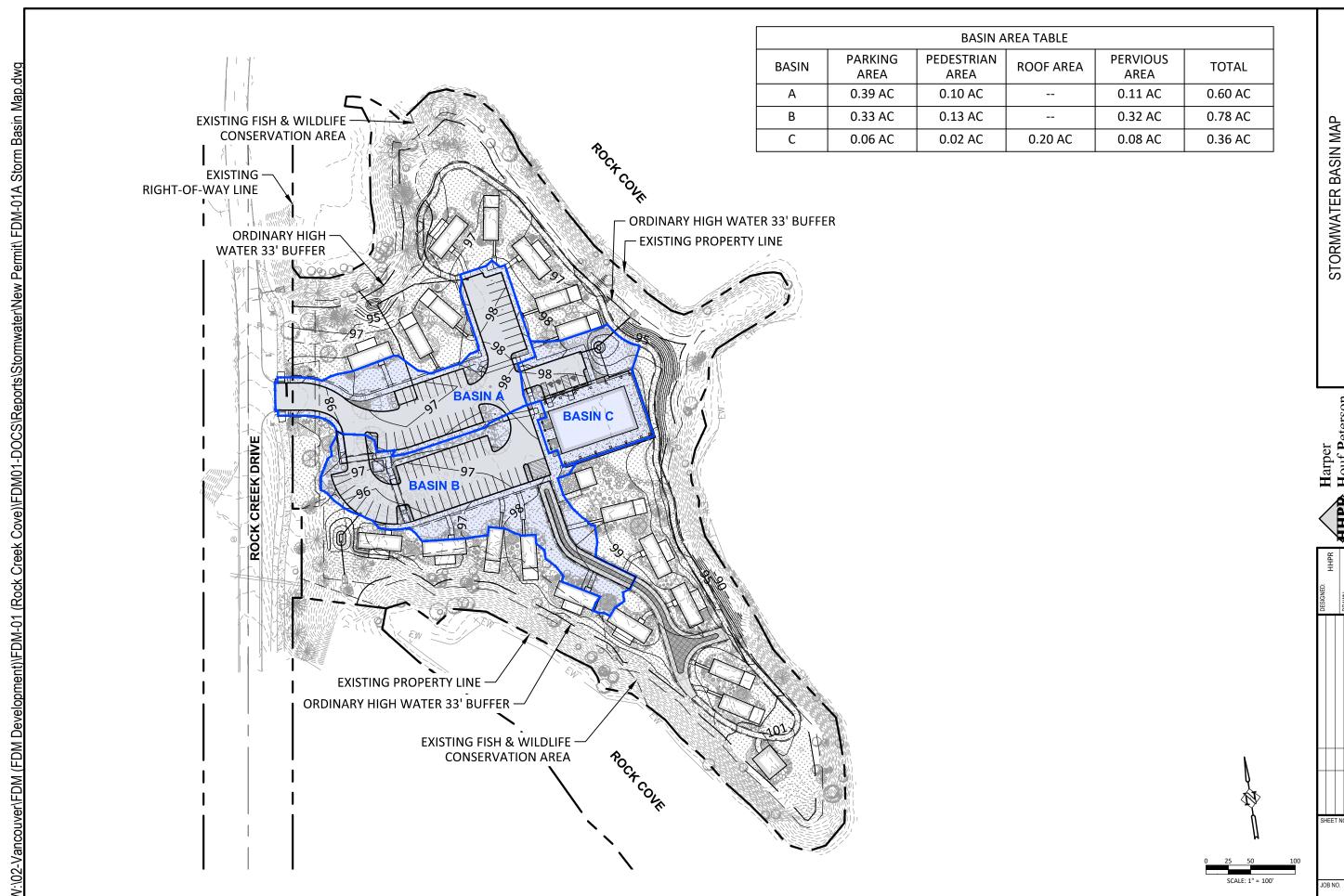
NOT FOR CONSTRUCTION

C6.02

FDM-01A

DEVELOPMENT INC.

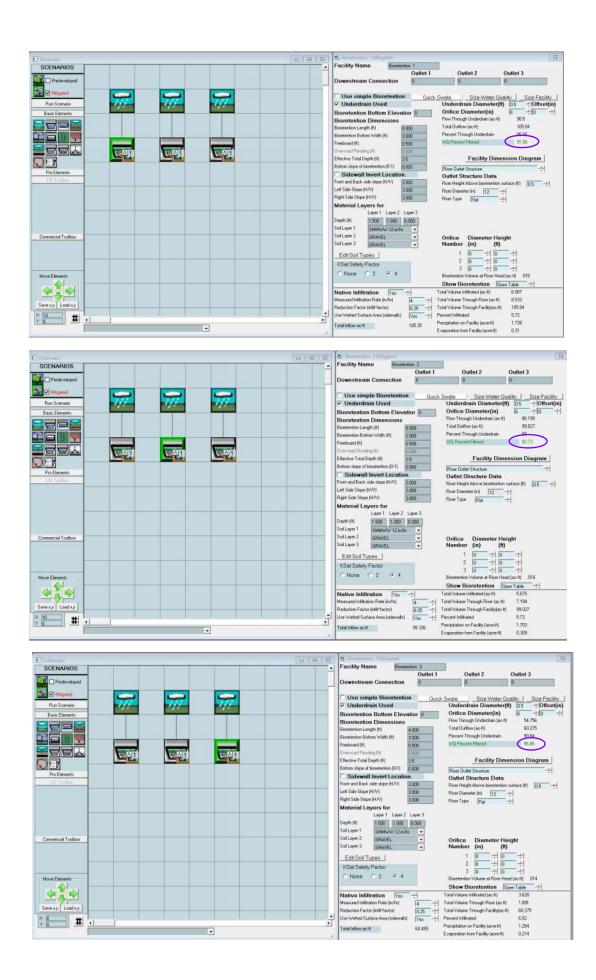
<u>APPENDIX 3</u> – Stormwater Calculations and Design Information



**CREEK COVE HOSPITALITY** STEVENSON, WASHINGTON ROCK (

FDM-01A

# WWHM2012 PROJECT REPORT



# General Model Information

Project Name: FDM-01 Rock Creek WWHM

Site Name: Rock Ck Cove

Site Address:

City: Steveson
Report Date: 5/18/2023
Gage: Portland
Data Start: 1948/10/01
Data End: 2009/09/30
Timestep: 15 Minute

Precip Scale: 0.000 (adjusted)

Version Date: 2019/09/13

Version: 4.2.17

#### **POC Thresholds**

Low Flow Threshold for POC1: 50 Percent of the 2 Year

High Flow Threshold for POC1: 50 Year

# Landuse Basin Data Predeveloped Land Use

### Mitigated Land Use

#### Basin A

Bypass: No

GroundWater: No

Pervious Land Use acre A B, Lawn, Flat 0.11

Pervious Total 0.11

Impervious Land Use acre SIDEWALKS FLAT 0.1 PARKING FLAT 0.39

Impervious Total 0.49

Basin Total 0.6

Element Flows To:

Surface Interflow Groundwater

Surface retention 1 Surface retention 1

FDM-01 Rock Creek WWHM 5/18/2023 4:42:36 PM Page 115

#### Basin B

Bypass: No

GroundWater: No

Pervious Land Use acre A B, Lawn, Flat 0.32

Pervious Total 0.32

Impervious Land Use acre SIDEWALKS FLAT 0.13 PARKING FLAT 0.33

Impervious Total 0.46

Basin Total 0.78

Element Flows To:

Surface Interflow Groundwater

Surface retention 2 Surface retention 2

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#### Basin 3

Bypass: No

GroundWater: No

Pervious Land Use acre A B, Lawn, Flat 0.08

Pervious Total 0.08

Impervious Land Use acre
ROOF TOPS FLAT 0.2
SIDEWALKS FLAT 0.02
PARKING FLAT 0.06

Impervious Total 0.28

Basin Total 0.36

Element Flows To:

Surface Interflow Groundwater

Surface retention 3 Surface retention 3

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# Routing Elements Predeveloped Routing

#### Mitigated Routing

#### Bioretention 1

Bottom Length: 8.00 ft.
Bottom Width: 3.00 ft.
Material thickness of first layer: 1.5

Material type for first layer: SMMWW 12 in/hr

Material thickness of second layer:

Material type for second layer: GRAVEL

Material thickness of third layer: 0

Material type for third layer: GRAVEL

Infiltration On

Infiltration rate: 4
Infiltration safety factor: 0.25

Wetted surface area On

Total Volume Infiltrated (ac-ft.):

Total Volume Through Riser (ac-ft.):

Total Volume Through Facility (ac-ft.):

Percent Infiltrated:

Total Precip Applied to Facility:

Total Evap From Facility:

6.007

8.533

105.04

105.04

1.738

0.31

Underdrain used

Underdrain Diameter (feet):
Orifice Diameter (in.):
Offset (in.):
Flow Through Underdrain (ac-ft.):
Total Outflow (ac-ft.):
Percent Through Underdrain:

0.5
0
90.5
105.04

Discharge Structure

Riser Height: 0.5 ft. Riser Diameter: 12 in.

Element Flows To:

Outlet 1 Outlet 2

#### Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	
0.0000	0.0095	0.0000	0.0000	0.0000
0.0385	0.0095	0.0000	0.0000	0.0000
0.0769	0.0093	0.0000	0.0000	0.0000
0.1154	0.0091	0.0000	0.0000	0.0000
0.1538	0.0089	0.0000	0.0000	0.0000
0.1923	0.0087	0.0001	0.0000	0.0000
0.2308	0.0084	0.0001	0.0000	0.0000
0.2692	0.0082	0.0001	0.0000	0.0000
0.3077	0.0080	0.0001	0.0000	0.0000
0.3462	0.0078	0.0001	0.0001	0.0000
0.3846	0.0076	0.0002	0.0001	0.0000
0.4231	0.0075	0.0002	0.0001	0.0000
0.4615	0.0073	0.0002	0.0002	0.0000
0.5000	0.0071	0.0002	0.0002	0.0000
0.5385	0.0069	0.0003	0.0003	0.0000
0.5769	0.0067	0.0003	0.0004	0.0000
0.6154	0.0065	0.0003	0.0005	0.0000
0.6538	0.0063	0.0003	0.0006	0.0000
0.6923	0.0062	0.0004	0.0006	0.0000

0.7308	0.0060	0.0004	0.0007	0.0000
0.7692 0.8077	0.0058 0.0056	0.0005 0.0005	0.0009 0.0011	0.0000 0.0000
0.8462	0.0055	0.0005	0.0011	0.0000
0.8846	0.0053	0.0006	0.0015	0.0000
0.9231	0.0052	0.0006	0.0018	0.0000
0.9615	0.0050	0.0007	0.0021	0.0000
1.0000	0.0048	0.0007	0.0022	0.0000
1.0385	0.0047	0.0008	0.0025	0.0000
1.0769	0.0045	0.0008	0.0029	0.0000
1.1154	0.0044	0.0009	0.0033	0.0000
1.1538	0.0042	0.0009	0.0038	0.0000
1.1923	0.0041	0.0010	0.0043	0.0000
1.2308	0.0039	0.0011	0.0049	0.0001
1.2692 1.3077	0.0038 0.0037	0.0011 0.0012	0.0055	0.0001 0.0001
1.3462	0.0037	0.0012	0.0058 0.0065	0.0001
1.3846	0.0033	0.0013	0.0073	0.0001
1.4231	0.0034	0.0013	0.0073	0.0001
1.4615	0.0031	0.0015	0.0091	0.0001
1.5000	0.0030	0.0016	0.0101	0.0001
1.5385	0.0029	0.0017	0.0112	0.0001
1.5769	0.0028	0.0017	0.0120	0.0001
1.6154	0.0027	0.0018	0.0128	0.0002
1.6538	0.0025	0.0019	0.0141	0.0002
1.6923	0.0024	0.0020	0.0155	0.0002
1.7308	0.0023	0.0021	0.0225	0.0003
1.7692	0.0022	0.0022	0.0232	0.0003
1.8077	0.0021	0.0023	0.0238	0.0003
1.8462	0.0020	0.0024	0.0245	0.0004
1.8846 1.9231	0.0019 0.0018	0.0025 0.0026	0.0252 0.0259	0.0004 0.0004
1.9615	0.0018	0.0020	0.0259	0.0004
2.0000	0.0017	0.0027	0.0200	0.0004
2.0385	0.0015	0.0029	0.0273	0.0005
2.0769	0.0014	0.0030	0.0288	0.0005
2.1154	0.0013	0.0031	0.0295	0.0005
2.1538	0.0013	0.0032	0.0303	0.0005
2.1923	0.0012	0.0034	0.0311	0.0006
2.2308	0.0011	0.0035	0.0318	0.0006
2.2692	0.0010	0.0036	0.0326	0.0006
2.3077	0.0009	0.0037	0.0334	0.0007
2.3462	0.0009	0.0039	0.0342	0.0007
2.3846	0.0008	0.0040	0.0350	0.0007
2.4231	0.0007	0.0042	0.0358	0.0008
2.4615 2.5000	0.0007 0.0006	0.0043 0.0045	0.0367 0.0375	0.0008 8000.0
2.5000	0.0006	0.0045	0.0375	0.0008
2.5000		0.0043 ia Tabla	0.0070	0.0000

Bioretention Hydraulic Table

# Stage(feet)Area(ac.)Volume(ac-ft.)Discharge(cfs)To Amended(cfs)Infilt(cfs)

2.5000	0.0095040.004462	0.0000	0.0294	0.0002
2.5385	0.0097230.004832	0.0000	0.0294	0.0004
2.5769	0.0099430.005210	0.0000	0.0316	0.0007
2.6154	0.0101670.005597	0.0000	0.0331	0.0009
2.6538	0.0103930.005992	0.0000	0.0347	0.0011
2.6923	0.0106210.006396	0.0000	0.0362	0.0014
2.7308	0.0108510.006809	0.0000	0.0379	0.0016

2.7692	0.0110840.007231	0.0000	0.0395	0.0018
2.8077	0.0113200.007662	0.0000	0.0413	0.0021
2.8462	0.0115580.008102	0.0000	0.0430	0.0023
2.8846	0.0117980.008551	0.0000	0.0448	0.0026
2.9231	0.0120410.009009	0.0000	0.0467	0.0028
2.9615	0.0122870.009477	0.0000	0.0486	0.0031
3.0000	0.0125340.009955	0.0000	0.0506	0.0033
3.0385	0.0127850.010441	0.0800	0.0526	0.0036
3.0769	0.0130370.010938	0.2257	0.0546	0.0038
3.1154	0.0132920.011444	0.4122	0.0567	0.0041
3.1538	0.0135500.011961	0.6273	0.0589	0.0043
3.1923	0.0138100.012487	0.8600	0.0611	0.0046
3.2308	0.0140720.013023	1.0991	0.0633	0.0049
3.2692	0.0143370.013569	1.3333	0.0656	0.0051
3.3077	0.0146050.014126	1.5516	0.0680	0.0054
3.3462	0.0148740.014693	1.7445	0.0704	0.0057
3.3846	0.0151470.015270	1.9054	0.0728	0.0060
3.4231	0.0154210.015858	2.0318	0.0754	0.0062
3.4615	0.0156980.016456	2.1274	0.0779	0.0065
3.5000	0.0159780.017066	2.2033	0.0806	0.0065

### Surface retention 1

Element Flows To: Outlet 1

Outlet 1 Outlet 2
Bioretention 1

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#### Bioretention 2

Bottom Length: 8.00 ft.
Bottom Width: 3.00 ft.
Material thickness of first layer: 1.5

Material type for first layer: SMMWW 12 in/hr

Material thickness of second layer:

Material type for second layer: GRAVEL

Material thickness of third layer: 0

Material type for third layer: GRAVEL

Infiltration On

Infiltration rate: 4
Infiltration safety factor: 0.25

Wetted surface area On

Total Volume Infiltrated (ac-ft.): 5.675
Total Volume Through Riser (ac-ft.): 7.194
Total Volume Through Facility (ac-ft.): 99.027
Percent Infiltrated: 5.73
Total Precip Applied to Facility: 1.703
Total Evap From Facility: 0.309

Underdrain used

Underdrain Diameter (feet): 0.5
Orifice Diameter (in.): 6
Offset (in.): 0

Flow Through Underdrain (ac-ft.):

Total Outflow (ac-ft.):

Percent Through Underdrain:

86.158
99.027
87

Discharge Structure

Riser Height: 0.5 ft. Riser Diameter: 12 in.

Element Flows To:

Outlet 1 Outlet 2

#### Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	
0.0000 0.0385	0.0095 0.0095	0.0000 0.0000	0.0000 0.0000	0.0000 0.0000
0.0363	0.0093	0.0000	0.0000	0.0000
0.1154	0.0091	0.0000	0.0000	0.0000
0.1538	0.0089	0.0000	0.0000	0.0000
0.1923	0.0087	0.0001	0.0000	0.0000
0.2308	0.0084	0.0001	0.0000	0.0000
0.2692	0.0082	0.0001	0.0000	0.0000
0.3077	0.0080	0.0001	0.0000	0.0000
0.3462	0.0078	0.0001	0.0001	0.0000
0.3846	0.0076	0.0002	0.0001	0.0000
0.4231	0.0075	0.0002	0.0001	0.0000
0.4615	0.0073	0.0002	0.0002	0.0000
0.5000	0.0071	0.0002	0.0002	0.0000
0.5385	0.0069	0.0003	0.0003	0.0000
0.5769	0.0067	0.0003	0.0004	0.0000
0.6154	0.0065	0.0003	0.0005	0.0000
0.6538	0.0063	0.0003	0.0006	0.0000
0.6923	0.0062	0.0004	0.0006	0.0000
0.7308	0.0060	0.0004	0.0007	0.0000
0.7692	0.0058	0.0005	0.0009	0.0000

0.8462         0.0055         0.0006           0.9231         0.0052         0.0006           0.9615         0.0050         0.0007           1.0000         0.0048         0.0007           1.0385         0.0047         0.0008           1.0769         0.0045         0.0008           1.1154         0.0044         0.0009           1.1538         0.0042         0.0009           1.1923         0.0041         0.0010           1.2692         0.0038         0.0011           1.3077         0.0037         0.0012           1.3462         0.0035         0.0013           1.3846         0.0034         0.0013           1.4231         0.0033         0.0014           1.4615         0.0031         0.0015           1.5000         0.0030         0.0016           1.5385         0.0029         0.0017           1.5769         0.0028         0.0017           1.6154         0.0027         0.0018           1.6923         0.0024         0.0020           1.7308         0.0025         0.0019           1.6923         0.0024         0.0022           1.8462         <	0.0015 0.0018 0.0021 0.0022 0.0025 0.0029 0.0033 0.0038 0.0049 0.0055 0.0058 0.0065 0.0073 0.0081 0.0011 0.0112 0.0120 0.0128 0.0141 0.0155 0.0225 0.0232 0.0238 0.0245 0.0252 0.0259 0.0266 0.0273 0.0281 0.0288 0.0273 0.0281 0.0288 0.0295 0.0288 0.0295 0.0303 0.0311 0.0318 0.0326 0.0334 0.0315 0.0315 0.0375 0.0375	0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0000 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001 0.0001
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# Stage(feet)Area(ac.)Volume(ac-ft.)Discharge(cfs)To Amended(cfs)Infilt(cfs)

2.5000	0.0095040.004462	0.0000	0.0294	0.0002
2.5385	0.0097230.004832	0.0000	0.0294	0.0004
2.5769	0.0099430.005210	0.0000	0.0316	0.0007
2.6154	0.0101670.005597	0.0000	0.0331	0.0009
2.6538	0.0103930.005992	0.0000	0.0347	0.0011
2.6923	0.0106210.006396	0.0000	0.0362	0.0014
2.7308	0.0108510.006809	0.0000	0.0379	0.0016
2.7692	0.0110840.007231	0.0000	0.0395	0.0018
2.8077	0.0113200.007662	0.0000	0.0413	0.0021

0.0115580.008102	0.0000	0.0430	0.0023
0.0117980.008551	0.0000	0.0448	0.0026
0.0120410.009009	0.0000	0.0467	0.0028
0.0122870.009477	0.0000	0.0486	0.0031
0.0125340.009955	0.0000	0.0506	0.0033
0.0127850.010441	0.0800	0.0526	0.0036
0.0130370.010938	0.2257	0.0546	0.0038
0.0132920.011444	0.4122	0.0567	0.0041
0.0135500.011961	0.6273	0.0589	0.0043
0.0138100.012487	0.8600	0.0611	0.0046
0.0140720.013023	1.0991	0.0633	0.0049
0.0143370.013569	1.3333	0.0656	0.0051
0.0146050.014126	1.5516	0.0680	0.0054
0.0148740.014693	1.7445	0.0704	0.0057
0.0151470.015270	1.9054	0.0728	0.0060
0.0154210.015858	2.0318	0.0754	0.0062
0.0156980.016456	2.1274	0.0779	0.0065
0.0159780.017066	2.2033	0.0806	0.0065
	0.011798 0.008551 0.012041 0.009009 0.012287 0.009477 0.012534 0.009955 0.012785 0.010441 0.013037 0.010938 0.013292 0.011444 0.013550 0.011961 0.013810 0.012487 0.014072 0.013023 0.014337 0.013569 0.014605 0.014126 0.014874 0.014693 0.015147 0.015270 0.015421 0.015858 0.015698 0.016456	0.011798 0.008551       0.0000         0.012041 0.009009       0.0000         0.012287 0.009477       0.0000         0.012534 0.009955       0.0000         0.012785 0.010441       0.0800         0.013037 0.010938       0.2257         0.013292 0.011444       0.4122         0.013550 0.011961       0.6273         0.01381 0.012487       0.8600         0.014072 0.013023       1.0991         0.014337 0.013569       1.3333         0.014605 0.014126       1.5516         0.014874 0.014693       1.7445         0.015147 0.015270       1.9054         0.015698 0.016456       2.1274	0.011798 0.008551       0.0000       0.0448         0.012041 0.009009       0.0000       0.0467         0.012287 0.009477       0.0000       0.0486         0.012534 0.009955       0.0000       0.0506         0.012785 0.010441       0.0800       0.0526         0.013037 0.010938       0.2257       0.0546         0.013292 0.011444       0.4122       0.0567         0.013550 0.011961       0.6273       0.0589         0.013810 0.012487       0.8600       0.0611         0.014072 0.013023       1.0991       0.0633         0.014337 0.013569       1.3333       0.0656         0.014874 0.014693       1.7445       0.0704         0.015147 0.015270       1.9054       0.0728         0.015698 0.016456       2.1274       0.0779

# Surface retention 2

Element Flows To: Outlet 1

Outlet 2 Bioretention 2

#### Bioretention 3

Bottom Length: 4.00 ft.
Bottom Width: 3.00 ft.
Material thickness of first layer: 1.5

Material type for first layer: SMMWW 12 in/hr

Material thickness of second layer:

Material type for second layer: GRAVEL

Material thickness of third layer: 0

Material type for third layer: GRAVEL

Infiltration On

Infiltration rate: 4
Infiltration safety factor: 0.25

Wetted surface area On

Total Volume Infiltrated (ac-ft.):

Total Volume Through Riser (ac-ft.):

1.891
Total Volume Through Facility (ac-ft.):

Percent Infiltrated:

Total Precip Applied to Facility:

1.264
Total Evap From Facility:

0.214

Underdrain used

Underdrain Diameter (feet): 0.5
Orifice Diameter (in.): 6
Offset (in.): 0

Flow Through Underdrain (ac-ft.): 54.756 Total Outflow (ac-ft.): 60.275 Percent Through Underdrain: 90.84

Discharge Structure

Riser Height: 0.5 ft. Riser Diameter: 12 in.

Element Flows To:

Outlet 1 Outlet 2

#### Bioretention Hydraulic Table

Stage(feet)	Area(ac.)	Volume(ac-ft.)	Discharge(cfs)	
0.0000	0.0079	0.0000	0.0000	0.0000
0.0385	0.0079	0.0000	0.0000	0.0000
0.0769	0.0077	0.0000	0.0000	0.0000
0.1154	0.0075	0.0000	0.0000	0.0000
0.1538	0.0073	0.0000	0.0000	0.0000
0.1923	0.0071	0.0000	0.0000	0.0000
0.2308	0.0069	0.0000	0.0000	0.0000
0.2692	0.0067	0.0001	0.0000	0.0000
0.3077	0.0065	0.0001	0.0000	0.0000
0.3462	0.0064	0.0001	0.0000	0.0000
0.3846	0.0062	0.0001	0.0001	0.0000
0.4231	0.0060	0.0001	0.0001	0.0000
0.4615	0.0058	0.0001	0.0001	0.0000
0.5000	0.0057	0.0001	0.0001	0.0000
0.5385	0.0055	0.0002	0.0002	0.0000
0.5769	0.0053	0.0002	0.0002	0.0000
0.6154	0.0052	0.0002	0.0003	0.0000
0.6538	0.0050	0.0002	0.0004	0.0000
0.6923	0.0049	0.0002	0.0004	0.0000
0.7308	0.0047	0.0003	0.0005	0.0000
0.7692	0.0046	0.0003	0.0006	0.0000

0.8077	0.0044	0.0003	0.0007	0.0000
0.8462	0.0044	0.0003	0.0007	0.0000
0.8846	0.0043	0.0003	0.0009	0.0000
0.9231	0.0041	0.0004	0.0011	0.0000
0.9231	0.0040	0.0004	0.0015	0.0000
			0.0015	
1.0000	0.0037	0.0005		0.0000
1.0385	0.0036	0.0005	0.0018	0.0000
1.0769	0.0035	0.0005	0.0021	0.0000
1.1154	0.0033	0.0006	0.0024	0.0000
1.1538	0.0032	0.0006	0.0028	0.0000
1.1923	0.0031	0.0007	0.0032	0.0000
1.2308	0.0029	0.0007	0.0037	0.0000
1.2692	0.0028	0.0008	0.0041	0.0000
1.3077	0.0027	0.0008	0.0043	0.0000
1.3462	0.0026	0.0009	0.0049	0.0000
1.3846	0.0025	0.0009	0.0055	0.0000
1.4231	0.0024	0.0010	0.0062	0.0001
1.4615	0.0023	0.0010	0.0069	0.0001
1.5000	0.0022	0.0011	0.0077	0.0001
1.5385	0.0021	0.0012	0.0086	0.0001
1.5769	0.0020	0.0012	0.0093	0.0001
1.6154	0.0019	0.0013	0.0099	0.0001
1.6538	0.0018	0.0014	0.0110	0.0001
1.6923	0.0017	0.0014	0.0121	0.0001
1.7308	0.0016	0.0015	0.0176	0.0002
1.7692	0.0015	0.0016	0.0182	0.0002
1.8077	0.0014	0.0016	0.0188	0.0002
1.8462 1.8846	0.0013 0.0013	0.0017 0.0018	0.0194 0.0200	0.0002
1.9231	0.0013	0.0018	0.0200	0.0002 0.0003
1.9615	0.0012	0.0019	0.0207	0.0003
2.0000	0.0011	0.0020	0.0213	0.0003
2.0385	0.0010	0.0020	0.0219	0.0003
2.0769	0.0009	0.0021	0.0220	0.0003
2.1154	0.0008	0.0022	0.0232	0.0003
2.1538	0.0008	0.0024	0.0246	0.0004
2.1923	0.0007	0.0025	0.0253	0.0004
2.2308	0.0007	0.0026	0.0260	0.0004
2.2692	0.0006	0.0027	0.0267	0.0004
2.3077	0.0005	0.0028	0.0274	0.0004
2.3462	0.0005	0.0029	0.0281	0.0005
2.3846	0.0004	0.0031	0.0288	0.0005
2.4231	0.0004	0.0032	0.0296	0.0005
2.4615	0.0004	0.0033	0.0303	0.0005
2.5000	0.0003	0.0034	0.0311	0.0006
2.5000	0.0003	0.0034	0.0311	0.0006
	Pioretention Hydrau			2.0000

Bioretention Hydraulic Table

# Stage(feet)Area(ac.)Volume(ac-ft.)Discharge(cfs)To Amended(cfs)Infilt(cfs)

2.5000	0.0078510.003420	0.0000	0.0243	0.0002
2.5385	0.0080480.003726	0.0000	0.0243	0.0004
2.5769	0.0082480.004039	0.0000	0.0262	0.0006
2.6154	0.0084500.004360	0.0000	0.0275	0.0008
2.6538	0.0086550.004689	0.0000	0.0289	0.0010
2.6923	0.0088620.005026	0.0000	0.0302	0.0012
2.7308	0.009071 0.005371	0.0000	0.0317	0.0014
2.7692	0.0092830.005724	0.0000	0.0331	0.0017
2.8077	0.0094980.006085	0.0000	0.0346	0.0019

0.0097140.006454	0.0000	0.0362	0.0021
0.0099340.006832	0.0000	0.0378	0.0023
0.0101550.007219	0.0000	0.0394	0.0025
0.0103790.007613	0.0000	0.0411	0.0028
0.0106060.008017	0.0000	0.0428	0.0030
0.0108350.008429	0.0800	0.0445	0.0032
0.0110670.008850	0.2257	0.0464	0.0035
0.0113000.009281	0.4122	0.0482	0.0037
0.0115370.009720	0.6273	0.0501	0.0040
0.0117760.010168	0.8600	0.0521	0.0042
0.0120170.010626	1.0991	0.0541	0.0044
0.0122610.011093	1.3333	0.0561	0.0047
0.0125070.011569	1.5516	0.0582	0.0049
0.0127550.012055	1.7445	0.0604	0.0052
0.0130060.012550	1.9054	0.0625	0.0055
0.0132600.013055	2.0318	0.0648	0.0057
0.0135160.013570	2.1274	0.0671	0.0060
0.0137740.014095	2.2033	0.0694	0.0060
	0.0099340.006832 0.0101550.007219 0.0103790.007613 0.0106060.008017 0.0108350.008429 0.0110670.008850 0.0113000.009281 0.0115370.009720 0.0117760.010168 0.0120170.010626 0.0122610.011093 0.0125070.011569 0.0127550.012055 0.0130060.012550 0.0135160.013570	0.0099340.006832       0.0000         0.0101550.007219       0.0000         0.0103790.007613       0.0000         0.0106060.008017       0.0000         0.0108350.008429       0.0800         0.0110670.008850       0.2257         0.0113000.009281       0.4122         0.0115370.009720       0.6273         0.0117760.010168       0.8600         0.0120170.010626       1.0991         0.0122610.011093       1.3333         0.0125070.011569       1.5516         0.0130060.012550       1.9054         0.0132600.013055       2.0318         0.0135160.013570       2.1274	0.0099340.006832         0.0000         0.0378           0.0101550.007219         0.0000         0.0394           0.0103790.007613         0.0000         0.0411           0.0106060.008017         0.0000         0.0428           0.0108350.008429         0.0800         0.0445           0.0110670.008850         0.2257         0.0464           0.0113000.009281         0.4122         0.0482           0.0115370.009720         0.6273         0.0501           0.0117760.010168         0.8600         0.0521           0.0120170.010626         1.0991         0.0541           0.0122610.011093         1.3333         0.0561           0.0125070.011569         1.5516         0.0582           0.0127550.012055         1.7445         0.0604           0.0130060.012550         1.9054         0.0625           0.0132600.013055         2.0318         0.0648           0.0135160.013570         2.1274         0.0671

### Surface retention 3

Element Flows To: Outlet 1

Outlet 1 Outlet 2

Bioretention 3

# Analysis Results POC 1

POC #1 was not reported because POC must exist in both scenarios and both scenarios must have been run.

# Model Default Modifications

Total of 0 changes have been made.

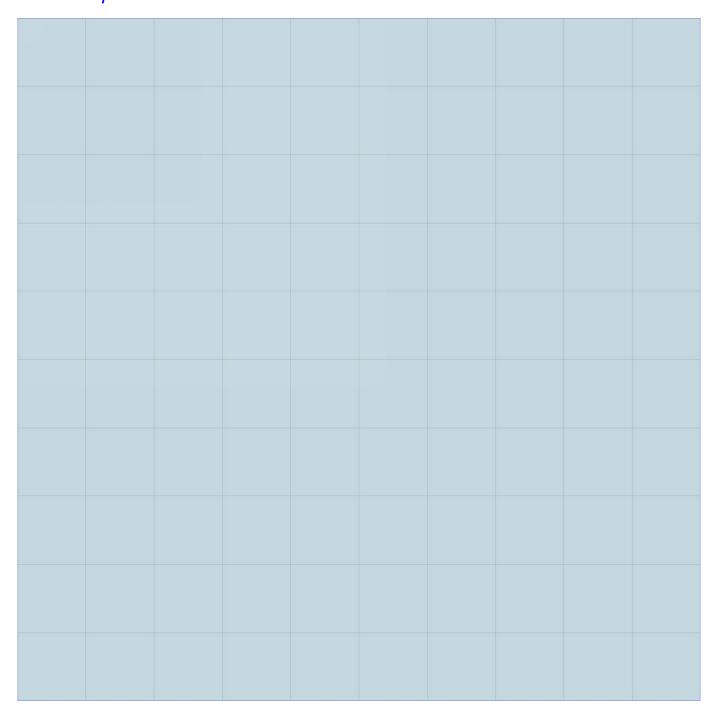
# PERLND Changes

No PERLND changes have been made.

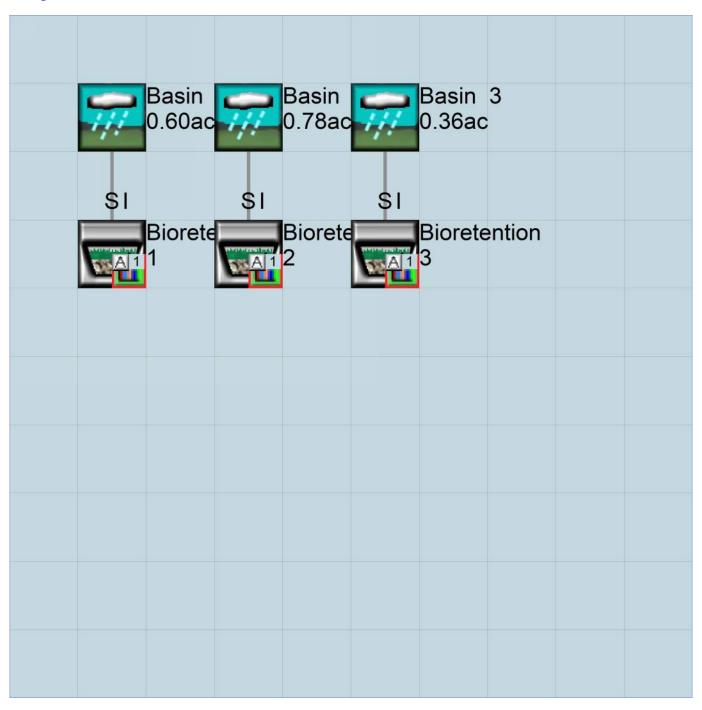
# **IMPLND Changes**

No IMPLND changes have been made.

# Appendix Predeveloped Schematic



# Mitigated Schematic



```
Predeveloped UCI File
RUN
GLOBAL
 WWHM4 model simulation
                       END 2009 09 30 3 0
 START 1948 10 01
 RUN INTERP OUTPUT LEVEL
 RESUME 0 RUN 1
                                      UNIT SYSTEM 1
END GLOBAL
FILES
            <---->***
<File> <Un#>
<-ID->
         26 FDM-01 Rock Creek WWHM.wdm
MDM
MESSII
         25
            PreFDM-01 Rock Creek WWHM.MES
         27
              PreFDM-01 Rock Creek WWHM.L61
         28
             PreFDM-01 Rock Creek WWHM.L62
END FILES
OPN SEQUENCE
                   INDELT 00:15
   INGRP
   END INGRP
END OPN SEQUENCE
DISPLY
 DISPLY-INFO1
   # - #<-----Title---->***TRAN PIVL DIG1 FIL1 PYR DIG2 FIL2 YRND
 END DISPLY-INFO1
END DISPLY
COPY
 TIMESERIES
   # - # NPT NMN ***
   1 1
 END TIMESERIES
END COPY
GENER
 OPCODE
  # # OPCD ***
 END OPCODE
 PARM
               K ***
 END PARM
END GENER
PERLND
 GEN-INFO
   <PLS ><----Name---->NBLKS Unit-systems Printer ***
                               User t-series Engl Metr ***
                                     in out
 END GEN-INFO
 *** Section PWATER***
 ACTIVITY
   <PLS > ******** Active Sections **********************
   # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
 END ACTIVITY
 PRINT-INFO
   <PLS > ********* Print-flags ***************** PIVL PYR
   # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ********
 END PRINT-INFO
 PWAT-PARM1
   <PLS > PWATER variable monthly parameter value flags ***
   # - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
 END PWAT-PARM1
```

# - # ***FOREST LZSN INFILT LSUR SLSUR KVARY AGWRC

PWATER input info: Part 2

PWAT-PARM2

END PWAT-PARM2

```
PWAT-PARM3
  END PWAT-PARM3
 PWAT-PARM4
  END PWAT-PARM4
 PWAT-STATE1
  <PLS > *** Initial conditions at start of simulation
         ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
   # - # *** CEPS SURS UZS IFWS LZS AGWS
                                                         GWVS
 END PWAT-STATE1
END PERLND
IMPLND
 GEN-INFO
  <PLS ><----- Name----> Unit-systems Printer ***
                     User t-series Engl Metr ***
  # - #
                            in out
 END GEN-INFO
 *** Section IWATER***
 ACTIVITY
  # - # ATMP SNOW IWAT SLD IWG IQAL ***
 END ACTIVITY
 PRINT-INFO
  <ILS > ****** Print-flags ****** PIVL PYR
   # - # ATMP SNOW IWAT SLD IWG IQAL *******
 END PRINT-INFO
 IWAT-PARM1
  <PLS > IWATER variable monthly parameter value flags ***
   # - # CSNO RTOP VRS VNN RTLI ***
 END IWAT-PARM1
 IWAT-PARM2
  <PLS > IWATER input info: Part 2 *
# - # *** LSUR SLSUR NSUR RETSC
 END IWAT-PARM2
 IWAT-PARM3
          IWATER input info: Part 3
  <PLS >
  # - # ***PETMAX PETMIN
 END IWAT-PARM3
 IWAT-STATE1
  <PLS > *** Initial conditions at start of simulation
   # - # *** RETS SURS
 END IWAT-STATE1
END IMPLND
SCHEMATIC
                     <--Area--> <-Target-> MBLK *** <-factor-> <Name> # Tbl# ***
<-Source->
<Name> #
*****Routing****
END SCHEMATIC
NETWORK
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
```

```
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***
END NETWORK
RCHRES
  GEN-INFO
   RCHRES Name Nexits Unit Systems Printer
                                                                              * * *
   # - #<----- Engl Metr LKFG
                                                                              * * *
                                                                               * * *
                                            in out
  END GEN-INFO
  *** Section RCHRES***
  ACTIVITY
    <PLS > ******** Active Sections **********************
    # - # HYFG ADFG CNFG HTFG SDFG GQFG OXFG NUFG PKFG PHFG ***
  END ACTIVITY
  PRINT-INFO
    <PLS > ******** Print-flags ******** PIVL PYR
    # - # HYDR ADCA CONS HEAT SED GQL OXRX NUTR PLNK PHCB PIVL PYR *******
  END PRINT-INFO
  HYDR-PARM1
    RCHRES Flags for each HYDR Section
    # - # VC A1 A2 A3 ODFVFG for each *** ODGTFG for each FUNCT for each FG FG FG possible exit *** possible exit possible exit ***
  END HYDR-PARM1
  HYDR-PARM2
                                            STCOR
  # - # FTABNO
                          LEN DELTH
                                                         KS
  <----><----><---->
                                                                              * * *
  END HYDR-PARM2
  HYDR-INIT
    RCHRES Initial conditions for each HYDR section
    <---->
                        <---><---><---> *** <---><---><--->
  END HYDR-INIT
END RCHRES
FTABLES
END FTABLES
EXT SOURCES
<-Volume-> <Member> SsysSgap<--Mult-->Tran <-Target vols> <-Grp> <-Member-> ***

      <Name>
      # <Name>
      # # 
      <Name>

      WDM
      2 PREC
      ENGL
      1.333
      PERLND
      1 999
      EXTNL
      PREC

      WDM
      2 PREC
      ENGL
      1.333
      IMPLND
      1 999
      EXTNL
      PREC

      WDM
      1 EVAP
      ENGL
      0.76
      PERLND
      1 999
      EXTNL
      PETINP

      WDM
      1 EVAP
      ENGL
      0.76
      IMPLND
      1 999
      EXTNL
      PETINP

                                                                  <Name> # # ***
END EXT SOURCES
EXT TARGETS
<-Volume-> <-Grp> <-Member-><--Mult-->Tran <-Volume-> <Member> Tsys Tgap Amd ***
<Name> # <Name> # #<-factor->strg <Name> # <Name> tem strg strg***
END EXT TARGETS
MASS-LINK
          <Name>
END MASS-LINK
END RUN
```

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#### Mitigated UCI File

RUN

```
GLOBAL
 WWHM4 model simulation
                         END 2009 09 30
 START 1948 10 01
                      END
3 0
 RUN INTERP OUTPUT LEVEL
 RESUME 0 RUN 1
                                    UNIT SYSTEM 1
END GLOBAL
FILES
<File> <Un#>
            <---->***
<-ID->
WDM
         26
            FDM-01 Rock Creek WWHM.wdm
MESSU
         25
            MitFDM-01 Rock Creek WWHM.MES
         27
             MitFDM-01 Rock Creek WWHM.L61
         28
             MitFDM-01 Rock Creek WWHM.L62
             POCFDM-01 Rock Creek WWHM1.dat
         30
END FILES
OPN SEQUENCE
   INGRP
                  INDELT 00:15
                7
    PERLND
              8
     IMPLND
     IMPLND
               11
     IMPLND
     GENER
                2
    RCHRES
    RCHRES
    GENER
    RCHRES
    RCHRES
    GENER
    RCHRES
                5
    RCHRES
               6
    COPY
                1
    COPY
              501
    DISPLY
              1
   END INGRP
END OPN SEQUENCE
DISPLY
 DISPLY-INFO1
   END DISPLY-INFO1
END DISPLY
COPY
 TIMESERIES
   # - # NPT NMN ***
          1 1
 501
           1
                1
 END TIMESERIES
END COPY
GENER
 OPCODE
       # OPCD ***
  #
   2
         24
   4
          24
   6
           24
 END OPCODE
 PARM
   #
               K ***
   2
               0.
   4
               0.
   6
               0.
 END PARM
END GENER
PERLND
 GEN-INFO
```

```
<PLS ><-----Name----->NBLKS Unit-systems Printer ***
                                User t-series Engl Metr ***
                                      in out ***
                                   1 1 1
   7 A/B, Lawn, Flat
 END GEN-INFO
 *** Section PWATER***
 ACTIVITY
   <PLS > ******** Active Sections ********************
   # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC ***
7 0 0 1 0 0 0 0 0 0 0 0 0
 END ACTIVITY
 PRINT-INFO
   # - # ATMP SNOW PWAT SED PST PWG PQAL MSTL PEST NITR PHOS TRAC *********
7 0 0 4 0 0 0 0 0 0 0 0 0 1 9
 END PRINT-INFO
 PWAT-PARM1
   <PLS > PWATER variable monthly parameter value flags ***
# - # CSNO RTOP UZFG VCS VUZ VNN VIFW VIRC VLE INFC HWT ***
7 0 0 0 0 0 0 0 0 0 0 0
 END PWAT-PARM1
             PWATER input info: Part 2 ***

LZSN INFILT LSUR SLSUR KVARY

1.72 0.05 0.3
 PWAT-PARM2
   <PLS >
                                                                  AGWRC
   # - # ***FOREST LZSN INFILT
        0
                      5
                                                                     0.996
 END PWAT-PARM2
 PWAT-PARM3
            PWATER input info: Part 3 ***
  <PLS >
   # - # ***PETMAX PETMIN INFEXP
7 0 0 2
                                        INFILD DEEPFR
                                                           BASETP
                                        2
                                                 0 0
                                                                    0
 END PWAT-PARM3
 PWAT-PARM4
             PWATER input info: Part 4
   <PLS >
   # - # CEPSC UZSN NSUR
7 0.1 0.5 0.25
                                        INTFW
0
                                                    IRC
                                                           LZETP ***
                                                        0.25
                                                    0.7
 END PWAT-PARM4
 PWAT-STATE1
   <PLS > *** Initial conditions at start of simulation
    ran from 1990 to end of 1992 (pat 1-11-95) RUN 21 ***
       # *** CEPS SURS UZS IFWS LZS AGWS 0 0 0 0 3 1
                                                                     GWVS
 END PWAT-STATE1
END PERLND
IMPLND
   <PLS ><----- Name----> Unit-systems Printer ***
                           User t-series Engl Metr ***
                                   in out
                                 0
   8
         SIDEWALKS/FLAT
  11
         PARKING/FLAT
                               1
                             1
         ROOF TOPS/FLAT
 END GEN-INFO
 *** Section IWATER***
 ACTIVITY
   <PLS > ******** Active Sections **********************
   # - # ATMP SNOW IWAT SLD IWG IQAL
       0 0 1 0 0 0
0 0 1 0 0 0
0 0 1 0 0 0
   8
  11
   4
 END ACTIVITY
```

```
PRINT-INFO
   <ILS > ****** Print-flags ****** PIVL PYR
   # - # ATMP SNOW IWAT SLD IWG IQAL *******
                                      1 9
        0 0 4 0 0 0
            0
               0 4 0 0
                                   0 1
                                           9
   4
           0
                 0 4 0
                              0
                                   0
                                            9
 END PRINT-INFO
 IWAT-PARM1
   <PLS > IWATER variable monthly parameter value flags ***
   # - # CSNO RTOP VRS VNN RTLI
                        0 0
        0 0
   8
                   0
                0 0
            0
                        0
                              0
  11
   4
                  0
                       0
 END IWAT-PARM1
 IWAT-PARM2
   <PLS > IWATER input info: Part 2 # - # *** LSUR SLSUR NSUR
  <PLS >
                                        RETSC
                                       0.1
   8
               400
                      0.01
                                0.1
                                         0.1
               400
                      0.01
                                0.1
  11
   4
               400
                      0.01
                                0.1
                                         0.1
 END IWAT-PARM2
 IWAT-PARM3
             IWATER input info: Part 3
   <PLS >
   # - # ***PETMAX PETMIN
   8
            0
                      0
  11
                 0
                          0
   4
                 0
                          0
 END IWAT-PARM3
 IWAT-STATE1
   <PLS > *** Initial conditions at start of simulation
   # - # *** RETS SURS
   8
                0
                       0
                 0
                          0
  11
   4
                 0
                          0
 END IWAT-STATE1
END IMPLND
SCHEMATIC
                         <--Area--> <-Target-> MBLK
                                                        * * *
<-Source->
<Name> #
                                      <Name> # Tbl#
                         <-factor->
Basin A***
PERLND
                                      RCHRES
                                                     2
                              0.11
                                               1
PERLND
                              0.11
                                      RCHRES
                                               1
                                                     3
      8
                                                     5
IMPLND
                               0.1
                                      RCHRES
                                               1
IMPLND 11
                              0.39
                                      RCHRES
                                               1
                                                     5
Basin B***
PERLND 7
                              0.32
                                      RCHRES
PERLND 7
                              0.32
                                      RCHRES
                                                     3
IMPLND 8
                              0.13
                                      RCHRES
                                              3
                                                     5
IMPLND 11
                              0.33
                                      RCHRES
                                               3
                                                     5
Basin 3***
      3
7
7
                              0.08
                                      RCHRES
                                               5
PERLND
                                               5
                                                     3
PERLND
                              0.08
                                      RCHRES
      4
IMPLND
                              0.2
                                      RCHRES
                                               5
                                                     5
IMPLND
      8
                              0.02
                                      RCHRES
                                               5
                                                     5
                              0.06
                                               5
IMPLND 11
                                      RCHRES
*****Routing****
      7
                                                   12
PERLND
                              0.11
                                      COPY
                                              1
                                      COPY
                                              1
IMPLND
       8
                               0.1
                                                    15
IMPLND 11
                              0.39
                                      COPY
                                               1
                                                    15
PERLND
        7
                              0.11
                                      COPY
                                               1
                                                    13
       1
                                               2
RCHRES
                                 1
                                      RCHRES
                                                    8
        3
                                 1
                                               4
                                                    8
RCHRES
                                      RCHRES
RCHRES
                                      COPY
                                               1
                                                    18
```

RCHRES 5 RCHRES 5 RCHRES 2 RCHRES 1 RCHRES 4 RCHRES 6 END SCHEMATIC	1 1 1 1	COPY 5 COPY 5	6 1 01 01 01	8 18 17 17 17		
NETWORK  <-Volume-> <-Grp> <-Member-> <mult <name=""> #</mult>	->strg  1  1		# # 1 1 3	<-Grp> INPUT EXTNL EXTNL EXTNL	<name> # # TIMSER 1 OUTDGT 1</name>	***
<-Volume-> <-Grp> <-Member-> <mult <name> # <name> # #&lt;-factor- END NETWORK</name></name></mult 					<-Member-> <name> # #</name>	* * * * * *
RCHRES  GEN-INFO  RCHRES Name Nexits  # - #<> U  1 Surface retentio-007 3 2 Bioretention 1 2 3 Surface retentio-011 3 4 Bioretention 2 2 5 Surface retentio-015 3 6 Bioretention 3 2 END GEN-INFO *** Section RCHRES***	Jser T- 1	-series : in out 1		0 : 0 : 0 : 0 : 0 :	L L L	*** *** ***
ACTIVITY <pls> ********** Active Secti  # - # HYFG ADFG CNFG HTFG SDFG G  1</pls>					****	
PRINT-INFO <pls> ************** Print-fl  # - # HYDR ADCA CONS HEAT SED  1</pls>	_	********  XRX NUTR  0		HCB PIVI 0	DYR **** L 9 L 9 L 9	****
HYDR-PARM1  RCHRES Flags for each HYDR Secti # - # VC A1 A2 A3 ODFVFG for e  FG FG FG FG possible ex  * * * * * * * * *  1	each *; kit *; 0 0 0 0 0 0 0 0 0 0	** ODGTFG  ** possib	* * 0 0 0 0 0 0 0 0		*** 2 1 2 2 2 2 2 2 2 1 2 2 2 1 2 2	xit 2 2 2 2 2 2

```
HYDR-PARM2
   # - # FTABNO LEN DELTH STCOR KS DB50
                                                                           * * *
  <----><----><---->
          1 0.01 0.0 0.0 0.0 0.0
2 0.01 0.0 0.0 0.0 0.0
3 0.01 0.0 0.0 0.0 0.0
4 0.01 0.0 0.0 0.0 0.0
5 0.01 0.0 0.0 0.0 0.0
6 0.01 0.0 0.0 0.0 0.0
   3
   4
   5
   6
 END HYDR-PARM2
 HYDR-INIT
   RCHRES Initial conditions for each missible exit

# - # *** VOL Initial value of COLIND Initial value of OUT

for each possible exit for each possible exit
   RCHRES Initial conditions for each HYDR section
                                                    Initial value of OUTDGT
                       <---><---><---> *** <---><--->
  <---->
                         1
            Ω
   2
                0
   3
                0
    4
                0
                0
   5
   6
 END HYDR-INIT
END RCHRES
SPEC-ACTIONS
*** User-Defined Variable Quantity Lines
* * *
                          addr
* * *
                          <--->
*** kwd varnam optyp opn vari s1 s2 s3 tp multiply lc ls ac as agfn ***
 UVQUAN vol2 RCHRES 2 VOL
 UVQUAN v2m2 GLOBAL WORKSP 1
UVQUAN vpo2 GLOBAL WORKSP 2
UVQUAN v2d2 GENER 2 K 1
                                          3
*** User-Defined Variable Quantity Lines
* * *
* * *
                          <--->
*** kwd varnam optyp opn vari s1 s2 s3 tp multiply lc ls ac as agfn *** <****> <---> <---> <-> <-><-><-><-> ***
 UVQUAN vol4 RCHRES 4 VOL
 UVQUAN v2m4 GLOBAL WORKSP 3
UVQUAN vpo4 GLOBAL WORKSP 4
UVQUAN v2d4 GENER 4 K 1
*** User-Defined Variable Quantity Lines
* * *
                          addr
* * *
                         <--->
*** kwd varnam optyp opn vari s1 s2 s3 tp multiply lc ls ac as agfn *** <****> <---> <---> <-> <->-> ***
 UVQUAN vol6 RCHRES 6 VOL
 UVQUAN v2m6 GLOBAL WORKSP 5
UVQUAN vpo6 GLOBAL WORKSP 6
 UVQUAN v2d6 GENER 6 K 1
*** User-Defined Target Variable Names
***
                   addr or
                                                addr or
* * *
                   <--->
                                                <--->
*** kwd varnam ct vari s1 s2 s3 frac oper 
<****> <---> <---> <---> <-->
                                                vari s1 s2 s3 frac oper
                                                 <---><-><-> <-->
 UVNAME v2m2 1 WORKSP 1
                                   1.0 QUAN
 UVNAME vpo2 1 WORKSP 2 1.0 QUAN UVNAME v2d2 1 K 1 1.0 QUAN
*** User-Defined Target Variable Names
***
                   addr or
* * *
                   <--->
                                                <--->
*** kwd varnam ct vari s1 s2 s3 frac oper
                                                vari s1 s2 s3 frac oper
         <---><-> <---> <-->
                                                <---><-><->
 1.0 QUAN
*** User-Defined Target Variable Names
```

```
* * *
                 addr or
                                            addr or
* * *
                 <--->
                                           <--->
       varnam ct vari s1 s2 s3 frac oper
                                           vari s1 s2 s3 frac oper
 <****> <---><-> <--> <-->
                                            <---><-><-><->
 UVNAME v2m6 1 WORKSP 5
                                1.0 QUAN
 UVNAME VP06 1 WORKSP 6 1.0 QUAN UVNAME V2d6 1 K 1 1.0 QUAN
*** opt foplop dcdts yr mo dy hr mn d t vnam s1 s2 s3 ac quantity tc ts rp
 = 228.87
 GENER
                                    v2m2
*** Compute remaining available pore space
 GENER 2
                                    vpo2
                                                  = v2m2
                                                 -= vol2
 GENER
                                    vpo2
*** Check to see if VPORA goes negative; if so set VPORA = 0.0
IF (vpo2 < 0.0) THEN
 GENER 2
                                                 = 0.0
                                    vpo2
END IF
*** Infiltration volume
                                    v2d2
 GENER 2
                                                  = vpo2
*** opt foplop dcdts yr mo dy hr mn d t
                                    vnam s1 s2 s3 ac quantity tc ts rp
 <****><-><-><> <> <> <> <> <> <>
                                    <----> <> <-><->
                                                  = 228.87
                                    v2m4
*** Compute remaining available pore space
 GENER
                                    vpo4
                                                  = v2m4
 GENER
                                    vpo4
                                                 -= vol4
*** Check to see if VPORA goes negative; if so set VPORA = 0.0
IF (vpo4 < 0.0) THEN
 GENER
                                    vpo4
END IF
*** Infiltration volume
                                    v2d4
 GENER 4
                                                  = vpo4
*** opt foplop dcdts yr mo dy hr mn d t
                                    vnam s1 s2 s3 ac quantity tc ts rp
 = 177.47
                                    v2m6
*** Compute remaining available pore space
                                                  = v2m6
 GENER 6
                                    vpo6
                                    vpo6
                                                 -= vol6
 GENER
*** Check to see if VPORA goes negative; if so set VPORA = 0.0
IF (vpo6 < 0.0) THEN
                                                  = 0.0
 GENER
                                    vpo6
END IF
*** Infiltration volume
                                    v2d6
 GENER 6
                                                  = vpo6
END SPEC-ACTIONS
FTABLES
 FTABLE
     5
  67
                    Volume Outflow1 Outflow2 Velocity Travel Time***
    Depth
             Area
                                                      (Minutes)***
     (ft)
          (acres) (acre-ft)
                           (cfs)
                                     (cfs)
                                             (ft/sec)
 0.000000 0.009504 0.000000 0.000000 0.000000
 0.038462 0.009504 0.000010 0.000000 0.000000
          0.009288 0.000021 0.000000 0.000000
 0.076923
 0.115385
         0.009075 0.000034 0.000000 0.000000
         0.008864 0.000047 0.000000 0.000000
 0.153846
 0.192308
          0.008655
                  0.000062
                           0.000000 0.000000
 0.230769
          0.008449
                  0.000078
                           0.000017
                                    0.000000
          0.008245
                  0.000095
                           0.000028
 0.269231
                                    0.000000
                           0.000043 0.000000
 0.307692
          0.008044
                  0.000114
                           0.000063 0.000000
         0.007845 0.000134
 0.346154
 0.384615
          0.007648 0.000155
                           0.000090 0.000001
 0.423077
         0.007454 0.000178 0.000125 0.000001
 0.500000 0.007074 0.000228 0.000221 0.000002
                           0.000286 0.000003
          0.006887
                  0.000256
 0.538462
                  0.000285
                           0.000364 0.000003
 0.576923
          0.006703
 0.615385
          0.006521
                   0.000316
                            0.000457
                                    0.000005
                            0.000567
 0.653846
          0.006342
                   0.000348
                                    0.000006
                           0.000635 0.000006
 0.692308
          0.006165
                  0.000382
 0.730769
          0.005991
                   0.000418
                           0.000733 0.000006
 0.769231
         0.005819
                  0.000456 0.000890 0.000007
```

```
0.807692
             0.005649
                        0.000496
                                   0.001069
                                              0.000009
                                              0.000012
             0.005482
  0.846154
                        0.000538
                                   0.001275
  0.884615
             0.005317
                        0.000581
                                   0.001509
                                              0.000015
  0.923077
             0.005155
                        0.000627
                                   0.001773
                                              0.000018
                        0.000675
                                   0.002071
  0.961538
             0.004995
                                              0.000022
  1.000000
             0.004838
                        0.000724
                                   0.002200
                                              0.000022
                                   0.002513
  1.038462
             0.004683
                        0.000776
                                              0.000023
                                   0.002900
  1.076923
             0.004531
                        0.000831
                                              0.000028
  1.115385
             0.004381
                        0.000887
                                   0.003332
                                              0.000033
  1.153846
             0.004233
                        0.000946
                                   0.003810
                                              0.000040
  1.192308
             0.004088
                        0.001007
                                   0.004338
                                              0.000047
  1.230769
             0.003946
                        0.001070
                                   0.004919
                                              0.000055
  1.269231
             0.003805
                        0.001136
                                   0.005507
                                              0.000055
  1.307692
             0.003668
                        0.001204
                                   0.005769
                                              0.000060
  1.346154
             0.003532
                        0.001274
                                   0.006491
                                              0.000070
                                   0.007279
  1.384615
             0.003400
                        0.001348
                                              0.000081
             0.003269
  1.423077
                        0.001423
                                   0.008137
                                              0.000094
                                   0.009069
  1.461538
             0.003141
                        0.001502
                                              0.000108
  1.500000
             0.003016
                        0.001575
                                   0.010079
                                              0.000124
                                              0.000142
  1.538462
             0.002893
                        0.001651
                                   0.011171
  1.576923
             0.002772
                        0.001730
                                   0.012028
                                              0.000142
  1.615385
             0.002654
                        0.001811
                                   0.012757
                                              0.000155
  1.653846
             0.002538
                        0.001894
                                   0.014059
                                              0.000176
             0.002425
                        0.001980
                                   0.015451
  1.692308
                                              0.000199
  1.730769
             0.002314
                        0.002069
                                   0.022486
                                              0.000298
  1.769231
             0.002205
                        0.002161
                                   0.023152
                                              0.000316
  1.807692
             0.002100
                        0.002255
                                   0.023826
                                              0.000335
  1.846154
             0.001996
                        0.002352
                                   0.024510
                                              0.000355
  1.884615
             0.001895
                        0.002452
                                   0.025203
                                              0.000376
  1.923077
             0.001796
                        0.002555
                                   0.025905
                                              0.000397
             0.001700
  1.961538
                        0.002660
                                   0.026616
                                              0.000419
  2.000000
             0.001606
                        0.002769
                                   0.027335
                                              0.000442
  2.038462
             0.001515
                        0.002880
                                   0.028064
                                              0.000466
  2.076923
             0.001426
                        0.002994
                                   0.028802
                                              0.000491
                        0.003112
  2.115385
             0.001340
                                   0.029548
                                              0.000518
             0.001256
                        0.003232
                                   0.030304
                                              0.000545
  2.153846
  2.192308
             0.001174
                        0.003356
                                   0.031068
                                              0.000573
  2.230769
             0.001095
                        0.003483
                                   0.031841
                                              0.000602
  2.269231
             0.001019
                        0.003613
                                   0.032622
                                              0.000632
  2.307692
             0.000945
                        0.003746
                                   0.033413
                                              0.000663
  2.346154
             0.000873
                        0.003883
                                   0.034212
                                              0.000696
  2.384615
             0.000804
                        0.004022
                                   0.035020
                                              0.000729
  2.423077
             0.000737
                        0.004166
                                   0.035837
                                              0.000764
  2.461538
             0.000672
                        0.004312
                                   0.036662
                                              0.000800
  2.500000
             0.000610
                                   0.037496
                        0.004462
                                              0.000837
  2.500000
             0.000551
                        0.005254
                                   0.037496
                                              0.000837
  END FTABLE
               2
  FTABLE
               1
   27
     Depth
                          Volume
                                  Outflow1
                                              Outflow2
                                                        Outflow3
                                                                   Velocity
                                                                              Travel
                 Area
Time***
                                    (cfs)
                                                (cfs)
                                                           (cfs)
      (ft)
              (acres) (acre-ft)
                                                                    (ft/sec)
(Minutes) * * *
  0.000000
            0.000551
                        0.000000
                                   0.00000
                                              0.000000
                                                         0.000220
                                                         0.000220
  0.038462
             0.009723
                        0.000370
                                   0.00000
                                              0.029411
  0.076923
             0.009943
                        0.000748
                                   0.00000
                                              0.031621
                                                         0.000443
             0.010167
                                              0.033120
  0.115385
                        0.001135
                                   0.000000
                                                         0.000668
  0.153846
             0.010393
                        0.001530
                                   0.000000
                                              0.034662
                                                         0.000896
  0.192308
                        0.001934
             0.010621
                                   0.000000
                                              0.036247
                                                         0.001126
  0.230769
             0.010851
                        0.002347
                                   0.00000
                                              0.037876
                                                         0.001358
                        0.002769
  0.269231
             0.011084
                                   0.000000
                                              0.039549
                                                         0.001594
  0.307692
             0.011320
                        0.003200
                                   0.00000
                                              0.041267
                                                         0.001831
  0.346154
             0.011558
                        0.003640
                                   0.000000
                                              0.043031
                                                         0.002071
                                   0.00000
                                              0.044842
  0.384615
             0.011798
                        0.004089
                                                         0.002313
                                   0.000000
  0.423077
             0.012041
                        0.004547
                                              0.046699
                                                         0.002558
  0.461538
             0.012287
                        0.005015
                                   0.00000
                                              0.048603
                                                         0.002806
                                   0.00000
  0.500000
             0.012534
                        0.005493
                                              0.050556
                                                         0.003056
                        0.005979
  0.538462
             0.012785
                                   0.079976
                                              0.052556
                                                         0.003308
             0.013037
                        0.006476
                                   0.225672
                                              0.054606
  0.576923
                                                         0.003563
  0.615385
             0.013292
                        0.006982
                                   0.412175
                                              0.056706
                                                         0.003820
```

```
0.653846
           0.013550
                      0.007499
                                 0.627270
                                           0.058856
                                                       0.004080
0.692308
           0.013810
                      0.008025
                                 0.859995
                                           0.061056
                                                      0.004342
0.730769
           0.014072
                      0.008561
                                 1.099144
                                           0.063308
                                                       0.004606
0.769231
           0.014337
                      0.009107
                                 1.333311
                                           0.065612
                                                       0.004873
0.807692
                                 1.551565
                                           0.067968
           0.014605
                      0.009664
                                                      0.005143
0.846154
           0.014874
                      0.010231
                                 1.744468
                                           0.070377
                                                       0.005415
                                           0.072840
0.884615
           0.015147
                      0.010808
                                 1.905359
                                                       0.005690
                                                      0.005966
0.923077
           0.015421
                      0.011396
                                 2.031838
                                           0.075357
0.961538
           0.015698
                      0.011994
                                 2.127417
                                           0.077929
                                                       0.006246
1.000000
           0.015978
                      0.012604
                                 2.203335
                                           0.080556
                                                       0.006528
END FTABLE
             1
FTABLE
 67
                                                                 Travel Time***
   Depth
               Area
                        Volume
                                 Outflow1
                                           Outflow2
                                                      Velocity
                     (acre-ft)
                                  (cfs)
                                              (cfs)
                                                                    (Minutes) * * *
    (ft)
            (acres)
                                                       (ft/sec)
0.00000
                      0.00000
                                 0.00000
           0.009504
                                           0.000000
                                           0.000000
0.038462
           0.009504
                      0.000010
                                 0.000000
                                 0.00000
0.076923
           0.009288
                      0.000021
                                           0.00000
           0.009075
0.115385
                      0.000034
                                 0.000000
                                           0.000000
0.153846
           0.008864
                      0.000047
                                 0.00000
                                           0.000000
0.192308
           0.008655
                      0.000062
                                 0.000000
                                           0.000000
                                 0.000017
0.230769
           0.008449
                      0.000078
                                           0.000000
0.269231
           0.008245
                      0.000095
                                 0.000028
                                           0.000000
0.307692
           0.008044
                      0.000114
                                 0.000043
                                           0.000000
0.346154
           0.007845
                      0.000134
                                 0.000063
                                           0.000000
                      0.000155
0.384615
           0.007648
                                 0.000090
                                           0.00001
0.423077
           0.007454
                      0.000178
                                 0.000125
                                           0.00001
0.461538
           0.007263
                      0.000203
                                 0.000168
                                           0.000001
0.500000
           0.007074
                      0.000228
                                 0.000221
                                           0.000002
0.538462
           0.006887
                      0.000256
                                 0.000286
                                           0.000003
0.576923
           0.006703
                      0.000285
                                 0.000364
                                           0.00003
           0.006521
                      0.000316
                                 0.000457
0.615385
                                           0.000005
0.653846
           0.006342
                      0.000348
                                 0.000567
                                           0.00006
0.692308
           0.006165
                      0.000382
                                 0.000635
                                           0.000006
0.730769
           0.005991
                      0.000418
                                 0.000733
                                           0.000006
0.769231
           0.005819
                      0.000456
                                 0.000890
                                           0.000007
0.807692
           0.005649
                      0.000496
                                 0.001069
                                           0.000009
0.846154
           0.005482
                      0.000538
                                 0.001275
                                           0.000012
0.884615
           0.005317
                      0.000581
                                 0.001509
                                           0.000015
0.923077
           0.005155
                      0.000627
                                 0.001773
                                           0.000018
0.961538
           0.004995
                      0.000675
                                 0.002071
                                           0.000022
1.000000
           0.004838
                      0.000724
                                 0.002200
                                           0.000022
           0.004683
                      0.000776
                                 0.002513
                                           0.000023
1.038462
1.076923
           0.004531
                      0.000831
                                 0.002900
                                           0.000028
1.115385
           0.004381
                      0.000887
                                 0.003332
                                           0.000033
1.153846
           0.004233
                      0.000946
                                 0.003810
                                           0.000040
1.192308
           0.004088
                      0.001007
                                 0.004338
                                           0.000047
1.230769
           0.003946
                      0.001070
                                 0.004919
                                           0.000055
1.269231
           0.003805
                      0.001136
                                 0.005507
                                           0.000055
1.307692
           0.003668
                      0.001204
                                 0.005769
                                           0.000060
1.346154
           0.003532
                      0.001274
                                 0.006491
                                           0.000070
           0.003400
                      0.001348
                                 0.007279
1.384615
                                           0.000081
1.423077
           0.003269
                      0.001423
                                 0.008137
                                           0.000094
                                 0.009069
1.461538
           0.003141
                      0.001502
                                           0.000108
1.500000
           0.003016
                      0.001575
                                 0.010079
                                           0.000124
1.538462
           0.002893
                      0.001651
                                 0.011171
                                           0.000142
           0.002772
                      0.001730
1.576923
                                 0.012028
                                           0.000142
1.615385
           0.002654
                      0.001811
                                 0.012757
                                           0.000155
           0.002538
                                 0.014059
1.653846
                      0.001894
                                           0.000176
1.692308
           0.002425
                      0.001980
                                 0.015451
                                           0.000199
           0.002314
1.730769
                      0.002069
                                 0.022486
                                           0.000298
1.769231
           0.002205
                      0.002161
                                 0.023152
                                           0.000316
1.807692
           0.002100
                      0.002255
                                 0.023826
                                           0.000335
                      0.002352
           0.001996
1.846154
                                 0.024510
                                           0.000355
           0.001895
                      0.002452
                                 0.025203
1.884615
                                           0.000376
1.923077
           0.001796
                      0.002555
                                 0.025905
                                           0.000397
1.961538
           0.001700
                      0.002660
                                 0.026616
                                           0.000419
2.000000
           0.001606
                      0.002769
                                 0.027335
                                           0.000442
                      0.002880
                                 0.028064
2.038462
           0.001515
                                           0.000466
2.076923
           0.001426
                      0.002994
                                 0.028802
                                           0.000491
```

```
2.115385
             0.001340
                       0.003112
                                   0.029548
                                             0.000518
                       0.003232
                                             0.000545
             0.001256
                                  0.030304
  2.153846
  2.192308
             0.001174
                       0.003356
                                  0.031068
                                             0.000573
  2.230769
             0.001095
                       0.003483
                                   0.031841
                                             0.000602
  2.269231
             0.001019
                       0.003613
                                   0.032622
                                             0.000632
  2.307692
             0.000945
                       0.003746
                                   0.033413
                                             0.000663
             0.000873
  2.346154
                       0.003883
                                   0.034212
                                             0.000696
  2.384615
             0.000804
                       0.004022
                                   0.035020
                                             0.000729
  2.423077
             0.000737
                        0.004166
                                   0.035837
                                             0.000764
  2.461538
             0.000672
                        0.004312
                                   0.036662
                                             0.000800
  2.500000
             0.000610
                       0.004462
                                   0.037496
                                             0.000837
  2.500000
             0.000551
                       0.005254
                                   0.037496
                                             0.000837
               4
  END FTABLE
  FTABLE
     Depth
                 Area
                          Volume
                                  Outflow1
                                             Outflow2
                                                        Outflow3
                                                                   Velocity
                                                                              Travel
Time***
              (acres) (acre-ft)
                                                           (cfs)
      (ft)
                                    (cfs)
                                                (cfs)
                                                                   (ft/sec)
(Minutes) * * *
  0.000000
            0.000551
                        0.000000
                                   0.00000
                                             0.000000
                                                        0.000220
             0.009723
                                             0.029411
  0.038462
                       0.000370
                                  0.000000
                                                        0.000220
                       0.000748
  0.076923
             0.009943
                                   0.00000
                                             0.031621
                                                        0.000443
  0.115385
             0.010167
                       0.001135
                                   0.00000
                                             0.033120
                                                        0.000668
  0.153846
             0.010393
                       0.001530
                                   0.00000
                                             0.034662
                                                        0.000896
  0.192308
             0.010621
                       0.001934
                                   0.000000
                                             0.036247
                                                        0.001126
                                   0.00000
  0.230769
             0.010851
                       0.002347
                                             0.037876
                                                        0.001358
                                   0.00000
  0.269231
             0.011084
                       0.002769
                                             0.039549
                                                        0.001594
  0.307692
                                   0.00000
             0.011320
                       0.003200
                                             0.041267
                                                        0.001831
  0.346154
             0.011558
                       0.003640
                                   0.00000
                                             0.043031
                                                        0.002071
             0.011798
  0.384615
                       0.004089
                                   0.000000
                                             0.044842
                                                        0.002313
  0.423077
             0.012041
                       0.004547
                                   0.00000
                                             0.046699
                                                        0.002558
  0.461538
                       0.005015
             0.012287
                                   0.00000
                                             0.048603
                                                        0.002806
  0.500000
             0.012534
                       0.005493
                                   0.00000
                                             0.050556
                                                        0.003056
  0.538462
             0.012785
                       0.005979
                                   0.079976
                                             0.052556
                                                        0.003308
             0.013037
                       0.006476
                                             0.054606
  0.576923
                                   0.225672
                                                        0.003563
             0.013292
                       0.006982
                                   0.412175
                                             0.056706
                                                        0.003820
  0.615385
  0.653846
             0.013550
                       0.007499
                                   0.627270
                                             0.058856
                                                        0.004080
             0.013810
                       0.008025
  0.692308
                                   0.859995
                                             0.061056
                                                        0.004342
  0.730769
             0.014072
                       0.008561
                                   1.099144
                                             0.063308
                                                        0.004606
  0.769231
                                                        0.004873
             0.014337
                       0.009107
                                  1.333311
                                             0.065612
  0.807692
                       0.009664
             0.014605
                                  1.551565
                                             0.067968
                                                        0.005143
  0.846154
             0.014874
                       0.010231
                                  1.744468
                                             0.070377
                                                        0.005415
                                   1.905359
  0.884615
             0.015147
                        0.010808
                                             0.072840
                                                        0.005690
  0.923077
             0.015421
                       0.011396
                                   2.031838
                                             0.075357
                                                        0.005966
                       0.011994
                                             0.077929
  0.961538
             0.015698
                                   2.127417
                                                        0.006246
  1.000000
             0.015978
                       0.012604
                                   2.203335
                                             0.080556
                                                        0.006528
  END FTABLE
               3
  FTABLE
               6
   67
                                                                   Travel Time***
     Depth
                          Volume
                                  Outflow1
                                             Outflow2
                                                        Velocity
                 Area
                       (acre-ft)
                                                (cfs)
                                                                     (Minutes) * * *
      (ft)
              (acres)
                                    (cfs)
                                                        (ft/sec)
  0.00000
             0.007851
                       0.00000
                                   0.00000
                                             0.00000
  0.038462
             0.007851
                       0.000005
                                   0.00000
                                             0.000000
  0.076923
             0.007656
                       0.000011
                                   0.00000
                                             0.00000
  0.115385
             0.007464
                       0.000018
                                   0.00000
                                             0.00000
  0.153846
             0.007274
                       0.000025
                                   0.00000
                                             0.000000
  0.192308
             0.007087
                       0.000033
                                   0.00000
                                             0.00000
  0.230769
             0.006902
                       0.000042
                                   0.000010
                                             0.000000
  0.269231
             0.006719
                       0.000052
                                   0.000016
                                             0.000000
  0.307692
             0.006539
                       0.000063
                                   0.000026
                                             0.00000
  0.346154
             0.006361
                       0.000075
                                   0.000038
                                             0.000000
  0.384615
             0.006186
                       0.000088
                                   0.000055
                                             0.000000
  0.423077
             0.006013
                       0.000102
                                   0.000078
                                             0.000000
                                   0.000106
  0.461538
             0.005843
                       0.000118
                                             0.00001
                                   0.000141
  0.500000
             0.005675
                       0.000134
                                             0.00001
  0.538462
             0.005510
                       0.000151
                                   0.000185
                                             0.00001
  0.576923
             0.005347
                        0.000170
                                   0.000238
                                             0.000001
  0.615385
             0.005186
                       0.000190
                                   0.000302
                                             0.000002
  0.653846
             0.005028
                       0.000212
                                   0.000378
                                             0.00003
  0.692308
             0.004872
                        0.000235
                                   0.000427
                                             0.00003
```

```
0.730769
             0.004719
                        0.000259
                                   0.000498
                                             0.000003
  0.769231
                                             0.00003
             0.004568
                        0.000285
                                   0.000609
  0.807692
             0.004420
                        0.000312
                                   0.000738
                                             0.00004
  0.846154
             0.004274
                        0.000341
                                   0.000887
                                             0.00006
                        0.000371
                                   0.001058
  0.884615
             0.004131
                                             0.000007
  0.923077
             0.003990
                        0.000403
                                   0.001253
                                             0.000009
                                   0.001474
  0.961538
             0.003851
                        0.000437
                                             0.000011
             0.003715
  1.000000
                        0.000473
                                   0.001576
                                             0.000011
  1.038462
             0.003581
                        0.000510
                                   0.001811
                                             0.000012
  1.076923
             0.003450
                        0.000549
                                   0.002104
                                             0.000015
  1.115385
             0.003321
                        0.000590
                                   0.002431
                                             0.000018
  1.153846
             0.003195
                        0.000633
                                   0.002796
                                             0.000021
  1.192308
             0.003071
                        0.000677
                                   0.003202
                                             0.000025
  1.230769
             0.002950
                        0.000724
                                   0.003651
                                             0.000030
  1.269231
             0.002831
                        0.000773
                                   0.004107
                                             0.000031
  1.307692
             0.002714
                        0.000824
                                   0.004324
                                             0.000034
                                   0.004889
  1.346154
             0.002600
                        0.000877
                                             0.000040
  1.384615
                                   0.005508
             0.002488
                        0.000932
                                             0.000046
  1.423077
             0.002379
                        0.000989
                                   0.006186
                                             0.000054
  1.461538
             0.002272
                        0.001048
                                   0.006925
                                             0.000063
  1.500000
             0.002168
                        0.001105
                                   0.007730
                                             0.000073
  1.538462
             0.002066
                        0.001163
                                   0.008603
                                             0.000084
  1.576923
             0.001967
                        0.001223
                                   0.009298
                                             0.000084
             0.001870
                        0.001286
                                   0.009900
  1.615385
                                             0.000093
  1.653846
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                        0.001351
                                   0.010952
                                             0.000106
  1.692308
             0.001683
                        0.001418
                                   0.012080
                                             0.000121
  1.730769
             0.001593
                        0.001487
                                   0.017644
                                             0.000183
  1.769231
             0.001506
                        0.001559
                                   0.018230
                                             0.000196
  1.807692
             0.001421
                        0.001633
                                   0.018825
                                             0.000209
  1.846154
             0.001339
                        0.001709
                                   0.019429
                                             0.000222
             0.001259
                        0.001788
                                   0.020043
  1.884615
                                             0.000237
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## Predeveloped HSPF Message File

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FDM-01 Rock Creek WWHM 5/18/2023 4:42:37 PM Page 3 150

# Mitigated HSPF Message File

# Disclaimer

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www.clearcreeksolutions.com

## <u>APPENDIX 4</u> – Geotechnical Report



## GEOTECHNICAL SITE INVESTIGATION REPORT

PROPOSED ROCK CREEK COVE DEVELOPMENT PARCEL # 02070100130200, 02070100130300 & 02070100130400 ROCK CREEK DRIVE, STEVENSON, WASHINGTON

**GNN PROJECT NO. 219-1183** 

**JANUARY 2020** 

Prepared for

FDM DEVELOPMENT INC. 5101 NE 82ND AVENUE, SUITE 200 **VANCOUVER, WA 98662** 



Prepared by

GN NORTHERN, INC. CONSULTING GEOTECHNICAL ENGINEERS YAKIMA, WASHINGTON (509) 248-9798 / (541) 387-3387

> Common Sense Approach to Earth and Engineering Since 1995



At GN Northern our mission is to serve our clients in the most efficient, cost effective way using the best resources and tools available while maintaining professionalism on every level. Our philosophy is to satisfy our clients through hard work, dedication and extraordinary efforts from all of our valued employees working as an extension of the design and construction team.



January 13, 2020

FDM Development Inc. 5101 NE 82nd Ave, Suite 200 Vancouver, WA 98662

Attn: Zachary Pyle, PE, Development Manager

CC: F. Dean Maldonado, Principal

**Subject:** Geotechnical Site Investigation Report

**Proposed Rock Creek Cove Development** 

Parcel # 02070100130200, 02070100130300 & 02070100130400

Rock Creek Drive, Stevenson, Washington

GNN Project No. 219-1183

#### Gentlemen,

As requested, GN Northern (GNN) has completed a geotechnical site investigation for the proposed Rock Creek Cove vacation homes project to be constructed at the vacant site located on Rock Creek Drive, east of the intersection with Attwell Road, in the City of Stevenson, Washington.

Based on the findings of our subsurface study, we conclude that the site is suitable for the proposed construction provided that our geotechnical recommendations presented in this report are followed during the design and construction phases of the project.

This report describes in detail the results of our investigation, summarizes our findings and presents our recommendations concerning earthwork and the design and construction of foundation for the proposed project. It is important that GN Northern provide consultation during the design phase as well as field compaction testing and geotechnical monitoring services during the earthwork phase to ensure implementation of the geotechnical recommendations.

If you have any questions regarding this report, please contact us at 509-248-9798 or 541-387-3387.

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Respectfully submitted,

GN Northern, Inc.

Karl A. Harmon, LEG, PE Senior Geologist/Engineer

Engineer ___

Engineering Geologist 2535

Karl A. Harmon

M. Yousuf Memon, PE Geotechnical Engineer





Page No.

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1.0 PURPOSE AND SCOPE OF SERVICES ...... 1 2.0 PROPOSED CONSTRUCTION ......2 3.0 FIELD EXPLORATION & LABORATORY TESTING ......2 5.0 SITE & REGIONAL GEOLOGY ......4 6.0 SUBSURFACE CONDITIONS......5 6.1 NRCS SOIL SURVEY.......6 6.2 Groundwater ......6 8.0 GEOLOGIC HAZARDS.......7 8.2 REGIONAL FAULTING & SURFACE FAULT RUPTURE.......8 8.4 SOIL LIQUEFACTION.......9 8.6 SITE SLOPES _______10 10.0 SEISMIC DESIGN PARAMETERS.......12 11.0 SUMMARY OF FINDINGS & CONCLUSIONS.......13 12.0 GEOTECHNICAL RECOMMENDATIONS ...... 15 14.0 CONTINUING GEOTECHNICAL SERVICES ......27 15.0 LIMITATIONS OF THE GEOTECHNICAL SITE INVESTIGATION REPORT ......28 **APPENDICES** APPENDIX I – VICINITY MAP (FIGURE 1), SITE EXPLORATION MAP (FIGURE 2), CRITICAL AREAS MAP (FIGURE 3) APPENDIX II – EXPLORATORY TEST-PIT LOGS, KEY CHART (FOR SOIL CLASSIFICATION) APPENDIX III – LABORATORY TESTING RESULTS APPENDIX IV – SITE & EXPLORATION PHOTOGRAPHS APPENDIX V – HISTORIC AERIAL PHOTOGRAPHS APPENDIX VI – SLOPE STABILITY ANALYSIS APPENDIX VII – NRCS SOIL SURVEY



#### 1.0 PURPOSE AND SCOPE OF SERVICES

This report has been prepared for the proposed Rock Creek Cove vacation homes project to be constructed at the vacant site located on Rock Creek Drive, east of the intersection with Attwell Road, in the City of Stevenson, Washington; site location is shown on the *Vicinity Map* (Figure 1, Appendix I). Our investigation was conducted to collect information regarding subsurface conditions and present recommendations for suitability of the subsurface materials to support the proposed building structures and allowable bearing capacity for the proposed construction.

GN Northern, Inc. has prepared this report for use by the client and their design consultants in the design of the proposed development. Do not use or rely upon this report for other locations or purposes without the written consent of GN Northern, Inc.

Our study was conducted in general accordance with our *Proposal for Geotechnical Engineering Services* dated October 29, 2019. Notice to proceed was provided in the form of a signed/authorized copy of our proposal via email on November 19, 2019.

A conceptual site plan (*Concept D*, prepared by FDM Development, dated 10/28/2019), along with a topographic survey of the project site (Lots 2, 3, and 4 of Rock Creek Cove, prepared by S&F Land Services, dated 12/11/2019), were provided by Mr. Pyle via email on December 17, 2019. Field exploration, consisting of twelve (12) test-pits and one (1) infiltration test, was completed on December 23, 2019. Locations of the exploratory test-pits and infiltration test are shown on the *Site Exploration Map* (Figure 2, Appendix I), and detailed test-pit logs are presented in Appendix II.

This report has been prepared to summarize the data obtained during this study and to present our recommendations based on the proposed construction and the subsurface conditions encountered at the site. Results of the field exploration were analyzed to develop recommendations for site development, earthwork, pavements, and foundation bearing capacity. Design parameters and a discussion of the geotechnical engineering considerations related to construction are included in this report.

1



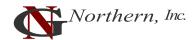
#### 2.0 PROPOSED CONSTRUCTION

Based on the preliminary information presented on the conceptual site plan and communication with your office, we understand that the proposed development will likely include approximately 15 to 25 structures. The various vacation rental structures are anticipated to consist of 6 to 8 single-room studio units along with 8 to 16 multi-story 3-bedroom units. Based on the current site layout, the studio units are planned across the southern finger, while the multi-story units are planned across the northern and western portions of the site. Proposed development will also include a 3-story central building with upstairs suite, central floor reception area, and lower floor kitchen and bar. Site development will also include associated infrastructure elements consisting of underground utilities, stormwater facilities, parking areas, and drive lanes. While the current site plan calls for a proposed wedding chapel/shelter on the eastern finger, we understand that development across this portion of the site may not be permitted.

Structural loading information was not available at the time of this report. Based on our experience with similar projects, we expect maximum wall loads to be on the order of 2,500 plf and maximum column loads to be less than 80 kips. It shall be noted that assumed loading is based on limited preliminary information provided at the time of this report. If loading conditions differ from those described herein, GNN should be given an opportunity to perform re-analysis. Settlement tolerances for structures are assumed to be limited to 1 inch, with differential settlement limited to  $\frac{1}{2}$  inch.

#### 3.0 FIELD EXPLORATION & LABORATORY TESTING

The field exploration was completed on December 23, 2019. A local public utility clearance was obtained prior to the field exploration. Twelve (12) exploratory test-pits were completed at various locations within the footprint of the proposed development. Test-pits were excavated by Riley Materials using a Link-Belt 145x4 excavator to depths of approximately 8 to 14.5 feet below existing ground surface (BGS) and logged by a GNN field geologist/engineer. Additionally, an infiltration test was performed on the north side of the entrance driveway. Upon completion, all excavations were loosely backfilled with excavation spoils. Test-hole locations are shown on *Site Exploration Map* (Figure 2)



The soils observed during our field exploration were classified according to the Unified Soil Classification System (USCS), utilizing the field classification procedures as outlined in ASTM D2488. A copy of the USCS Classification Chart is included in Appendix II. Photographs of the site and exploration are presented in Appendix IV. Depths referred to in this report are relative to the existing ground surface elevation at the time of our investigation. The surface and subsurface conditions described in this report are as observed at the time of our field investigation.

Representative samples of the subsurface soils obtained from the field exploration were selected for testing to determine the index properties of the soils in general accordance with ASTM procedures. The following laboratory tests were performed:

**Table 1: Laboratory Tests Performed** 

Test	To determine
Particle Size Distribution (ASTM D6913)	Soil classification based on proportion of sand, silt, and clay-sized particles
Natural Moisture Content (ASTM D2216)	Soil moisture content indicative of in-situ condition at the time samples were taken

Results of the laboratory test are included on the test-pit logs and are also presented in graphic form in Appendix III attached to the end of the report.

## 4.0 SITE CONDITIONS

The project site is located east of the intersection of Rock Creek Drive and Attwell Road, approximately ½-mile north of State Highway 14, in the City of Stevenson, Washington. The 6.4-acre project site is currently comprised of three separate parcels identified by the Skamania County Assessor as Parcel Numbers: 020701001302000 (Lot 2), 020701001303000 (Lot 3), and 020701001304000 (Lot 4) located within the SW ¼ of the NW ¼ of Section 1, Township 2 North and Range 7 East, Willamette Meridian.

The subject site is generally characterized as an irregular shaped peninsula with several fingers extending east from Rock Creek Drive into Rock Cove. The majority of the upper surface of the site is relatively flat, while the irregular shaped peninsula fingers typically include steep slopes along the perimeter down to the shoreline. Surface conditions across the site include a variety of gravel covered and paved areas (asphalt and concrete), as well as areas with a dense growth of mature trees and vegetation, with selected areas across slope faces that include a veneer of angular



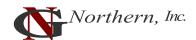
rock (apparent rip-rap). Recently placed stockpiles of apparent landscape clippings are present across an area located south of the existing entrance driveway.

Surface topography across the subject site has been historically altered by previous grading activity related to the preexisting use. The upper historically graded portions of the site are relatively flat at elevations ranging from approximately 95' to 101' across a majority of the site. Site grades step down towards that eastern finger with surface elevations ranging from approximately 87' to 90'. The surrounding edges of the various peninsula fingers typically include relatively steep slopes, with gradients as steep as 1H:1V, from the upper flat portions descending down to the shoreline.

The history of past use and development of the property was not investigated as part of our scope of services for this geotechnical site investigation. Based on our cursory review of available historic aerial photos (Appendix V) and topographic maps, along with a previously completed phase II environmental site assessment (Maul Foster Alongi, 2017), the site is known to have been historically developed with an industrial lumber mill facility. Scattered buried remnants related to the noted previous development and operations at the site including concrete foundation and slabs, miscellaneous utilities, trash and debris should be anticipated. Additionally, the eastern finger extending into Rock Cove appears to have been created by historic filling of the area between the main portion of the site and a preexisting island toward the eastern tip. The 1935 aerial photograph taken prior to historic site development of the site shows the site vicinity at the time when the Rock Cove had not been flooded by construction of the Bonneville Dam.

#### 5.0 SITE & REGIONAL GEOLOGY

The City of Stevenson and Skamania County are located in the South Cascades physiographic province that extends from the Columbia River to the south to Interstate 90 to the north, and is dominated by three massive stratovolcanoes. The current day volcanoes are the most recent installments of a 40-million-year-old volcanic complex called the Cascades Volcanic Arc. The bedrock geology of the western Columbia Gorge is dominated by Oligocene to early Miocene volcaniclastic rocks and minor interbedded lava flows of the ancestral Cascade Volcanic Arc. At many locations, the ancestral arc rocks are unconformably overlain by lava flows of the middle Miocene Columbia River Basalt Group, late Miocene to Pliocene fluvial deposits, or Quaternary olivine-phyric mafic lavas (Pierson et al., 2016).



The western part of the Columbia River Gorge is characterized by massive landslides on the Washington side, and the instability of these land masses is associated with abundant rainfall, high relief, composition and structure of the underlying rocks, tectonic uplift associated with the structural evolution of the Cascade Range and Yakima Fold Belt, and valley-side erosion by the incising Columbia River, which flows across the uplifting terrains (Pierson et al., 2016). The Cascade landslide complex is one such landslide feature that spans from the town of North Bonneville to the western portion of Stevenson. The Cascade landslide complex is subdivided into four individual landslides: the Carpenters Lake, Bonneville, and Red Bluffs landslides, as well as a reactivated part of the Red Bluffs landslide body known as the Crescent Lake landslide. Immediately east of the Cascade landslide complex is the newly recognized Stevenson landslide which is occupied by the City of Stevenson.

The project site is located near the eastern toe of the Red Bluffs landslide, approximately 1-mile east of the reactivated Crescent Lake landslide. The head scarp of the Red Bluffs landslide is located approximately 3½ miles northwest of the site. Surface geology at the site is mapped as Quaternary landslide deposits [Qls] of the Red Bluffs landslide (mass wasting deposits), consisting of poorly sorted blocks, boulders, gravels, and fines sediments produced by the gravitational failure and rotational-translational slide of bedrock and/or unconsolidated sediments above the bedrock (Korosec, 1987).

#### 6.0 SUBSURFACE CONDITIONS

Based on the findings of our field exploration, subsurface soils at the project site include a variably-thick layer of artificial fill soils likely associated with historic site development, atop the native silty gravel with sand stratum (mass wasting deposits). The undocumented artificial fill soils were noted to depths of approximately 3 to 8 feet across the upper portion of the site. Test-pit TP-9 excavated on the lower eastern finger encountered fill to the full depth of exploration (~8 feet) that is believed to represent historic fill placed to create new land. Fill soils were generally classified as silty gravel with sand and variable amounts of cobbles and boulders, and with some areas also including organics, wood debris and miscellaneous trash. The fill soils at the site are likely to be related to the previous historic development at the site. The apparent native underlying soils were classified as Silty Gravel with Sand (GM) and included varying amounts of cobbles and boulders. The native soil stratum typically appeared medium dense. Due to similar soil condition between

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the upper fills and the underlaying native stratum, the fill/native transition was typically ambiguous and therefore not clearly discernable within the test-pits. Test-pit logs in Appendix II show detailed descriptions and stratification of the soils encountered.

## 6.1 NRCS Soil Survey

Although altered at the surface, the soil survey map of the site prepared by the Natural Resources Conservation Service (NRCS) identifies the site soils as *Arents* with typical profile described as *gravelly sandy loam* grading to *extremely gravelly sandy loam*. Based on the NRCS map (Appendix VII), these units generally consist of *well drained* materials.

#### 6.2 Groundwater

Groundwater was encountered within two of the exploratory test-pits at depths ranging from approximately 12 to 14 feet BGS at the time of our exploration in late December. Approximate correlating groundwater elevations ranged from approximately 83' in TP-3 in the western portion, down to 78' in TP-8 near the eastern portion. A review of the Washington Department of Ecology's online water well log database revealed a lack of nearby water wells in the site vicinity. Water levels within the adjacent Rock Cove portion of the Columbia River, controlled by the down-river Bonneville Dam, are typically noted at an elevation approximately 20 to 25 feet below the upper leveled-off site elevation. Therefore, we believe groundwater at the site is not directly affected by pool elevations in the Columbia River, and is likely controlled by the complex hydrogeological conditions of the up-gradient mass-wasting landslide deposits, as well as regional precipitation and snowmelt. Groundwater levels will fluctuate with irrigation, precipitation, drainage, and regional pumping from wells.

#### 7.0 SOIL INFILTRATION TESTING

A single infiltration test was performed on the north side of the existing entrance drive at a depth of approximately 5.5 feet BGS using a small-scale Pilot Infiltration Test (PIT). To the degree possible, care was exercised during excavation to attempt to maintain relatively uniform side walls, and the resulting size and geometry of the finished test-pit was carefully recorded in the field. Water was introduced into the test-pit using a garden hose connected to a nearby fire hydrant. The water flow into the test-pit was continued until the soils with the test-pit were saturated and a



constant flow rate was established. The stabilized inflow rate was measured and recorded, and the resulting un-factored infiltration rates are presented in the table below:

**Table 2: Infiltration Test Results** 

Test ID	Approximate Location (GPS Coordinates)	Soil Tested	Field Infiltration Rate
P-1	45°41'20.69"N, 121°53'56.06"W	Silty Gravel	4 inches/hour

The infiltration rate presented herein represents the un-factored field soil infiltration rate. An appropriate factor of safety should be applied to the field infiltration rate to determine long-term design infiltration rate. Determination of safety factors for long-term design infiltration should consider the following: pretreatment, potential for bio-fouling, system maintainability, horizontal and vertical variability of soils, and type of infiltration testing. Typical factors of safety for these soils generally range from 2 to 3. If stormwater management facilities are selected at other locations, additional site-specific infiltration testing shall be performed.

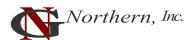
#### 8.0 GEOLOGIC HAZARDS

Potential geologic hazards that may affect the proposed development include: [i] landslides & slope instability, [ii] seismic hazards (ground shaking, surface fault rupture, soil liquefaction, and other secondary earthquake-related hazards), and [iii] flooding & erosion. The perimeter/shoreline edges of the subject property are generally all mapped by the City of Stevenson's Critical Areas & Geologic Hazards Map as 'Potentially Unstable Slope' which refers to an area with slopes of 25% or greater per Stevenson Municipal Code (SMC), Chapter 18.13, Section 18.13.090, Critical Area - Geologically Hazardous Areas. A discussion follows on the specific hazards to this site:

#### 8.1 Landslides

As discussed above in Section 5.0, the project site lies within the Cascade landslide complex that is subdivided into four individual landslides (Carpenters Lake, Bonneville, Red Bluffs, & Crescent Lake landslide). The Bonneville landslide has been dated to have occurred from 1416-1452 A.D. by a combination of dating methods. The Red Bluffs landslide has crosscutting morphologic features suggesting a younger age than that of the Bonneville landslide, with an age range of 1760-1770 A.D. The Crescent Lake landslide has reactivated within the last few decades and currently is moving downslope at an average rate of 11–18 cm/year and possibly as fast as 25 cm/year (Pierson et al., 2016). Results of another recent study (Hu et al., 2015) showed that the central upper part of

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the Crescent Lake landslide moved a total of 700 mm downslope during a 4-year observation period from 2007 to 2011, and that the movement was seasonal and showed a strong correlation with winter precipitation. In contrast to the Crescent Lake landslide, coherent parts of Red Bluffs, Bonneville and Stevenson landslides were observed to remain stable during the observation period.

Although considered a recent landslide (< 1,000 years old), the Red Bluffs landslide is not considered an active landslide (movement in last 20 years). Based on Table 18.13.090-1, Landslide Hazard Classification, of the Stevenson Municipal Code (SMC), the landslide hazard for the site classifies as 'Moderate Hazard'.

## 8.2 Regional Faulting & Surface Fault Rupture

The nearest regional faulting with Quaternary displacement (< 130,000 years) consists of the Faults near The Dalles located approximately 12 miles east of the project site (Czajkowski, 2014). Published slip rates for these faults are listed at less than 0.2 mm/year. For the purposes of this report, an active fault is defined as a fault that has had displacement within the Holocene epoch or last 11,700 years. Due to the lack of any known active fault traces in the immediate site vicinity, surface fault rupture is unlikely to occur at the subject property. While future fault rupture could occur at other locations, rupture would most likely occur along previously established fault traces.

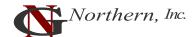
#### 8.3 Earthquakes & Seismic Conditions

Earthquakes caused by movements along crustal faults, generally in the upper 10 to 15 miles, occur on the crust of the North America tectonic plate when built-up stresses near the surface are released. The two largest crustal earthquakes felt in the state of Washington included the 1872, M 6.8 quake near Lake Chelan and the 1936, M 6.0 Walla Walla earthquake. Noteworthy to the City of Stevenson, the Mount Saint Helens Seismic Zone is located approximately 30 miles towards the north-northwest. The following list provides information gathered from the online USGS database regarding historic earthquakes ( $\geq$ 4.0 M) within the past 50 years for epicenters within 100 kilometers of project site, sorted by magnitude (largest to smallest):

Table 3: Earthquakes within 100-kilometers of project site

Date(s) of Event	Magnitude(s)	Nearby Faults / Seismic Zone	Approx. Distance from Site (miles)
March to May, 1980	4.0 - 5.7	Mt. Saint Helens Seismic Zone	33 - 47
March 25, 1993	5.6	Mt. Angel Fault Zone	57
February 14, 1981	5.2	Mt. Saint Helens Seismic Zone	48

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May 13, 1981	4.5	Mt. Saint Helens Seismic Zone	50
June 29, 2002	4.5	Faults near The Dalles	26
March 1, 1982	4.4	Mt. Saint Helens Seismic Zone	48
February 14, 2011	4.3	Mt. Saint Helens Seismic Zone	44
July 14, 2008	4.2	Unknown	60
December 13, 1974	4.1	Faults near The Dalles	33
February 2, 1981	4.0	Toppenish Ridge Fault Zone	59

Based on seismic scenarios published by the Washington State Department of Natural Resources (DNR), M 7.0 Mount Saint Helens and M 7.1 Mill Creek earthquake events would result in a shaking intensity of 'V' (moderate shaking) on the Modified Mercalli Intensity (MMI) scale. We further used the USGS deaggregation tool which provides the relative contributions of hazard for each seismic source based on Probabilistic Seismic Hazard Analysis (PSHA). Based on the deaggregation, it appears that about 23% of the contribution to the probabilistic hazard at the site comes from the Cascadia Subduction Zone, with the remaining contribution primarily from the shallower sources.

## 8.4 Soil Liquefaction

Liquefaction is the loss of soil strength from sudden shock (usually earthquake shaking), causing the soil to become a fluid mass. In general, for the effects of liquefaction to be manifested at the surface, groundwater levels must be within 50 feet of the ground surface and the soils within the saturated zone must also be susceptible to liquefaction. Based on the published Liquefaction Susceptibility Map of of Skamania County, Washington (Palmer et al., 2004a), the site is mapped with a 'low to moderate' relative suceptibility for seismically-induced liquefaction to occur. A detailed assessment of the liquefaction potential at the site, including liquefaction-induced settlement and the effects of lateral spreading, is beyond the scope of this investigation.

## 8.5 Secondary Seismic Hazards

Additional secondary seismic hazards related to ground shaking include ground subsidence, tsunamis, and seiches. The site is far inland, so the hazard from tsunamis is non-existent. The potential hazard of seiches from a significant seismic event is relatively low for development on the upper portion of the project site that is elevated approximately 20 to 25 feet above Rock Cove.



## 8.6 Site Slopes

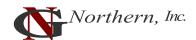
Surface topography across the subject site has been historically altered by previous grading activity related to the preexisting lumber mill facility. The upper historically graded portions of the site are relatively flat at elevations ranging from approximately 95' to 101'. The surrounding edges of the various peninsula fingers typically include relatively steep slopes, with gradients as great as 1H:1V, from the upper flat portions descending down to the shoreline. A field reconnaissance of the subject property was performed to observe site conditions and look for common geomorphic features of landslides as well as indications of possible signs demonstrating recent activity and instability of slide masses. While several areas across the site include a relatively dense cover of vegetation, no apparent indications of recent failures or significant slope instability were observed. Section 9.0 presents results of a preliminary slope stability analysis completed at the site and Section 12.0 provides recommendations for appropriate structure setbacks.

## 8.7 Flooding and Erosion

The subject property is mapped by Federal Emergency Management Agency (FEMA) as Zone 'C' which translates to areas of minimal flooding. Portions of the subject property are however situated in areas where sheet flow and erosion may occur. Soil erodibility is only one of several factors affecting the erosion susceptibility. Soil erosion by water also increases with the length and steepness of the site slopes due to the increased velocity of runoff and resulting greater degree of scour and sediment transport. The need for and design of erosion protection measures is within the purview of the design Civil Engineer. Appropriate erosion and sediment control plan(s) and a drainage plan shall be prepared by the project civil engineer with the final construction drawings. Erosion should be mitigated with appropriate BMPs consisting of proper drainage design including collecting and disposal (conveyance) of water to approved points of discharge in a non-erosive manner. Appropriate project design, construction, and maintenance will be necessary to mitigate the site erosion hazards.

#### 9.0 SLOPE STABILITY ANALYSIS

A preliminary slope stability analysis was conducted for a critical slope section across the southern finger as shown on Figure 2. The analysis was conducted using a generalized geologic cross-section model developed from the existing site topography and data obtained from our subsurface exploration. An output of our slope stability analysis is attached in Appendix VI.



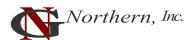
The slope stability analysis was conducted by a two-dimensional limit equilibrium stability analysis of selected trial failure surfaces using the computer program *SLIDE (Version 7)*. Potential circular-arc failure surfaces were evaluated using the Spencer method under static conditions. The computer program searched for critical potential failure surfaces with low computed factors of safety. The computed factor of safety (FS) against slope failure is simply the ratio of total resisting forces or moments (strength of the slope) to the total driving forces or moments for planar or circular failure surfaces respectively. A slope with a factor of safety of 1.0 is in equilibrium, indicating that the disturbing forces driving the slope down are equal to its strength to resist failure. Simply put slope-failure result when the strength of the slope is overcome by gravity.

The selection of unit weight and shear strength parameters for the various earth materials were based on judgment and data obtained during our field investigation, laboratory testing, review of previous studies, research and previous experience with similar materials in similar geotechnical and geologic settings. Engineering and geologic judgment must be applied to the estimated shear strength parameters in order to consider lateral and vertical variations in the subsurface conditions, such as degree of cementation, fracturing, planes of weakness, and gradational characteristics. The following geotechnical strength parameters were used in our stability calculations:

**Table 4: Estimated Strength Parameters** 

	Shear Strengt	Tireid XV/sirelad		
Material	Friction Angle: φ	Cohesion: c (psf)	Unit Weight (pcf)	
Fill/Disturbed Soil	33	25	120	
Native Silty Gravel w/ Sand	35	50	130 (moist) 138 (saturated)	

GN Northern recommends that any existing or reconfigured slopes should meet or be designed and constructed to meet a minimum factor of safety of 1.5 for the static condition and 1.1 under seismic loading. Based on the results of our slope stability analysis, we conclude that the steep perimeter slopes do not meet minimum recommended safety factors. Consequently, the currently proposed layout with future structures sited at/over the edge of slopes is generally considered unfeasible, and remedial grading and/or other appropriate mitigation measures will be required to increase slope safety factors and provide adequate subgrade support for the proposed structures.



In lieu of appropriate remediation of the slope stability concerns, in order to provide sufficient vertical and lateral support for the proposed foundations without significant risk of detrimental settlement, appropriate increased setbacks/embedment for the new building foundations should be maintained. It should be understood however that while the proposed structures may not be at significant risk from slope instability, the existing slopes will remain at risk for some future failure if not appropriately remediated.

#### 10.0 SEISMIC DESIGN PARAMETERS

Based on subsurface data obtained during or field exploration, along with our review of the published NEHRP Site Class Map of Skamania County, Washington (Palmer et al., 2004b), a site class 'D' as defined by 2015 International Building Code (IBC) is applicable. According to Mapped Spectral Acceleration obtained from the USGS Seismic Design Maps using the 2015 IBC, the following site-specific design values may be used:

**Table 5: IBC Design Response Spectra Parameters** 

Seismic Design Parameter	Value (unit)
$S_s$	0.657 (g)
$S_1$	0.292 (g)
Fa	1.274 (unitless)
$F_{\mathbf{v}}$	1.816 (unitless)
$\mathrm{SM}_{\mathrm{s}}$	0.837 (g)
$SM_1$	0.530 (g)
$\mathrm{SD}_{\mathrm{s}}$	0.558 (g)
$\mathrm{SD}_1$	0.354 (g)

 $S_S = MCE$  spectral response acceleration at short periods

It shall be noted that determination of an appropriate site class requires shear wave velocity, soil undrained shear strength, or standard penetration resistance (N-value) data in the upper 100 feet of the subsurface profile, which was beyond the scope of this investigation.

 $S_1 = MCE$  spectral response acceleration at 1-second period

 $F_a$  = Site coefficient for short periods

 $F_v$  = Site coefficient for 1-second period

SM_S = MCE spectral response acceleration at short periods as adjusted for site effects

SM₁ = MCE spectral response acceleration at 1-second period as adjusted for site effects

SD_S = Design spectral response acceleration at short periods

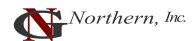
 $SD_1$  = Design spectral response acceleration at 1-second period



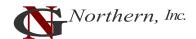
#### 11.0 SUMMARY OF FINDINGS & CONCLUSIONS

Conditions imposed by the proposed development have been evaluated on the basis of assumed elevations and engineering characteristics of the subsurface materials encountered in the exploratory test-pits, and their anticipated behavior both during and after construction. The following is a summary of our findings, conclusions and professional opinions based on the data obtained from a review of selected technical literature and the site evaluation.

- ➤ Based on the findings of this geotechnical evaluation and our understanding of the proposed development, from a geotechnical perspective, it is our opinion that the site is suitable for the proposed development, provided the soil design parameters and site-specific recommendations in this report are followed in the design and construction of the project.
- Final design plans for the proposed development, including grading, drainage and finished elevations, were not provided at the time of this report. Once the plans are finalized, GNN must be provided an opportunity to review final design plans to provide revised recommendations if/as necessary.
- Site soils include a variably-thick layer of artificial fill soils believed to be related to historic site development, atop the native silty gravels with sand. The undocumented artificial fill soils, largely made-up of similar soils that were apparently derived from onsite and/or near sources, extend to depths ranging from 3 to 8 feet and include some areas with miscellaneous trash and debris. Our estimation of the depth of fill materials is based on selected, localized points of exploration, and cannot quantify the full extent of the onsite fill. Additional undocumented fill soils with trash/debris, buried within the subsurface profile, may extend to greater depths at isolated locations across the site.
- ➤ Groundwater was encountered within the two of our test-pits at depths ranging from approximately 12 to 14 feet BGS at the time of our exploration in late December. Approximate correlating groundwater elevations ranged from approximately 83' in TP-3 in the western portion, down to 78' in TP-8 near the eastern portion. We believe groundwater at the site is not directly affected by pool elevations in the Columbia River, and is likely controlled by the complex hydrogeological conditions of the up-gradient mass-wasting landslide deposits, as well as regional precipitation and snowmelt.



- The onsite silty gravel soils, screened and processed to be free of oversize rocks (>5 inches) and any deleterious materials including trash and debris, are generally suitable for reuse as engineered fill and utility trench backfill.
- The proposed building structures may be supported on conventional shallow foundations bearing on a layer of crushed rock atop the recompacted native subgrade in accordance with the recommendations of this report. However, due to presence of artificial fill soils across future building footprints, over-excavation of the existing fill soils to a competent native stratum and replacement with engineered fill will be required.
- Due to ecological constraints, it appears that remedial grading of the onsite slopes to improve long-term stability is not considered feasible. Therefore, deeper embedment of the building foundations will be required in order to meet the minimum setback requirements while ignoring the stability of the onsite slopes.
- Appropriate slope setbacks for future structures should be incorporated in the final planning and design of the project. Slopes setbacks shall adhere to IBC 2015 Section 1808.7 Foundations on or Adjacent to Slopes, as well as the recommendations of this report.
- ➤ Site grading shall incorporate the requirements of IBC 2015, Appendix J *Grading*.
- ➤ Upon completion, all test-pit excavations were loosely backfilled with excavation spoils. The contractor is responsible to locate the test-pits to re-excavate the loose soils and re-place as compacted engineered fill.
- ➤ The underlying geologic condition for seismic design is site class 'D'. The *minimum* seismic design should comply with the 2015 International Building Code (IBC) and ASCE 07-10, Minimum Design Loads for Buildings and Other Structures.
- The near-surface site soils are susceptible to wind and water erosion when exposed during grading operations. Preventative measures and appropriate BMPs to control runoff and reduce erosion should be incorporated into site grading plans.
- ➤ Based on our evaluation, the risk for liquefaction at the project site is considered low to moderate. A site-specific liquefaction analysis to assess the risk of soil liquefaction and liquefaction-induced settlement was beyond the scope of this geotechnical evaluation and would require additional exploration including a 50-foot deep boring with continuous penetration testing.



#### 12.0 GEOTECHNICAL RECOMMENDATIONS

The following geotechnical recommendations are based on our current understanding of the proposed project as shown on the conceptual site plan (Concept D, prepared by FDM Development, dated 10/28/2019), and as described in Section 2.0 of this report. The report is prepared to comply with the 2015 International Building Code Section 1803, Geotechnical Investigations, and as required by Subsection 1803.2, Investigations Required. Please note that Soil Design Parameters and Recommendations presented in this report are predicated upon appropriate geotechnical monitoring and testing of the site preparation and foundation and building pad construction by a representative of GNN's Geotechnical-Engineer-of-Record (GER). Any deviation and nonconformity from this requirement may invalidate, partially or in whole, the following recommendations. We recommend that we be engaged to review grading and foundation plans in order to provide revised, augmented, and/or additional geotechnical recommendations as required.

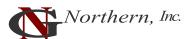
### 12.1 Site Development - Grading

Site grading shall incorporate the requirements of IBC 2015 Appendix J. The project GER or a representative of the GER should observe site clearing, grading, and the bottoms of excavations before placing fills. Local variations in soil conditions may warrant increasing the depth of over-excavation and recompaction. Seasonal weather conditions may adversely affect grading operations. To improve compaction efforts and prevent potential pumping and unstable ground conditions, we suggest performing site grading during dryer periods of the year.

Soil conditions shall be evaluated by in-place density testing, visual evaluation, probing, and proof-rolling of the imported fill and re-compacted on-site soil as it is prepared to check for compliance with recommendations of this report. A moisture-density curve shall be established in accordance with the ASTM D1557 method for all onsite soils and imported fill materials used as structural fill.

## 12.2 Clearing and Grubbing

At the start of site grading, any vegetation, large roots, non-engineered/artificial fill, including trash and debris, and any abandoned underground utilities shall be removed from the proposed building and structural areas. The surface shall be stripped of all topsoil and/or organic growth



(vegetation) that may exist within the proposed structural areas. The topsoil and organic rich soils shall either be stockpiled on-site separately for future use or be removed from the construction area. Depth of stripping can be minimized with real-time onsite observation of sufficient removals. Areas disturbed during clearing shall be properly backfilled and compacted as described below.

## 12.3 Suitability of the Onsite Soils as Engineered Fill

The onsite silty gravel with sand soils, screened and processed to be free of oversize rocks (>5 inches) and deleterious materials including trash and debris, are generally suitable for reuse as engineered fill and utility trench backfill. Suitable onsite soils shall be placed in maximum 8-inch lifts (loose) and compacted to at least 95% relative compaction (ASTM D1557) near its optimum moisture content. Compaction of these soils shall be performed within a range of  $\pm 2\%$  of optimum moisture to achieve the proper degree of compaction.

## 12.4 Temporary Excavations

It shall be the responsibility of the contractor to maintain safe temporary slope configurations since the contractor is at the job site, able to observe the nature and conditions of the slopes and be able to monitor the subsurface conditions encountered. Unsupported vertical cuts deeper than 4 feet are not recommended if worker access is necessary. The cuts shall be adequately sloped, shored or supported to prevent injury to personnel from caving and sloughing. The contractor and subcontractors shall be aware of and familiar with applicable local, state and federal safety regulation including the current OSHA Excavation and Trench Safety Standards, and OSHA Health and Safety Standards for Excavations, 29 CFR Part 1929, or successor regulations.

According to chapter 296-155 of the Washington Administrative Code (WAC), it is our opinion that the soil encountered at the site is classified as Type C soils. We recommend that temporary, unsupported, open cut slopes shall be no steeper than 1.5 feet horizontal to 1.0 feet vertical (1.5H:1V) in Type C soils. No heavy equipment should be allowed near the top of temporary cut slopes unless the cut slopes are adequately braced. Final (permanent) fill slopes should be graded to an angle of 2H:1V or flatter. Where unstable soils are encountered, flatter slopes may be required.



## 12.5 Utility Excavation, Pipe Bedding and Trench Backfill

To provide suitable support and bedding for the pipe, we recommend the utilities be founded on suitable bedding material consisting of clean sand and/or sand & gravel mixture. To minimize trench subgrade disturbance during excavation, the excavator should use a smooth-edged bucket rather than a toothed bucket.

Pipe bedding and pipe zone materials shall conform to Section 9-03.12(3) of the WSDOT Standard Specifications. Pipe bedding should provide a firm uniform cradle for support of the pipes. A minimum 4-inch thickness of bedding material beneath the pipe should be provided. Prior to installation of the pipe, the pipe bedding should be shaped to fit the lower part of the pipe exterior with reasonable closeness to provide uniform support along the pipe. Pipe bedding material should be used as pipe zone backfill and placed in layers and tamped around the pipes to obtain complete contact. To protect the pipe, bedding material should extend at least 6 inches above the top of the pipe.

Placement of bedding material is particularly critical where maintenance of precise grades is essential. Backfill placed within the first 12 inches above utility lines should be compacted to at least 90% of the maximum dry density (ASTM D1557), such that the utility lines are not damaged during backfill placement and compaction. In addition, rock fragments greater than 1 inch in maximum dimension should be excluded from this first lift. The remainder of the utility excavations should be backfilled and compacted to 95% of the maximum dry density as determined by ASTM D1557.

Onsite soils are considered suitable for utility trench backfill provided they are free of oversize material and trash/debris and can be adequately compacted. All excavations should be wide enough to allow for compaction around the haunches of pipes and underground tanks. We recommend that utility trenching, installation, and backfilling conform to all applicable federal, state, and local regulations such as OSHA and WISHA for open excavations.

Compaction of backfill material should be accomplished with soils within  $\pm 2\%$  of their optimum moisture content in order to achieve the minimum specified compaction levels recommended in this report. However, initial lift thickness could be increased to levels recommended by the



### 12.6 Imported Crushed Rock Structural Fill

Imported structural fill shall consist of well-graded, crushed aggregate material meeting the grading requirements of Washington State Department of Transportation (WSDOT) Standard Specification 9-03.9(3) (1-1/4 inch minus Base Course Material) presented here:

Table 6: WSDOT Standard Spec. 9-03.9(3)

(-)				
Sieve Size	Percent Passing (by Weight)			
1 ¹ / ₄ Inch Square	99 - 100			
1 Inch Square	80 - 100			
5/8 Inch Square	50 - 80			
U.S. No. 4	25 - 45			
U.S. No. 40	3 – 18			
U.S. No. 200	Less than 7.5			

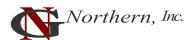
A fifty (50) pound sample of each imported fill material shall be collected by GNN personnel prior to placement to ensure proper gradation and establish the moisture-density relationship (proctor curve).

## 12.7 Compaction Requirements for Engineered Fill

All fill or backfill shall be approved by a representative of the GER, placed in uniform lifts, and compacted to a minimum 95% of the maximum dry density as determined by ASTM D1557. The compaction effort must be verified by a representative of the GER in the field using a nuclear density gauge in accordance with ASTM D6938. The thickness of the loose, non-compacted, lift of structural fill shall not exceed 8 inches for heavy-duty compactors or 4 inches for hand operated compactors.

## 12.8 Building Pad & Foundation Subgrade Preparation

Building structures may be supported on conventional shallow foundations bearing on subgrade prepared in accordance with the recommendations of this report. We recommend that all building foundations, including all exterior footings, interior footings and isolated column footings for any over-hang patio roof/decks, be supported on uniform improved native subgrade support conditions. The minimum footing depth shall be 24 inches below adjacent grades for frost protection and bearing capacity considerations. Interior footings may be supported at nominal depths below the floor. All footings shall be protected against weather and water damage during/after construction.

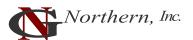


Following completion of site clearing and grubbing operations, all foundation areas shall be over-excavated to expose the native silty gravels. We anticipate the native soils in the vicinity of the currently proposed building footprints will range from depths of approximately 3 to 8 feet BGS. In order to reduce the risk of differential settlement, we recommend the differential in depth of foundation over-excavation (thickness of fill) be limited to 50%; i.e. if the deepest required foundation over-ex is 6 feet, then no portion of the foundation excavation shall be less than 3 feet below footing elevation. The exposed native gravelly stratum shall be moisture-conditioned (as necessary) and proof-compacted to a dense and non-yielding surface. Any soft spots encountered during compaction shall be over-excavated an additional 12 inches and replaced as compacted fill. Although not anticipated, deeper foundation over-excavations may extend into groundwater; consequently, employment of appropriate means of dewatering by the contractor may be required.

Foundation backfill shall consist of suitable screened/processed onsite soils (see *Suitability of Onsite Soils as Engineered Fill*) and/or imported 2-inch minus Gravel Borrow material (meeting the grading and quality requirements of WSDOT Standard Spec. Sec. 9-03.14(1)). The upper 12 inches of backfill directly below the foundations shall consist of imported 1½"-minus crushed rock structural fill placed as engineered fill, moisture-conditioned and compacted to at least 95% of the maximum dry density as determined by the ASTM D1557. Crushed rock structural fill shall extend minimum 12 inches beyond the edges of the footings.

Where future buildings are proposed near or on the existing slopes, building foundations will be required to be constructed with appropriate setbacks in accordance with IBC 2015 Section 1808.7 (see *Slope Setbacks* section below). In general, if buildings are constructed with the current proposed layout, deeper embedment of the foundations will be required in order to meet the minimum setback, such that a minimum distance of 10 feet from the exterior face of the footings to a projected 2H:1V slope face from the toe of the existing slope is maintained. These recommendations may require the need for stepped foundations across the building structure, or deeper foundations such as taller stem-walls or columns.

Footings constructed in accordance with the above recommendations may be designed for an allowable bearing capacity of **2,500 pounds per square foot (psf)**. The allowable bearing pressure may be increased by 1/3 for short-term transient loading conditions. The estimated total settlement



for footings is approximately 1-inch with differential settlement less than half that magnitude. The weight of the foundation concrete below grade may be neglected in dead load computations.

Lateral forces on foundations from short term wind and seismic loading would be resisted by friction at the base of foundations and passive earth pressure against the buried portions. We recommend an allowable passive earth pressure for the compacted onsite soil of **220 pcf**. This lateral foundation resistance value includes a factor of safety of 1.5. We recommend a coefficient of friction of **0.45** be used between cast-in-place concrete and imported crushed rock fill. An appropriate factor of safety should be used to calculate sliding resistance at the base of footings.

#### 12.9 Slab-on-Grade Floors

We recommend placing a minimum 6-inch layer of crushed aggregate fill beneath all slabs. The material shall meet the WSDOT Specification 9-03.9 (3), "Crushed Surfacing Top Course". The crushed rock material shall be compacted to at least 95% of the maximum dry density as determined by the ASTM D1557 method. Prior to placement of crushed aggregate fill, the building pad shall be prepared as described above in the *Building Pad & Foundation Subgrade Preparation* section. We recommend a modulus of subgrade reaction equal to 120 pounds per cubic inch (pci) based on a value for gravel presented in the Portland Cement Association publication No. EB075.01D. Slab thickness, reinforcement and joint spacing shall be determined by a licensed engineer based on the intended use and loading.

An appropriate vapor retarder (15-mil polyethylene liner) shall be used (ASTM E1745/E1643) beneath areas receiving moisture sensitive resilient flooring/VCT where prevention of moisture migration through slab is essential. The slab designer should refer to ACI 302 and/or ACI 360 for procedures and cautions regarding the use and placement of a vapor retarder. The architect shall determine the need and use of a vapor retarder.

### 12.10 Retaining Walls

The following table presents recommendations for lateral earth pressures for use in retaining wall design. The values are given in terms of equivalent fluid pressures without surcharge loads and are based on the assumption that proper drainage is provided behind the wall, the backfill is horizontal and that no-buildup of hydrostatic pressure occurs.



**Table 7: Lateral Earth Pressures** 

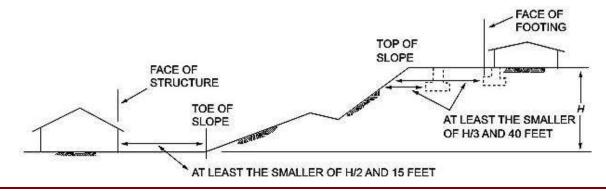
<b>Lateral Pressures</b>	Suitable Onsite Soils
Active Pressure Use when wall is permitted to rotate 0.1 to 0.2% of wall height for granular backfill	38 pcf - level ground
At-Rest Pressure	56 pcf - level ground

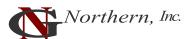
<u>Drainage</u>: Retaining structures should include adequate back drainage to avoid build-up of hydrostatic pressures. Positive drainage for retaining walls should consist of a vertical layer of permeable material (chimney drain), such as a pea gravel or crushed rock (typically ¼- to ¾-inch crushed), at least 18 inches thick, positioned between the retaining wall and the backfill. We recommend installing a non-woven filter fabric such as Mirafi 140N between the drainage material and the general backfill to prevent fines from migrating into the drainage material. A 4-inch diameter perforated or slotted drain-pipe, wrapped or socked in filter fabric, shall be installed at the bottom of the chimney drain.

<u>Backfill and Subgrade Compaction</u>: Compaction on the retained side of the wall within a horizontal distance equal to one wall height should be performed by hand-operated or other lightweight compaction equipment. This is intended to reduce potential locked-in lateral pressures caused by compaction with heavy grading equipment. Retaining wall foundations and subgrade improvements shall be constructed in accordance with the recommendations of this report.

## 12.11 Slope Setbacks

In accordance with IBC 2015 Section 1808.7 *Foundations on or Adjacent to Slopes*: "foundations on or adjacent to slope surfaces shall be founded in firm material with an embedment and setback from the slope surface sufficient to provide vertical and lateral support for the foundation without detrimental settlement." IBC Figure 1808.7.1 (presented below) defines the appropriate minimum setbacks from ascending and descending slope surfaces:





Appropriate setbacks can be accommodated by lateral offset and/or increased embedment. The long-term performance of the structure near slopes is dependent on the protection of slopes from erosion or over steepening from subsequent slope grading. Slopes should be maintained to prevent erosion or undermining of the toe.

#### 12.12 Flexible Pavement

Due to the presence of undocumented fills throughout the project site, remedial grading will be required to minimize the risk of pavement distress. We recommend that the new pavement section be constructed on an improved subgrade. Due to the presence of artificial fills soils that include some miscellaneous trash and debris, the pavement subgrade over-excavation be completed in accordance with one of the following two options:

- (1) Pavement areas shall be fully over-excavated to remove the artificial fill soils. Based on our site exploration, we anticipate that the maximum depth of excavation could be as great as approximately 8 feet.
- (2) Excavate the proposed pavement areas to a minimum depth of 12 inches BGS. We recommend installing a Mirafi 600X geotextile fabric at the bottom of the over-ex. <u>It must be understood that if this option is selected, the owner must accept some risks related to future distresses to the pavements including the potential for settlement and cracking.</u>

After appropriate over-excavation is complete and confirmed by a representative of the GER, the exposed native subgrade shall be moisture-conditioned and compacted to a dense and non-yielding surface. After a suitable subgrade is confirmed by a representative of the GER, the over-excavation shall be backfilled with engineered structural fill soil consisting of suitable/screened onsite soil (see Section 12.3) and/or imported 2-inch minus Gravel Borrow material (meeting the grading and quality requirements of WSDOT Standard Spec. Sec. 9-03.14(1)). Engineered structural fill soils shall be placed in max. 8-inch thick loose lifts and each lift compacted to 95% of ASTM D1557. The following table presents recommended light duty and heavy-duty asphalt pavement sections for proposed project to constructed atop the prepared subgrade:



**Table 8: Recommended Asphalt Concrete Paving Sections** 

Traffic	<b>Asphalt Thickness</b>	<b>Crushed Aggregate Base Course</b>	
Traine	(inches)	(inches)	
Heavy Duty†	4.0	10*	
Standard Duty ††	3.0	6	

[†]Heavy duty applies to pavements subjected to truck traffic and drive lanes

Pavement section recommendations assume proper drainage and construction monitoring. Pavement shall be constructed on a dense and non-yielding surface. All fills used to raise low areas must be compacted structural fills and shall be placed under engineering control conditions.

Soils containing roots or organic materials shall be completely removed from the proposed paved areas prior to subgrade construction. The upper 12 inches of subgrade soils beneath the pavement section shall be moisture conditioned and proof-compacted to a dense and non-yielding condition. All fills used to raise low areas must be compacted onsite soils or structural gravel fill and shall be placed under engineering control conditions. The finished surface shall be smooth, uniform and free of localized weak/soft spots. All subgrade deficiency corrections and drainage provisions shall be made prior to placing the aggregate base course. All underground utilities shall be protected prior to grading.

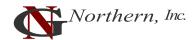
The HMAC utilized for the project should be designed and produced in accordance with Section 5-04 Hot Mix Asphalt of the *Washington Department of Transportation 2014 Standard Specifications for Road and Bridge Construction* (WSDOT Specifications). Aggregate Base material shall comply with Section 9-03.9(3) Crushed Surfacing of the *WSDOT Specifications*. Aggregate base or pavement materials should not be placed when the surface is wet.

#### 12.13 Subgrade Protection

The degree to which construction grading problems develop is expected to be dependent, in part, on the time of year that construction proceeds and the precautions which are taken by the contractor to protect the subgrade. The fine-grained soils currently present on site are considered to be moisture and disturbance sensitive due to their fines content and may become unstable (pumping) if allowed to increase in moisture content and are disturbed (rutted) by construction traffic if wet. If necessary, the construction access road should be covered with a layer of gravel or

^{††}Standard duty applies to general parking areas

^{*}The upper 2" of crushed rock should be top course rock placed over the base course layer



quarry spalls course. The soils are also susceptible to erosion in the presence of moving water. The soils shall be stabilized to minimize the potential of erosion into the foundation excavation. The site shall be graded to prevent water from ponding within construction areas and/or flowing into excavations. Accumulated water must be removed immediately along with any unstable soil. Foundation concrete shall be placed and excavations backfilled as soon as possible to protect the bearing grade. We further recommend that soils that become unstable are to be either:

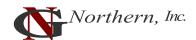
- Removed and replaced with structural compacted gravel fill, or
- Mechanically stabilized with a coarse crushed aggregate (possibly underlain with a geotextile) and compacted into the subgrade.

### 12.14 Surface Drainage

With respect to surface water drainage, we recommend that the ground surface be sloped to drain away from the structure. Final exterior site grades shall promote free and positive drainage from the building areas. Water shall not be allowed to pond or to collect adjacent to foundations or within the immediate building area. We recommend that a gradient of at least 5% for a minimum distance of 10 feet from the building perimeter be provided, except in paved locations. In paved areas, a minimum gradient of 1% should be provided unless provisions are included for collection/disposal of surface water adjacent to the structure. Catch basins, drainage swales, or other drainage facilities should be aptly located. All surface water such as that coming from roof downspouts and catch basins be collected in tight drain lines and carried to a suitable discharge point, such as a storm drain system. Surface water and downspout water should not discharge into a perforated or slotted subdrain, nor should such water discharge onto the ground surface adjacent to the building. Cleanouts should be provided at convenient locations along all drain lines.

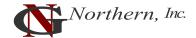
### 12.15 Wet Weather Conditions

The project site soils are fine-grained and sensitive to moisture during handling and compaction. Proceeding with site earthwork operations using these soils during wet weather could add project costs and/or delays. The stability of exposed soils may rapidly deteriorate due to a change in moisture content. Therefore, if possible, complete site clearing, preparation, and earthwork during periods of warm, dry weather when soil moisture can be controlled by aeration. During/subsequent to wet weather, drying or compacting the on-site soils will be difficult. It may be necessary to



amend the on-site soils or import granular materials for use as structural fill. If earthwork takes place in wet weather/conditions, the following recommendations should be followed:

- Fill material should consist of clean, granular soil, and not more than 3% fines (by weight) should pass the No. 200 sieve. Fines should be non-plastic. These soils would have to be imported to the site.
- Earthwork should be accomplished in small sections and carried through to completion to reduce exposure to wet weather. Soils that becomes too wet for compaction should be removed and replaced with clean, granular material.
- The construction area ground surface should be sloped and sealed to reduce water infiltration, to promote rapid runoff, and to prevent water ponding.
- To prevent soil disturbance, the size or type of equipment may have to be limited.
- Work areas and stockpiles should be covered with plastic. Straw bales, straw wattles, geotextile silt fences, and other measures should be used as appropriate to control soil erosion.
- Excavation and fill placement should be observed on a full-time basis by a representative of GER to determine that unsuitable materials are removed and that suitable compaction and site drainage is achieved.



### 13.0 REFERENCES

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- U.S. Department of the Interior, U.S. Geological Survey (USGS), (1979). Bonneville Dam Quadrangle, Washington-Oregon, 7.5 Minute Series (Topographic). DMA 1675 III NW-Series V891.
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- Washington State Department of Natural Resources (DNR), Washington Division of Geology and Earth Resources, on-line mapping tool, https://fortress.wa.gov/dnr/protectiongis/geology/

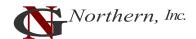


### 14.0 CONTINUING GEOTECHNICAL SERVICES

GNN recommends that the Client should maintain an adequate program of geotechnical consultation, construction monitoring, and soils testing during the final design and construction phases to monitor compliance with GNN's geotechnical recommendations. Maintaining GNN as the geotechnical consultant from beginning to end of the project will provide continuity of services. If GN Northern, Inc. is not retained by the owner/developer and/or the contractor to provide the recommended geotechnical inspections/observations and testing services, the geotechnical engineering firm or testing/inspection firm providing tests and observations shall assume the role and responsibilities of Geotechnical Engineer-of-Record.

GNN can provide construction monitoring and testing as additional services. The costs of these services are not included in our present fee arrangement, but can be obtained from our office. The recommended construction monitoring and testing includes, but is not necessarily limited to, the following:

- Consultation during the design stages of the project.
- ➤ Review of the grading and drainage plans to monitor compliance and proper implementation of the recommendations in GNN's Report.
- ➤ Observation and quality control testing during site preparation, grading, and placement of engineered fill as required by the local building ordinances.
- > Geotechnical engineering consultation as needed during construction



### 15.0 LIMITATIONS OF THE GEOTECHNICAL SITE INVESTIGATION REPORT

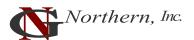
This GEOTECHNICAL SITE INVESTIGATION REPORT ("Report") was prepared for the exclusive use of the Client. GN Northern, Inc.'s (GNN) findings, conclusions and recommendations in this Report are based on selected points of field exploration, and GNN's understanding of the proposed project at the time the Report is prepared. Furthermore, GNN's findings and recommendations are based on the assumption that soil, rock and/or groundwater conditions do not vary significantly from those found at specific exploratory locations at the project site. Variations in soil, bedrock and/or groundwater conditions could exist between and beyond the exploration points. The nature and extent of these variations may not become evident until during or after construction. Variations in soil, bedrock and groundwater may require additional studies, consultation, and revisions to GNN's recommendations in the Report.

In many cases the scope of geotechnical exploration and the test locations are selected by others without consultation from the geotechnical engineer/consultant. GNN assumes no responsibility and, by preparing this Report, does not impliedly or expressly validate the scope of exploration and the test locations selected by others.

This Report's findings are valid as of the issued date of this Report. However, changes in conditions of the subject property or adjoining properties can occur due to passage of time, natural processes, or works of man. In addition, applicable building standards/codes may change over time. Accordingly, findings, conclusions, and recommendations of this Report may be invalidated, wholly or partially, by changes outside of GNN's control. Therefore, this Report is subject to review and shall not be relied upon after a period of **one (1) year** from the issued date of the Report.

In the event that any changes in the nature, design, or location of structures are planned, the findings, conclusions and recommendations contained in this Report shall not be considered valid unless the changes are reviewed by GNN and the findings, conclusions, and recommendations of this Report are modified or verified in writing.

This Report is issued with the understanding that the owner or the owner's representative has the responsibility to bring the findings, conclusions, and recommendations contained herein to the attention of the architect and design professional(s) for the project so that they are incorporated

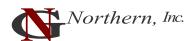


into the plans and construction specifications, and any follow-up addendum for the project. The owner or the owner's representative also has the responsibility to verify that the general contractor and all subcontractors follow such recommendations during construction. It is further understood that the owner or the owner's representative is responsible for submittal of this Report to the appropriate governing agencies. The foregoing notwithstanding, no party other than the Client shall have any right to rely on this Report and GNN shall have no liability to any third party who claims injury due to reliance upon this Report, which is prepared exclusively for Client's use and reliance.

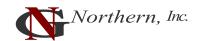
GNN has provided geotechnical services in accordance with generally accepted geotechnical engineering practices in this locality at this time. GNN expressly disclaims all warranties and guarantees, express or implied.

Client shall provide GNN an opportunity to review the final design and specifications so that earthwork, drainage and foundation recommendations may be properly interpreted and implemented in the design and specifications. If GNN is not accorded the review opportunity, GNN shall have no responsibility for misinterpretation of GNN's recommendations.

Although GNN can provide environmental assessment and investigation services for an additional cost, the current scope of GNN's services does not include an environmental assessment or an investigation for the presence or absence of wetlands, hazardous or toxic materials in the soil, surface water, groundwater, or air on, below, or adjacent to the subject property.



### **APPENDICES**

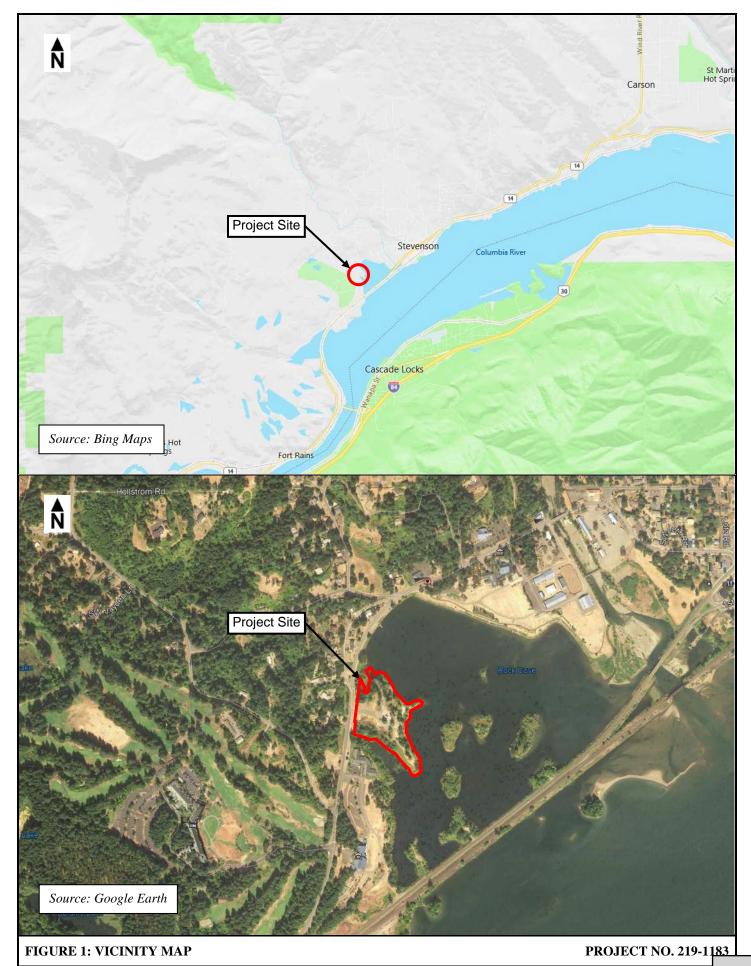


Appendix I

<u>Vicinity Map (Figure 1)</u>

<u>Site Exploration Map (Figure 2)</u>

<u>Critical Areas Map (Figure 3)</u>





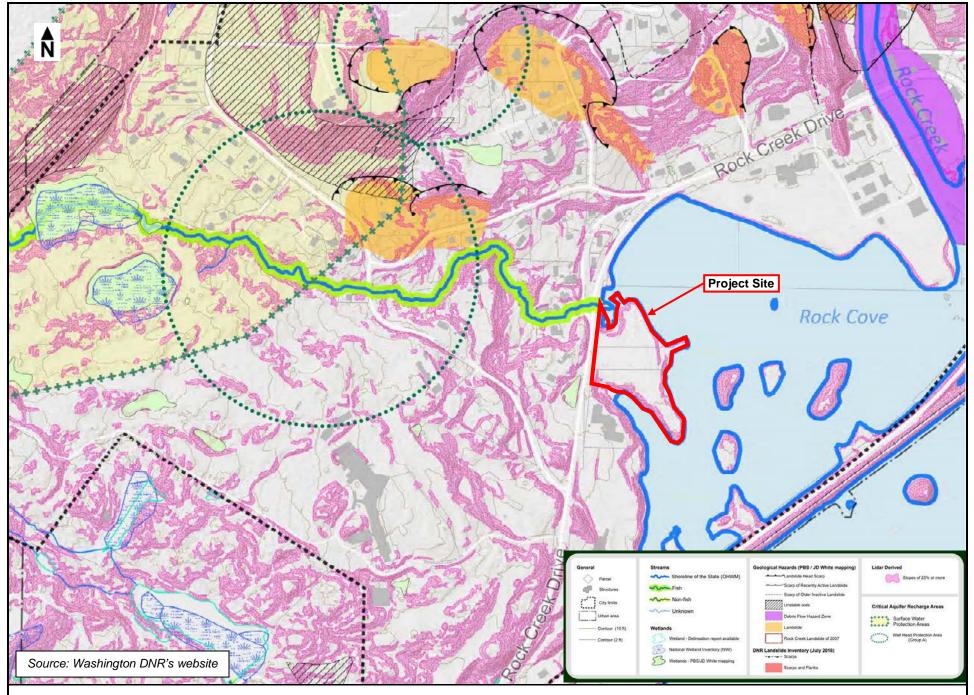
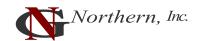


FIGURE 3: CRITICAL AREAS MAP

PROJECT NO. 219-1183



Appendix II

<u>Exploratory Test-Pit Logs</u>

<u>Key Chart (for Soil Classification)</u>

## TEST PIT NUMBER TP-1 PAGE 1 OF 1

6		Fax: (509) 248-4			PROJECT NAME Draw 15 10 10 5
					PROJECT NAME Proposed Rock Creek Cove Development
1		MBER 219-1183			PROJECT LOCATION Rock Creek Drive, Stevenson, WA
					ETED 12/23/19 GROUND ELEVATION 98 ft TEST PIT SIZE 36 x 96 inches
		CONTRACTOR R	_		GROUND WATER LEVELS:
					AT TIME OF EXCAVATION
					ED BY _MYM         AT END OF EXCAVATION           121°53'55.44"W         AFTER EXCAVATION
NOT		JX. GP3 Coolds 4.	J 41 1	9.59 N,	121°53'55.44"W AFTER EXCAVATION
1183 LOGS.GPJ O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION
N219-					~6" to 18" LANDSCAPE CUTTINGS / ORGANIC DEBRIS
183 ROCK CREEK COVE, STEVENSON/2	- - - - -		GP- GM	1	FILL: POORLY GRADED GRAVEL WITH SILT AND SAND, (GP-GM) gray, moist to wet, appears loose to medium dense, with cobbles, with wood and organic debris  - pipe at ~3' BGS
DROPBOXS-ACTIVE PROJECTS\219-1	GB	MC = 22% Fines = 18%			CLAYEY GRAVEL WITH SAND, (GC) brown, wet, appears loose to medium dense, with organics and roots (APPARENT NATIVE)
- C:\USERS\GN NORTHERN   - 0:\USERS\GN NORTHER	- - - -		GC		- becomes blueish gray, moist, appears medium dense (NATIVE)  - with boulders from 10' to 11'
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/13/20 14.05 - C.\USERS\GN NORTHERN\DROPBOX\6.4CT\VERS\CTS\C1.2   1	- - - - - -				4.5
NERAL BH / TP / WEL					<ul> <li>Significant amount of surface water flowing into test-pit excavation</li> <li>Referenced elevations are approximate and based on Survey Topography for Lots 2, 3, and 4 of Rock Creek Cove dated December 11, 2019 prepared by S&amp;F Land Services</li></ul>
<b>В</b>					

### **TEST PIT NUMBER TP-2**

PAGE 1 OF 1

			Fax: (509) 248-4					
			Development				PROJECT NAME Proposed Rock Creek Cove Development	—
	PROJ	ECT NUI	WBER 219-1183				PROJECT LOCATION Rock Creek Drive, Stevenson, WA	
	DATE	STARTE	<b>ED</b> 12/23/19		COMPLETE	<b>ED</b> 12/23/19	GROUND ELEVATION 98 ft TEST PIT SIZE 36 x 96 inches	
			CONTRACTOR R	-				
	EXCA	VATION	METHOD Link-Be	lt 145	x4 Excavato	or	AT TIME OF EXCAVATION	
		_	KAH			BY MYM	AT END OF EXCAVATION	
	NOTE	S Appro	ox. GPS Coords.: 4	5°41'1	18.75"N, 12 ⁻	1°53'55.09"W	AFTER EXCAVATION	
(219-1183 LOGS.GPJ	O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	FILL: SILTY GRAV cobbles, with roots	MATERIAL DESCRIPTION  EL WITH SAND, (GM) brown, angular, moist, appears loose, with	
JECTS/219-1183 ROCK CREEK COVE, STEVENSON	2.5	<b>∰</b> GB	MC = 28% Fines = 47%	GM			brown, appears loose to medium dense, some cobbles	
DROPBOX\5-ACTIVE PRO.	7.5				8.0	- with a significant a	amount of woody debris, organics, roots	90.0
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/13/20 14:05 - C.\USERS\GN NORTHERN\DROPBOX\S-ACTIVE PROJECTS\219-1183 ROCK CREEK COVE, STEVENSON\219-1183 LOGS.GPJ	10.0			GM		dense, with cobble	ITH SAND, (GM) brown, subrounded, moist, appears medium dense to s and boulders (APPARENT NATIVE)	84.0
GENERAL BH / TP / WELL - GI							tions are approximate and based on Google Earth topography Bottom of test pit at 14.0 feet.	4

## TEST PIT NUMBER TP-3 PAGE 1 OF 1

CLIEN	IT FDM	Development			PROJECT NAME Proposed Rock Creek Cove Development	
1					PROJECT LOCATION Rock Creek Drive, Stevenson, WA	
DATE	STARTE	<b>D</b> 12/23/19	(	COMPI	LETED12/23/19         GROUND ELEVATION _ 97 ft         TEST PIT SIZE36 x 96 inches	
		CONTRACTOR R				
					avator $\sqrt{2}$ AT TIME OF EXCAVATION 14.00 ft / Elev 83.00 ft	
		KAH			KED BY MYM AT END OF EXCAVATION	
NOTE		x. GPS Coords.: 4	5°41'2	0.75"N	, 121°53'55.36"W AFTER EXCAVATION	_
183 LOGS.GPJ O DEPTH O (ft)	SAMPLE TYPE NUMBER	TESTS	U.S.C.S.	GRAPHIC LOG	MATERIAL DESCRIPTION	
EEK COVE, STEVENSON/219-11:			GP- GM		FILL: POORLY GRADED GRAVEL WITH SILT AND SAND, (GP-GM) gray brown, angular, wet, appears loose to medium dense, with cobbles	94.0
GENERAL BH / TP / WELL - GINT STD US LAB. GDT - 1/13/20 14:05 - C:\USERS\(GR) NORTHERN\(DR) PROXY5-ACTIVE PROJECTS\(CR) 2   1   1   1   1   1   1   1   1   1			GM		SILTY GRAVEL WITH SAND, (GM) brown, subrounded, moist, appears medium dense, trace cobbles (APPARENT NATIVE)  - becomes blueish gray	94.0
LL - GINT STD US LAB.GDT - 1/13/20 14:05 - C:\USERS\GI	€ GB	MC = 29% Fines = 28%			∑ 14.5  - Groundwater encountered at ∼14' BGS at time of excavation	<u>82.5</u>
/ WEL					<ul> <li>Groundwater encountered at ~14' BGS at time of excavation</li> <li>Referenced elevations are approximate and based on Google Earth topography</li> <li>Bottom of test pit at 14.5 feet.</li> </ul>	
上/ H					bottom of test pit at 14.0 leet.	
RAL B						
GENE					19	5

## TEST PIT NUMBER TP-4 PAGE 1 OF 1

F C E E	PROJE DATE : EXCAV EXCAV	STARTEI /ATION ( /ATION I ED BY _	Devel IBER D 12 CONT METH	219-1183 :/23/19 RACTOR _ OD _Link-E	COMPLETED 12/23/19 Riley Materials Belt 145x4 Excavator	PROJECT LOCATION Rock Creek Drive, Stevenson, WA  GROUND ELEVATION 94.9 ft TEST PIT SIZE 36 x 96 inches  GROUND WATER LEVELS:  AT TIME OF EXCAVATION  AT END OF EXCAVATION	
183 LOGS.GPJ	0.0 (#)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	
SIGN NORTHERNIDROPBOXI5-ACTIVE PROJECTS/219-1183 ROCK CREEK COVE, STEVENSOM/219-1	2.5		GM	1.0 6 7.7 7.7 7.7 7.7 7.7 7.7 7.7 7.7 7.7 7	trace boulders  - chainsaw blade	D, (GM) brown, moist, appears loose to medium dense, some cobbles,	3.9 8.9
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/13/20 14:05	12.5				- Groundwater not encountered at - Referenced elevations are appro		1.9

## TEST PIT NUMBER TP-5 PAGE 1 OF 1

	CLIEN	T FDM		opmen					_ PROJECT NAME _	Proposed Rock	Creek Cove Developn	nent	
											Drive, Stevenson, W		
									_ GROUND ELEVAT		TEST PIT SIZE	36 x 96 in	ches
						145x4 Excavate			<del>-</del>		<b></b>		
						CHECKED					<b>-</b>		
	NOTE	S Appro	X. GP	S Coor	ds.: 45	°41'22.14"N, 12	1°53′53.51′	VV	_ AFTER EXC	AVAIION			
183 LOGS.GPJ	O DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG					MATERIAL DESC	CRIPTION			
N\219-1				7 7 7 7 7		TOPSOIL/SLA	SH/DUFF						
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/1/3/20 14:05 - C:\USERS\GN NORTHERNIDROPBOX\5-ACTIVE PROJECTS\219-1183 ROCK CREEK COVE, STEVENSON\219-1183 LOGS. GPJ	2.5 		GM		5.0	SILTY GRAVE (APPARENT N	L WITH SA ATIVE)	AND, (GM)	ne of excavation nate and based on Go Bottom of test pit	moist, appears m	edium dense, some c		95.9 e 91.9
GENERAL BH / TP / WELL -													197

## TEST PIT NUMBER TP-6 PAGE 1 OF 1

	CLIEN	T FDM	Deve	lopment		PROJECT NAME Proposed Rock Creek Cove Development					
	PROJE	ECT NUM	/IBER	219-1183		PROJECT LOCATION Rock Creek Drive, Stevenson, WA	_				
	DATE	STARTE	<b>D</b> _12	2/23/19	<b>COMPLETED</b> 12/23/19	GROUND ELEVATION 98 ft TEST PIT SIZE 36 x 96 inches					
	EXCA	VATION	CONT	RACTOR _	Riley Materials	_ GROUND WATER LEVELS:					
	EXCA	VATION	METH	OD Link-B	Belt 145x4 Excavator	AT TIME OF EXCAVATION					
	LOGG	ED BY _	KAH		CHECKED BY MYM						
	NOTE	S Appro	x. GP	S Coords.:	45°41'21.16"N, 121°53'53.95"W	AFTER EXCAVATION	_				
NORTHERNIDROPBOXIS-ACTIVE PROJECTS/219-1183 ROCK CREEK COVE, STEVENSON/219-1183 LOGS.GPJ	o DEPTH (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION					
19-11	0.0			9 6 4	~12" CONCRETE SLAB		$\neg$				
ONZ				1.0 مُ الْمُ			97.0				
VENS	_				FILL: BASALTIC GRAVEL/COBBLE	ES, angular, some silty/sandy soil matrix	71.0				
STE,				2.0		Ç	96.0				
SOVE	2.5				FILL: SILTY SAND, (SM) gray, fine	grained, damp to moist, appears medium dense					
Ä			SM	3.0		Ç	95.0				
X R					SILTY GRAVEL WITH SAND, (GM) to dense, with cobbles and boulders	brown, rounded to subrounded, damp to moist, appears medium dense					
ROO				Paris	to delise, with copples and boulders	(AFFANENTINATIVE)					
-1183											
\$\219	5.0			Pari							
JECT											
PRO				Par							
TIVE											
5-AC				Par							
N N	7.5										
N N			GM	90							
ERN											
RTH	_			99							
N N	_										
RS/G	10.0										
:\USE											
)5-C	_										
0 14:0	_										
1/13/2	_			0 12.0		٤	36.0				
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/13/20 14:05 - C:\USERS\GN					- Groundwater not encountered at ti	me of excavation					
AB.G					- Releienced elevations are approxi	mate and based on Google Earth topography Bottom of test pit at 12.0 feet.					
I SN											
TSTE											
- GIN											
ÆLL											
TP / V											
BH/											
RAL											
GENE						198	]				

## TEST PIT NUMBER TP-7 PAGE 1 OF 1

	CLIEN	T FDM	Deve	lopment				_ PROJECT N	IAME Proposed Roo	ck Creek Cove Developn	nent		
	PROJI	ECT NUN	/IBER	219-11	83			_ PROJECT L	OCATION Rock Cre	eek Drive, Stevenson, W	A	_	
	DATE	STARTE	D _12	2/23/19	cc	OMPLETED 1	2/23/19	GROUND E	LEVATION 97.6 ft	TEST PIT SIZE	36 x 96 inches		
	EXCA	VATION	CONT	RACTO	R Riley Mate	erials		GROUND W	ATER LEVELS:				
	EXCA	VATION	METH	OD Lin	k-Belt 145x4	Excavator		AT TII	ME OF EXCAVATION				
	LOGG	ED BY _	KAH		CH	HECKED BY _	MYM	AT END OF EXCAVATION					
	NOTE	S Appro	x. GF	S Coord	ls.: 45°41'19.	86"N, 121°53'	52.14"W	AFTER EXCAVATION					
83 LOGS.GPJ	O DEPTH O (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG				MATERIAI	_ DESCRIPTION				
19-11	0.0			<u>, , , , , , , , , , , , , , , , , , , </u>	~6" TC	OPSOIL						07 1	
EEK COVE, STEVENSON\21	   2.5		GM	3	FILL: S trace b	SILTY GRAVE poulders	E WITH SAND	, (GM) brown, m	oist, appears loose to	o medium dense, some	cobbles,	<u>97.1</u> 94.6	
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/13/20 14:05 - C.\USERS\GN NORTHERN\DROPBOX\S-ACTIVE PROJECTS\219-1183 ROCK CREEK COVE, STEVENSON\219-1183 LOGS.GPJ	5.0 		GM		SILTY (APPA	ndwater not er	ncountered at ti	ime of excavatio		medium dense, some o	obbles	94.6 84.6	
GENERAL BH / TP / WELL - GINT STE					- 116161	ionioca enevalit	опо аго арргохі		est pit at 13.0 feet.	год, арпу	199	9	

- Groundwater not encountered at time of excavation
- Referenced elevations are approximate and based on Google Earth topography

  Bottom of test pit at 13.0 feet.

199

### **TEST PIT NUMBER TP-8**

PAGE 1 OF 1

				248-42		
	IT <u>FDM</u>					
	ECT NUM					
						GROUND ELEVATION 89.5 ft TEST PIT SIZE 36 x 96 inches
					ey Materials	
					145x4 Excavator	
	ED BY _				<del></del>	AT END OF EXCAVATION
NOTE		JX. GF	3 0001	us 40	°41'20.44"N, 121°53'51.63"W	AFTER EXCAVATION
183 LOGS.GPJ O DEPTH O (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG			MATERIAL DESCRIPTION
219-11					FILL: SILTY GRAVEL WITH SAND,	(GM) brown, moist, appears loose, some cobbles
COVE, STEVENSON 2: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2: 2:		GM			SILTY GRAVEL WITH SAND, (GM) (APPARENT NATIVE)	brown, damp to moist, appears medium dense, some cobbles
GENERAL BH / TP / WELL - GINT STD US LAB GDT - 1/13/20 14:05 - C:\USERS\GN NORTHERN\DROX\S-ACTIVE PROJECTS\Z19-1183 ROCK CREEK COVE, STEVENSON\Z19-1183 LOGS GPJ		GM			- becomes moist to wet	
- GINT STD US LAB.GDT - 1/13/.				<u>∇</u> 14.5		7
WELL					<ul> <li>Groundwater encountered at ~12' E</li> <li>Referenced elevations are approxin</li> </ul>	GS at time of excavation nate and based on Google Earth topography
ERAL BH / TP / '						Bottom of test pit at 14.5 feet.
E E E						200

### **TEST PIT NUMBER TP-9**

PAGE 1 OF 1

CLIEN	IT FDM	Devel	opmer	nt				PROJECT NAME _	Propos	sed Rock C	Creek C	Cove Developr	nent	_
PROJ	ECT NUM	IBER	219-1	1183				PROJECT LOCATI	ON R	ock Creek	Drive,	Stevenson, W	/A	
DATE	STARTE	<b>D</b> 12	/23/19	)	COMPLETE	ED 12/23/19		GROUND ELEVATI	ION <u>8</u>	7 ft	т	EST PIT SIZE	36 x	96 inches
EXCA	VATION (	CONT	RACTO	OR Riley	Materials			GROUND WATER	LEVEL	S:				
EXCA	VATION I	METH	OD L	ink-Belt 1	45x4 Excavato	or		AT TIME OF	EXCAV	ATION				
LOGG	ED BY	KAH			CHECKED	BY MYM		AT END OF E	EXCAV	ATION	-			
NOTES Approx. GPS Coords.: 45°41'20.74"N, 121°53'49.97"W								AFTER EXC	AVATIC	ON				
o DEPTH (ft)	SAN							MATERIAL DESC	CRIPTIO	ON				
				1.0F	ILL: SILTY GF	SOIL/ORGANIO RAVEL WITH S (APPARENT FI		GM) brown, moist, ap	opears	loose to m	nedium	dense, some	 cobble	8 <u>6.0</u>
2.5 2.5 						( · · · · · · · · · · · · · · · · · · ·	,							
5.0		GM												

- Groundwater not encountered at time of excavation
   Referenced elevations are approximate and based on Google Earth topography Bottom of test pit at 8.0 feet.

7.5

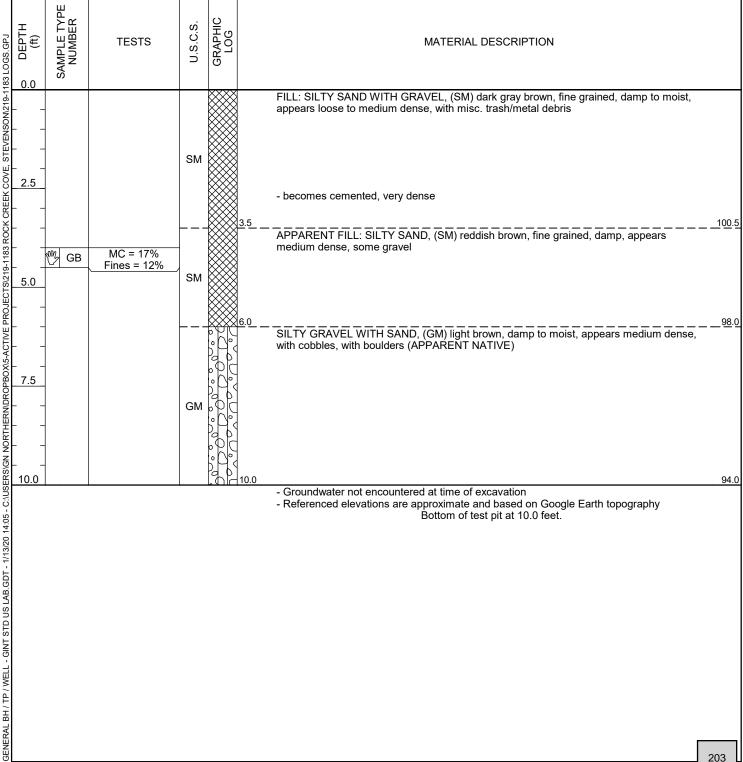
79.0

## TEST PIT NUMBER TP-10 PAGE 1 OF 1

CLIEN	T FDM		lopment		PROJECT NAME Proposed Rock Creek Cove Development	
PROJ	ECT NUM	/IBER	219-118	33	PROJECT LOCATION Rock Creek Drive, Stevenson, WA	
DATE	STARTE	D 12	2/23/19	COMPLETED _12/23/19	GROUND ELEVATION 100.3 ft TEST PIT SIZE 36 x 96 inches	<u> </u>
EXCA	VATION	CONT	RACTOR	R _Riley Materials	GROUND WATER LEVELS:	
EXCA	VATION	METH	IOD Link	k-Belt 145x4 Excavator	AT TIME OF EXCAVATION	
LOGG	ED BY _	KAH		CHECKED BY MYM	AT END OF EXCAVATION	
NOTE	S Appro	x. GF	S Coord	s.: 45°41'15.46"N, 121°53'49.93"W	AFTER EXCAVATION	
183 LOGS. GPJ O DEPTH O (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION	
OCK CREEK COVE, STEVENSONIZ19-118		GM		cobbles	L WITH SAND, (GM) brown, moist, appears loose to medium dense, some	
GENERAL BH / TP / WELL - GINT STD US LAB. GDT - 1/13/20 14:05 - C.: USERSIGN NORTHERNIDROPBOX/5-ACTIVE PROJECTS/219-1183 ROCK CREEK COVE, STEVENSON/219-1183 LOGS. GPJ  1		GM			M) light brown, damp to moist, appears medium dense, some roots in upper NATIVE)	<u>96.3</u>
LAB.GDT - 1/13/20 14:05 - C.\USERS\GN NORT			13 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0			87.3
GENERAL BH / TP / WELL - GINT STD US.				- Groundwater not encountered at	time of excavation eximate and based on Google Earth topography Bottom of test pit at 13.0 feet.	

### **TEST PIT NUMBER TP-11**

CLIENT FDM Development	PROJECT NAME Proposed Rock Creek Cove Development
PROJECT NUMBER 219-1183	PROJECT LOCATION Rock Creek Drive, Stevenson, WA
DATE STARTED         12/23/19         COMPLETED         12/23/19	GROUND ELEVATION 104 ft TEST PIT SIZE 36 x 96 inches
EXCAVATION CONTRACTOR Riley Materials	GROUND WATER LEVELS:
EXCAVATION METHOD Link-Belt 145x4 Excavator	AT TIME OF EXCAVATION
LOGGED BY KAH CHECKED BY MYM	AT END OF EXCAVATION
NOTES Approx. GPS Coords.: 45°41'16.39"N, 121°53'50.59"W	AFTER EXCAVATION



- Groundwater not encountered at time of excavation
- Referenced elevations are approximate and based on Google Earth topography Bottom of test pit at 10.0 feet.

# GN Northern Inc.

11115 E. Montgomery, Suite C Spokane Valley, WA, 99206 Telephone: (509) 248-9798

## TEST PIT NUMBER TP-12 PAGE 1 OF 1

0	IT [D.4		: (509) 248		DDO IFOT NAME. Draw and Dook Creak Cova Davidor mant					
	IT <u>FDM</u>		-		<del></del>					
			219-1183		PROJECT LOCATION Rock Creek Drive, Stevenson, WA					
	STARTE			COMPLETED <u>12/23/19</u>						
				Riley Materials						
				Selt 145x4 Excavator	AT TIME OF EXCAVATION					
	SED BY _				AT END OF EXCAVATION					
NOTE	S Appro	X. GP	S Coords.:	45°41'17.30"N, 121°53'51.73"W	AFTER EXCAVATION					
183 LOGS.GPJ O DEPTH O (ft)	SAMPLE TYPE NUMBER	U.S.C.S.	GRAPHIC LOG		MATERIAL DESCRIPTION					
119-1			71 7	~12" TOPSOIL/DUFF						
CREEK COVE, STEVENSONIZ			1.0	APPARENT FILL: SILTY GRAVEL W	/ITH SAND, (GM) brown, damp, appears medium dense, some roots					
E PROJECTS/219-1183 ROCK		GM	6.0		93.5					
4:05 - C:\USERS\GN NORTHERN\DROPBOX\5-ACTIVE		GM		NATIVE)	ight brown, damp, appears medium dense, some cobbles (APPARENT					
GENERAL BH / TP / WELL - GINT STD US LAB.GDT - 1/13/20 14:05 - C:\USERS\GN NORTHER\NDROPBOX\5-ACTIVE PROJECTS\\219-1183 ROCK CREEK COVE, STEVENSON\\219-1183 LOGS.GPJ   0 DEPT   0 DEPT				- Groundwater not encountered at tim - Referenced elevations are approxim	le of excavation late and based on Google Earth topography Bottom of test pit at 11.0 feet.					

### **KEY CHART**

RELATIVE DENSITY OR CONSISTENCY VERSUS SPT N-VALUE						
COARSE-GRAINED SOILS			FINE-GRAINED SOILS			
DENSITY	N (BLOWS/FT)	FIELD TEST	CONSISTENCY	N (BLOWS/FT)	FIELD TEST	
Very Loose	0 – 4	Easily penetrated with ½-inch reinforcing rod pushed by hand	Very Soft	0-2	Easily penetrated several inches by thumb	
Loose	4 – 10	Difficult to penetrate with ½-inch reinforcing rod pushed by hand	Soft	2 – 4	Easily penetrated one inch by thumb	
Medium -Dense	10 – 30	Easily penetrated with ½-inch rod driven with a 5-lb hammer	Medium-Stiff	4 – 8	Penetrated over ½-inch by thumb with moderate effort	
Dense	30 – 50	Difficult to penetrate with ½-inch rod driven with a 5-lb hammer	Stiff	8 – 15	Indented about ½-inch by thumb but penetrated with great effort	
Very Dense	> 50	penetrated only a few inches with ½-inch rod driven with a 5-lb hammer	Very Stiff	15 – 30	Readily indented by thumb	
			Hard	> 30	Indented with difficulty by thumbnail	

USCS SOIL CLASSIFICATION						
MAJOR DIVISIONS				GROUP DESCRIPTION		
Coarse- Grained Soils <50% passes #200 sieve	Gravel and	Gravel (with little or no fines)	엻	GW	Well-graded Gravel	
	Gravelly Soils <50% coarse fraction passes		1	GP	Poorly Graded Gravel	
		Gravel (with >12% fines)		GM	Silty Gravel	
	#4 sieve			GC	Clayey Gravel	
	Sand and Sandy Soils >50% coarse fraction passes #4 sieve	Sand (with little or no fines)		SW	Well-graded Sand	
				SP	Poorly graded Sand	
		Sand (with >12% fines)		SM	Silty Sand	
			//	SC	Clayey Sand	
Fine- Grained Soils	Silt and Clay Liquid Limit < 50			ML	Silt	
				CL	Lean Clay	
				OL	Organic Silt and Clay (low plasticity)	
>50% passes #200 sieve	<b>Silt</b> and <b>Clay</b> Liquid Limit > 50			MH	Inorganic Silt	
				СН	Inorganic Clay	
				ОН	Organic Clay and Silt (med. to high plasticity)	
Highly Organic Soils				PT	Peat Top Soil	

	LOG SYMBOLS				
X	2S	2" OD Split Spoon (SPT)			
	3S	3" OD Split Spoon			
	NS	Non-Standard Split Spoon			
	ST	Shelby Tube			
	CR	Core Run			
	BG	Bag Sample			
M	TV	Torvane Reading			
I	PP	Penetrometer Reading			
	NR	No Recovery			
<u></u>	GW	Groundwater Table			

Modifiers			
DESCRIPTION	RANGE		
Trace	<5%		
Little	5% – 12%		
Some	>12%		

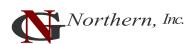
MOISTURE CONTENT				
DESCRIPTION FIELD OBSERVATION				
Dry	Absence of moisture, dusty, dry to the touch			
Moist	Damp but not visible water			
Wet	Visible free water			

### MAJOR DIVISIONS WITH GRAIN SIZE SIEVE SIZE 12" 3" 3/4" 40 4 10 200 GRAIN SIZE (INCHES) 12 0.0029 0.75 0.19 0.0171Gravel Sand Boulders Cobbles Silt and Clay Coarse Fine Coarse Medium Fine

### SOIL CLASSIFICATION INCLUDES

- 1. Group Name
- 2. Group Symbol
- 3. Color
- 4. Moisture content
- 5. Density / consistency
- 6. Cementation
- 7. Particle size (if applicable)
- 8. Odor (if present)
- 9. Comments

Conditions shown on boring and testpit logs represent our observations at the time and location of the fieldwork, modifications based on lab test, analysis, and geological and engineering judgment. These conditions may not exist at other times and locations, even in close proximity thereof. This information was gathered as part of our investigation, and we are not responsible for any use or interpretation of the information by others.



## Appendix III Laboratory Testing Results

### **GRAIN SIZE DISTRIBUTION**

%Silt

17.9

47.0

28.2

11.7

%Clay

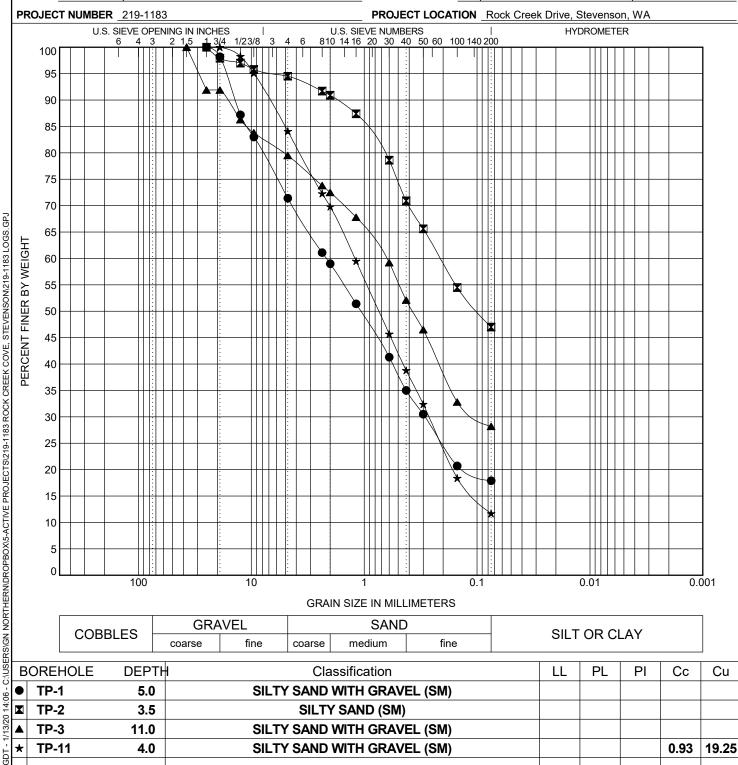
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GN Northern Inc. 11115 E. Montgomery, Suite C Spokane Valley, WA, 99206 Telephone: (509) 248-9798 Fax: (509) 248-4220

CLIENT FDM Development

PROJECT NAME Proposed Rock Creek Cove Development



**BOREHOLE** DEPTH D100 D60 D30 D10 %Gravel %Sand lacktrianTP-1 5.0 25 2.164 0.29 28.6 53.5  $\blacksquare$ 3.5 0.211 TP-2 25 5.5 47.5 TP-3 11.0 37.5 0.639 0.098 20.5 51.3 * **TP-11** 4.0 19 1.211 0.266 15.9 72.4



# Appendix IV Site & Exploration Photographs



Excavation of test-pit TP-1, looking west



Exposed subsurface soil profile within test-pit TP-1



Excavation of test-pit TP-2, looking southwest



Exposed subsurface soil profile within test-pit TP-2



Excavation of test-pit TP-3, looking west



Exposed subsurface soil profile within test-pit TP-3

PLATE 1: SITE & EXPLORATION PHOTOGRAPHS



View of site conditions near test-pit TP-4



Exposed subsurface soil profile within test-pit TP-4



Excavation of test-pit TP-5, looking east



Exposed subsurface soil profile within test-pit TP-5



Excavation of test-pit TP-6, looking north



Exposed subsurface soil profile within test-pit TP-6

PLATE 2: SITE & EXPLORATION PHOTOGRAPHS



PLATE 3: SITE & EXPLORATION PHOTOGRAPHS





Excavation of test-pit TP-12, looking southwest



Exposed subsurface soil profile within test-pit TP-12



View of site conditions near test-pit TP-12, looking northwest



Infiltration test setup at test-pit P-1

PLATE 4: SITE & EXPLORATION PHOTOGRAPHS



## Appendix V Historic Aerial Photographs



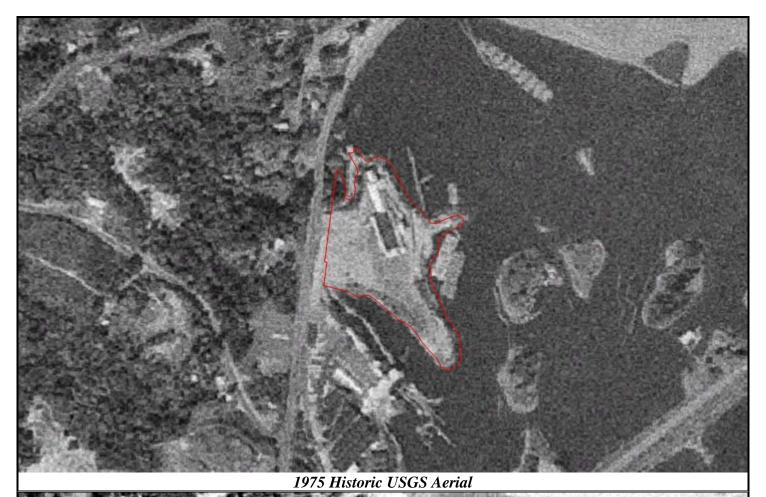




1955 Historic USGS Aerial



PLATE 2: HISTORIC AERIAL PHOTOGRAPHS



1979 Historic USGS Aerial



1981 Historic USGS Aerial



PLATE 4: HISTORIC AERIAL PHOTOGRAPHS



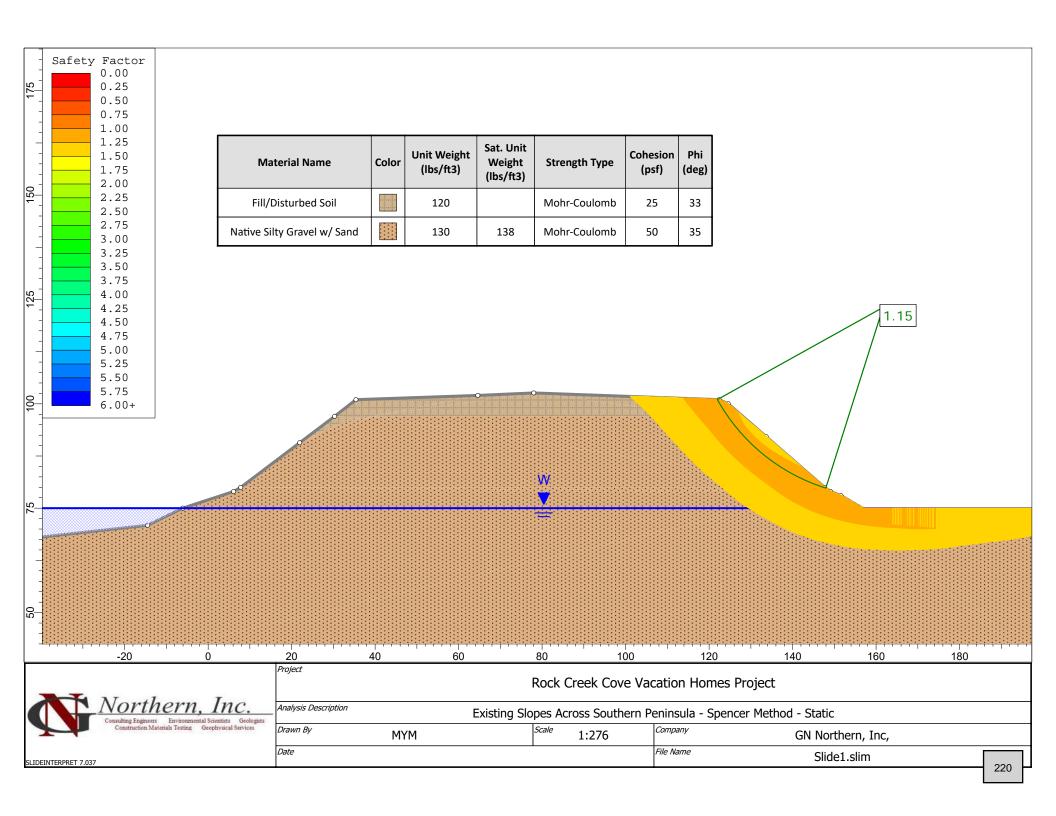
1993 Historic USGS Aerial



PLATE 5: HISTORIC AERIAL PHOTOGRAPHS



# Appendix VI Slope Stability Analysis





# Appendix VII NRCS Soil Survey



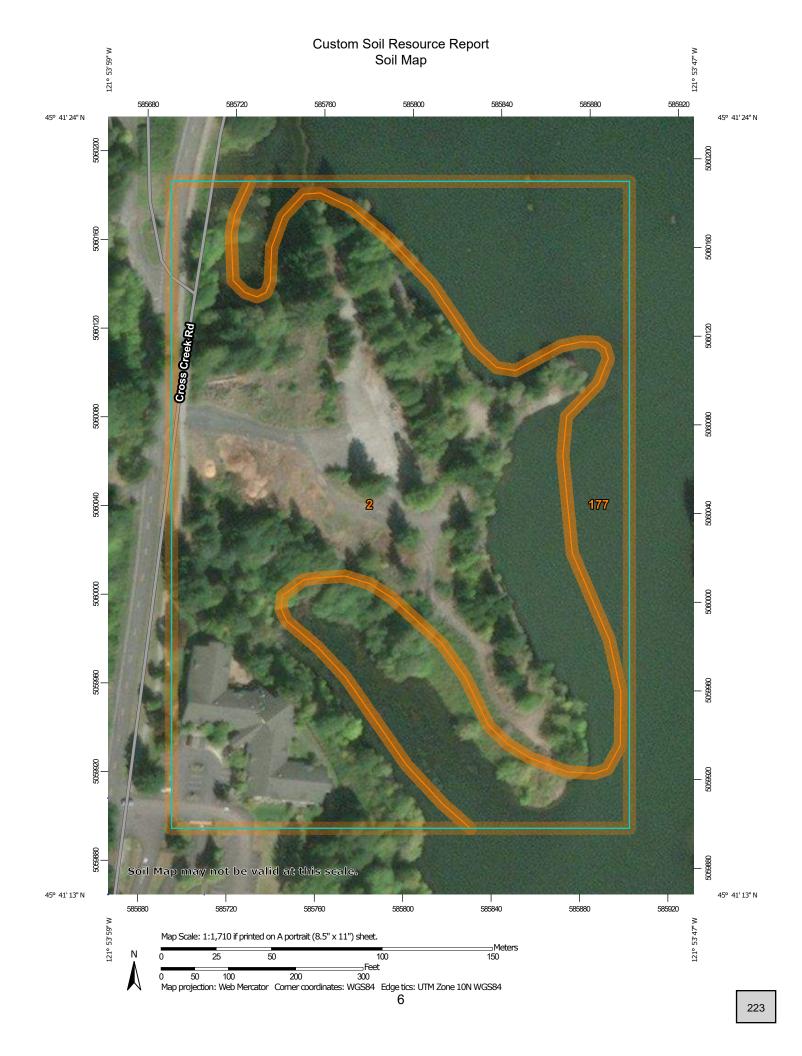
**VRCS** 

Natural Resources Conservation Service A product of the National Cooperative Soil Survey, a joint effort of the United States Department of Agriculture and other Federal agencies, State agencies including the Agricultural Experiment Stations, and local participants

# Custom Soil Resource Report for Skamania County Area, Washington

**Rock Creek Cove Vacation Homes Project** 





# Skamania County Area, Washington

## 2-Arents, 0 to 5 percent slopes

#### **Map Unit Setting**

National map unit symbol: 1hhrw

Elevation: 0 to 200 feet

Mean annual precipitation: 40 to 80 inches Mean annual air temperature: 45 to 52 degrees F

Frost-free period: 90 to 200 days

Farmland classification: Farmland of statewide importance

#### **Map Unit Composition**

Arents and similar soils: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

#### **Description of Arents**

#### Setting

Landform: Terraces

#### **Typical profile**

H1 - 0 to 24 inches: gravelly sandy loam

H2 - 24 to 60 inches: extremely gravelly sandy loam

#### Properties and qualities

Slope: 0 to 5 percent

Depth to restrictive feature: More than 80 inches

Natural drainage class: Well drained

Capacity of the most limiting layer to transmit water (Ksat): Moderately high to

high (0.57 to 5.95 in/hr)

Depth to water table: More than 80 inches

Frequency of flooding: None Frequency of ponding: None

Available water storage in profile: Moderate (about 6.3 inches)

#### Interpretive groups

Land capability classification (irrigated): None specified

Land capability classification (nonirrigated): 3s

Hydrologic Soil Group: A Hydric soil rating: No

#### 177—Water

### **Map Unit Composition**

Water: 100 percent

Estimates are based on observations, descriptions, and transects of the mapunit.

10 224

# <u>APPENDIX 5</u> – Critical Areas Assessment



January 21, 2020

Zachary Pyle, PE
Development Manager
FDM Development, Inc.
5453 Ridgeline Dr #160
Kennewick, WA 99338
zpyle@fdmdevelopment.com
(210) 849-5592

Re: Rock Cove Preliminary Critical Areas Assessment

#### Zach,

Ecological Land Services (ELS) completed a field assessment for FDM Development to determine whether wetlands or fish and wildlife habitat conservation areas (hereafter collectively termed critical areas) are located on or adjacent to parcels 02070100130300, 02070100130400, and 02070100130200 (hereafter referred to as the study area) in the City of Stevenson, Skamania County, Washington. The study area is in the SW ¼ of the NW ¼ of Section 1, Township 2 N, and Range 7 East of the Willamette Meridian, coordinates 45.6890, -121.8992, and accessed from Rock Cove Drive (Figure 1). City of Stevenson zoning is "Commercial Recreation" (CR).

ELS completed fieldwork for a critical areas determination on December 30, 2019 in collaboration with Washington Department of Ecology (Ecology) staff. This letter provides a description of the study area's existing conditions as observed on December 30th and a summary of critical areas findings in accordance with Stevenson Municipal Code (SMC), Title 18 "Environmental Protection", Chapters 18.08 "Shoreline Management" and 18.13 "Critical Areas and Natural Resource Lands", and Stevenson's Shoreline Master Programs (SMP) dated 1977 (approved) and 2018 (in review).

#### Site Description

The study area consists of three parcels that form a peninsula in Rock Cove; Rock Cove is a side channel of the Columbia River formed by the berm for Lewis and Clark Hwy (WA 14) and an adjacent railroad. An unnamed tributary enters Rock Cove north of the study area and Rock Creek enters Rock Cove to the east (Figure 3). An open connection between Rock Cove and the Columbia River is present at its confluence with Rock Creek, southeast of the study area. The study area is currently undeveloped (there are no buildings) but it retains improvements from prior industrial land uses that include concrete and gravel surfaces, gravel roads accessing various points within the study area, a graveled boat launch, and riprap embankments that span the majority of shoreline. A line of abandoned wooden pilings is located just offshore northeast.

Dominant vegetation in the study area included Douglas fir (*Pseudotsuga menziesii*) and red alder (*Alnus rubra*) with Himalayan blackberry (*Rubus armeniacus*) in the understory and rooted in riprap along the

shoreline, and clusters of reed canarygrass (*Phalaris arundinacea*) and soft rush (*Juncus effuses*) rooted in places along the water's edge, at the head of sediment bars and mudflats, and along the river's ordinary high water mark (OHWM).

#### Methods

ELS followed the U.S. Army Corps of Engineers (Corps) Routine Determination Method described in the "Wetland Delineation Manual" (Environmental Laboratory 1987) and the "Regional Supplement to the Corps of Engineers Wetland Delineation Manual: Western Mountains, Valleys, and Coast Region (Version 2.0)" (Corps 2010). To make determinations about the presence of wetland in the study area. For regulatory purposes under the Clean Water Act (Section 404) the Environmental Protection Agency (EPA) defines wetlands as "those areas that are inundated or saturated by surface or ground water at a frequency and duration sufficient to support, and that under normal circumstances do support, a prevalence of vegetation typically adapted for life in saturated soil conditions" (EPA 2014). Wetlands are regulated as "Waters of the United States" by the Corps, as "Waters of the State" by Ecology, and locally by the City of Stevenson.

The Revised Code of Washington (RCW) 90.58.030(2)(b) and Washington Administrative Code (WAC) 173-22-030(11), defines ordinary high water mark as the action of water "so common and usual and so long continued in all ordinary years as to mark upon the soil a character distinct from that of the abutting upland." In collaboration with Ecology staff, ELS used principles in this guidance to identify transitions in vegetation, wrack lines, scouring under trees and exposed roots, and breaks in topography to distinguish the OHWM of the Columbia River along the study area boundary. Ecology and ELS flagged the OHWM with consecutively numbered orange tape flagging. The flag locations were professionally surveyed by S&F Land Services.

#### Critical areas findings

ELS and Ecology identified one unnamed tributary north of the study area (Figures 2 and 3). The tributary is identified as a Type F (fish-bearing) water by Washington Department of Natural Resources (DNR) (Figure 4). Rock Creek is east of the study area and is designated as Type S, a shoreline of the state. Rock Cove surrounds the study area on three sides. The Columbia River is designated Type S and is a shoreline of statewide significance. There were no wetlands or other surface waters in the study area, and no priority habitat for terrestrial wildlife. According to SMC 18.13.095(D), the area designated as a fish and wildlife habitat conservation area (FWHCA) for Type F waters is 100 feet and for Type S waters, 150 feet. SMC 18.13.095(D)(3) addresses functionally isolated buffers, indicating areas that "do not protect the FWHCA from adverse impacts due to features such as "lawns, pre-existing roads, structures, or vertical separation" are exempt from buffer criteria. Accordingly, portions of the study area are exempt from the FWHCA for Rock Cove due to areas of maintained vegetation and the presence of riprap which is both structural and vertical separation from Rock Cove (Figure 2).

SMC 18.13.095(D)(6) outlines provisions for buffer averaging or riparian habitat buffer reduction with mitigation to allow reasonable use of a parcel.

¹ Table 18.13.095-1 - Fish & Wildlife Habitat Conservation Area Protective Buffer Widths

Averaged buffers must meet the following conditions:

- a. There are no feasible alternatives to the site design
- b. The averaged buffer will not result in degradation of the FWHCA's functions and values.
- c. The total buffer area after averaging is equal to the area required without averaging.
- d. The buffer at its narrowest point is never less than 75% of the required base buffer width.

Reduced buffers must meet the following conditions:

- a. mitigation involves restoration or enhancement of all remaining buffers.
- b. Conservation covenants shall--and performance bonds may--be required.
- c. Reduced buffers do not result in a net loss of existing buffer functions.

### December 2018 SMP requirements

The standard shoreline management area (or shoreline setback) for all designated shorelines is 200 feet, measured landward from the OHWM. The study area is zoned "active waterfront"; according to the 2018 SMP, setbacks for development proposed in active waterfront is typically 50 feet.²

Regarding improvements from prior industrial land uses including concrete and gravel surfaces, gravel roads, the graveled boat launch, and riprap embankments, the following condition applies:

A shoreline use that was lawfully constructed prior to the effective date of the SMA or the December 2018 SMP and that does not conform to the current SMP standards is considered a nonconforming use. For the purposes of the December 2018 SMP, existing roads (whether asphalt, gravel, or dirt) are considered nonconforming uses and do not need a Shoreline Conditional Use Permit to be retained or improved (SMP 2018).

Thank you for the opportunity to provide this information. The findings in this letter are intended for FDM Development's planning strategy and should be considered preliminary until they're reviewed and approved in writing by the City of Stevenson and Washington Department of Ecology. If you have any questions, please contact me by phone (360) 578-1371 or email andrew@eco-land.com.

Sincerely,

Andrew R. Allison

Wetland Scientist, Principal

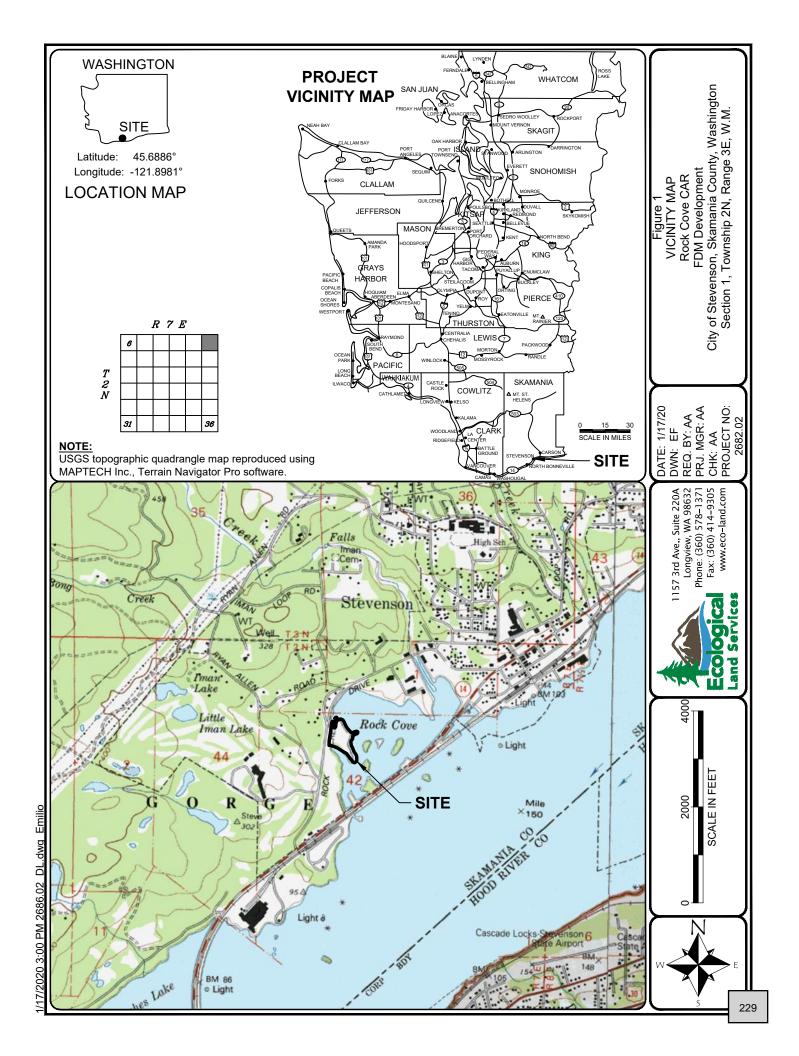
Attachments:

Figures 1-4

Photoplates 1-4

City of Stevenson 2018 SMP "Table 5.1 Shoreline Use & Setback Standards"

² Tables identifying setback distances per development type are attached to this letter for reference.





Site Boundary

**OHWM** 

Stream with Flow Direction

FWHCA Buffer for Type F

FWHCA Buffer for Type S Shoreline Management Plan Setback

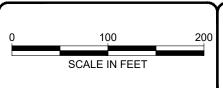
Culvert

Existing Graveled or Concrete Surfacing

Existing Rip Rap

- Aerial from Google Earth™. 1.
- OHWM line was determined through a joint effort by **Ecological Land Services and Washington Department** of Ecology on December 30, 2019. OHWM flags were professionally surveyed by S&F Land Services December 30-31, 2019.
- FWHCA buffer is functionally isolated along existing riprap and existing graveled or concrete surfacing.



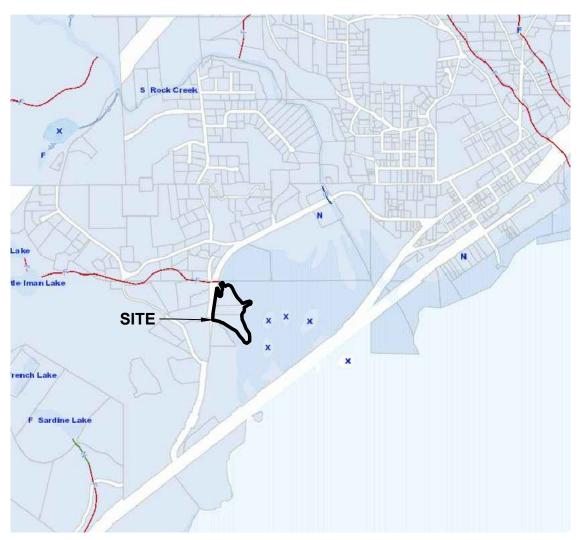




DWN: EF 1157 3rd Ave., Suite 220A Longview, WA 98632 Phone: (360) 578–1371 Fax: (360) 414-9305 CHK: AA www.eco-land.com

DATE: 1/17/20 REQ. BY: AA PRJ. MGR: AA PROJECT NO: 2682.02

Figure 2 SITE MAP Rock Cove CAR FDM Development City of Stevenson, Skamania County, Washington Section 1, Township 2N, Range 3E, W.M.



No mapped streams indicated onsite by the Washington State Department of Natural Resources (DNR).

## **LEGEND**:

# Streams Streams Type S Type F Type N, Np, Ns U, unknown X, non-typed per WAC 222-16

<u>NOTE:</u> Map provided online by Washington State Department of Natural Resources at web address: https://fortress.wa.gov/dnr/protectiongis/fpamt/index.html



1/17/2020 3:00 PM 2686.02 DL.dwg Emilio



Photo 1. Inflow point of the unnamed tributary via concrete culvert.



Photo 3. Overview of unnamed tributary's confluence with Rock Cove.



Photo 2. Unnamed tributary flowing toward Rock Cove.



Photo 4. Mud flat adjoining Rock Cove.



DATE: 1/17/20 DWN: ARBA MGR: ARBA PR#: 2682.02

Site Photos

Rock Cove Preliminary Critical Areas Assessment
FDM Development, Inc.

Photoplate 1

City of Stevenson, Washington



Photo 1. Vegetated shoreline on the north end of the study area.



Photo 3. Riprap on the eastern shoreline, facing north.



Photo 2. Vegetated shoreline extending toward the unnamed tributary.



Photo 4. Riprap on the eastern shoreline, facing south.



DATE: 1/17/20 DWN: ARBA MGR: ARBA

PR#: 2682.02

## Photoplate 2 Site Photos

Rock Cove Preliminary Critical Areas Assessment FDM Development, Inc. City of Stevenson, Washington



Photo 1. Graveled boat launch on the east side of the study area.



Photo 3. Vegetated shoreline and mud flat in the southwest portion of the study area, facing south.



Photo 2. Vegetated shoreline on the west side, facing south.



Photo 4. Groomed vegetation in the center of the study area.



DATE: 1/17/20 DWN: ARBA MGR: ARBA PR#: 2682.02

# Photoplate 3 Site Photos

Rock Cove Preliminary Critical Areas Assessment FDM Development, Inc. City of Stevenson, Washington



Photo 1. Existing concrete and gravel surfacing.



Photo 3. Groomed vegetation in the center of the study area.



Photo 2. Existing concrete and gravel surfacing.



Photo 4. Existing gravel road.



DATE: 1/17/20 DWN: ARBA MGR: ARBA PR#: 2682.02 Photoplate 4
Site Photos

Rock Cove Preliminary Critical Areas Assessment FDM Development, Inc. City of Stevenson, Washington

Table 5.1 – Shoreline Use & Setback Standards										
				Shorelin	e Enviror	nment Desi	ignation			
			Most	Restrictive	1	to	Least Rest	rictive		
	AQUATIC		NAT	URAL		SHORELINE RESIDENTIAL		SAN RVANCY	ACTIVE WATERFRONT	
	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)
Р	= Permitte	d, C=Cond	itional Use	e, X= Not Pe	rmitted, n	/a= Not Ap	plicable			
Agriculture & Mining										
Agriculture	Χ	n/a	X	n/a	Χ	n/a	X	n/a	Χ	n/a
Mining	Χ	n/a	X	n/a	Χ	n/a	X	n/a	Χ	n/a
Aquaculture										
Water-Oriented Non-Water Oriented	C X	n/a	Х	n/a	Х	n/a	C X	0 n/a	C C	0 150
Boating Facilities & Overwater S	tructures									
Non-motorized Boat Launch			С		P		Р		P	
Motorized Boat Launch			Х		С		С		Р	
Mooring Buoy		ent	С		С		Р		Р	
Float	ent	muc	Х		C		С		Р	
Private Leisure Deck	djac	virc	Х	n/a	С	n/a	C	n/a	Р	n/a
Public Leisure Pier	See Adjacent	d Fr		.,,,	C	, -	P	,	P	, -
Single-User Residential Dock	ν, ν	Upland Environment	X		C		C		P	
Joint-Use Moorage			X		P		P		Р	
Marina			X		X		C		P	
Commercial & Industrial									·	
Water-Dependent	P				X ¹	0	Р	0	P	0
Water-Related, Water Enjoyment	C	n/a	Х	n/a	$X^1$	75	P .	50	Р	33
Non-Water-Oriented	X	,,,=		.,,	X	-	C ²	150	C ²	100
Forest Practices										
All	Х	n/a	С	50	P	50	Р	50	P	25
Institutional		, -								
Water-Dependent	С		С	0	С	0	Р	0	P	0
Water-Related	X		X	n/a	C	100	Р	75	P	50
Non-Water-Oriented	X	n/a	X	n/a	C	100	C	100	P	100
Cemetery	X		X	n/a	C	50	P	50	C	50
Instream Structures				,						
All	С	n/a	С	0	С	0	С	0	С	0
	-	, <del>-</del>	_	-	-	-		-	_	-

TABLE 5.1 – SHORELINE USE & SETBACK STANDARDS, CONT.										
	Shoreline Environment Designation									
		Most Restrictive to Least Restrictive								
	AQ	UATIC	NAT	URAL		RELINE ENTIAL	URBAN CONSERVANCY		ACTIVE WATERFRONT	
	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)
Р	= Permit	ted, C=Cond	itional Use	, X= Not Pe	ermitted, n	/a= Not Ap	plicable			
Land Division										
All	С	n/a	С	n/a	Р	n/a	Р	n/a	Р	n/a
Recreational										
Water-Dependent	Р		Р	0	Р	0	Р	0	Р	0
Water-Related/Water-Enjoyment	Х		С	100	Р	50	Р	50	Р	50
Trail Parallel to the Shoreline, View Platform	С	n/a	Р	50	Р	50	Р	33	Р	25
Dirt or Gravel Public Access Trail to the Water	Х	1.1/ 0	Р	0	Р	0	Р	0	Р	0
Non-Water-Oriented (golf course, sports field)	Х		Х	n/a	X	n/a	С	150	С	100
Residential	ı									
Single-Family	Х		X		Р	50	С	50	Х	N/A
Multi-Family	Х	n/a	X	n/a	Р	50	Р	50	Р	50
Over-Water Residence	Х		X		Χ	n/a	Х	n/a	Χ	n/a
Transportation & Parking Facilit	ies									
Highway/Arterial Road	С		Х	n/a	С	100	Р	50	Р	50
Access & Collector Road	Х		C	100	Р	100	Р	50	Р	50
Private Road	X		C	100	Р	50	С	50	C	50
Bridge	С	n/a	С	0	С	0	Р	0	Р	0
Railroad	C		С	100	С	100	Р	50	Р	50
Airport	Х		Х	n/a	Χ	n/a	С	150	С	150
Primary Parking Facility	Х		Х	n/a	Χ	n/a	Х	n/a	Χ	n/a
Accessory Parking (On-Site Parking Serving another Use, Including Recreation/Vista Uses)	Х		Р	100	Р	100	P	50	Р	33

TABLE 5.1 – SHORELINE USE & S	SETBACK S	TANDARDS	s, cont.							
		Shoreline Environment Designation								
			Most	Restrictive	t	О	Least Rest	rictive		
	AQU	JATIC	NAT	URAL		ENTIAL	URBAN CONSERVANCY			TIVE RFRONT
	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)	Allowance	Setbacks (ft)
	P= Permitte	ed, C=Cond	litional Use	, X= Not Pe	ermitted, n	/a= Not Ap	plicable			
Utilities										
Water-Oriented	Р	n/a	С	0	С	0	Р	0	Р	0
Non-Water-Oriented (Parallel)	Х	n/a	С	100	С	50	Р	50	Р	33
Non-water-Oriented (Perpendicular)	С	n/a	С	0	С	0	С	0	Р	0

^{1 –} All Industrial uses are prohibited, however, a Water-Oriented Commercial use may be allowed as a conditional use in the Shoreline Residential SED.

^{2 –} Conditionally allowed only when a) the project provides a significant public benefit with respect to SMA objectives (e.g., providing public access and ecological restoration) and i) is part of a mixed-use project that includes water-dependent uses or ii) navigability is severely limited or b) the site is physically separated from the shoreline by another property or public right-of-way.

<u>APPENDIX 6</u> – Operation and Maintenance Manuals

#### STORMWATER FACILITIES OPERATIONS AND MAINTENANCE PLAN

Owner: FDM Development

Project Name: Rock Creek Cove Hospitality

**Engineer:** Harper Houf Peterson Righellis Inc.

1220 Main Street, Suite 150 Vancouver, WA 98660

Date: May 2023

The following Operations and Maintenance (O&M) outlines the necessary requirements for stormwater conveyance and water quality facilities for Rock Creek Cove Hospitality.

This manual is provided in addition to the City of Stevenson's General Requirements.

Water Quality facilities on this project will be owned and maintained by FDM Development.

#### Summary:

The facilities to be maintained under this plan consist of inlets, pipes, manholes, and bioretention facilities. These facilities are shown on the civil improvement plans for the project.

# **General O&M Requirements and Performance Measures**

# O&M Activity

System Component	Remove Sediment, Trash, Debris, and Vegetation	Clean Out/ Control pollution	Manage Vegetation
Structural Storm Sewer Devices  Sedimentation Compartment/ Catch Basin	Sediment Accumulation does Not exceed 1 foot deep or exceed design specifications for sediment storage.	No flammable Chemicals or vapors are present in amounts that would present a fire hazard, exceed pollution control requirements presented in this table, or produce vapors that exceed 10% of the lower explosive limit for that chemical.	
Inlets/Outlets	No trash/debris/ Sediment obstructs more than 25% of the inlet/outlet structure. Flow is not restricted or impounded.		

# **Maintenance and Inspection Procedures**

# Catch Basin and Inlet Inspection/Cleaning

Inspection of catch basins and ditch inlets to be performed no less than annually or in the event of system failure.

	Action	Response/Remark
1.	Check the amount of trash, debris, and other material at the catch basin/ditch inlet.	Make note of the amount of trash and other material at the catch basin. Measure the sediment (in inches) in the catch basin or ditch inlet. Note significant evidence of pollution (oil, grease, foam, odors, etc.)
2.	Remove accessible trash, debris, sediment, etc. from the catch basin/ditch inlet.	Place the debris on a truck so that it can be hauled to disposal.
3.	Inspect the catch basin/ditch inlet, checking that the grate and cover are in place and in good condition.	Check that:  The frame is even with the curb and the top slab is free of holes and cracks.  The frame is sitting flush on the top slab.  The inlet grate is in place and is undamaged.
4.	Check for cracks in the catch basin/ditch inlet structure	Check the basin walls, bottom, and at the joints of the inlet/outlet pipes. Look for dirt entering the catch basin or ditch inlet through cracks.
5.	Check for settling and/or misalignment of the catch basin/ditch inlet.	Check if:  The frame has settled more than 1 inch.  The frame has rotated more than 2 inches out of alignment.
6.	Make notes for machine cleaning, major repair, or replacement of the catch basin/ditch inlet.	Note any particular problems at the catch basin.

# **Drywells/Manholes Inspection/Cleaning**

Inspection of manholes to be performed no less that annually or in the event of system failure.

## Inspection

	Action	Response/Remark
1.	Test the manholes or flow structure for a hazardous atmosphere.	
2.	Inspect the manhole or flow structure frame and cover.	<ul> <li>Check that:</li> <li>The cover is accessible.</li> <li>The manhole cover is in place and in good working condition.</li> <li>All bolts and locks are in place.</li> <li>The cover locks properly.</li> <li>The cover is not difficult to remove.</li> </ul>
3.	Check the amount of sediment in the manhole or structure.	Measure the depth of sediment. Record the depth of sediment. Remove sediment when depths exceeds 1/3 the sump depth.
4.	Check for plugging of the manhole or control structure inlet.	
5.	Make notes for cleaning and repair of the manhole or flow structure.	Note any particular structural problems at the manhole or flow structure. Note visual evidence of pollution or unusual odors. Report all problems immediately for follow-up action.
6.	Report the work completed.	Record:

# Cleaning

	Action	Response/Remark
1.	Test the manhole or flow structure manhole for a hazardous atmosphere.	
2.	Follow vactor manufacturer guidelines to pump water and debris from the manhole or flow structure. Closely monitor the level of accumulation material.	Note any significant signs of pollution, such as oil and grease, foam and unusual odors.
3.	Make notes for repairs to manhole or flow structure.	The manhole cover or flow control frame should be in place and in good condition.
4.	Report the work completed	Record:

#### **Stormwater Bioretention Facilities Operation and Maintenance**

The stormwater bioretention facilities are designed to trap pollutants by filtering and slowing flows, allowing particles to settle out. All facility components, vegetation, and source controls shall be inspected for proper operations and structural stability, at a minimum, quarterly for the first 2 years from the date of installation, 2 times per year thereafter, and within 48 hours after each major storm event. The maintenance staff must keep a log, recording all inspection dates, observations, and maintenance activities. The following items shall be inspected and maintained as stated:

Bioretention Inlet (pipe) shall maintain a calm flow of water entering the planter.

- Source of erosion shall be identified and controlled when native soil is exposed or erosion channels are forming.
- Sediment accumulation shall be hand-removed with minimum damage to vegetation using proper erosion control measures. Sediment shall be removed if it is more than 4" thick or so thick as to damage or kill vegetation.
- Inlet shall be cleared when conveyance capacity is plugged. Sources of sediment and debris shall be identified and corrected.
- Splash blocks shall be inspected and any deficiencies in structure such as cracking, rotting, and failure shall be repaired.

Bioretention Soil Media (BSM) shall allow stormwater to percolate uniformly through the planter/swale. If it does not drain within 48 hours, it shall be tilled and replanted according to design specifications.

- Annual or semi-annual tilling shall be implemented if compaction or clogging continues.
- Debris in quantities that inhibit operation shall be removed routinely (e.g., no less than quarterly), or upon discovery.

Bioretention Outlet shall maintain sheet flow of water exiting the planter/swale unless a catch basin is installed. Source of erosion damage shall be identified and controlled when native soil is exposed or erosion channels are forming.

- Outlets shall be cleared when 50% of the conveyance capacity is plugged.
- · Sources of sediment and debris shall be identified and corrected.

Vegetation shall be healthy and dense enough to provide filtering while protecting underlying soils from erosion.

Mulch shall be replenished as needed to ensure survival of vegetation.

- Vegetation, large shrubs or trees that interfere with planter/swale operation shall be pruned.
- Fallen leaves and debris from deciduous plant foliage shall be removed.
- Nuisance vegetation (such as blackberries and English Ivy) shall be removed when discovered. Invasive vegetation contributing up to 25% of vegetation of all species shall be removed and replaced.
- Dead vegetation and woody material shall be removed to maintain less than 10% of area coverage or when planter/swale function is impaired. Vegetation shall be replaced within 3 months, or immediately if required to maintain cover density and control erosion where soils are exposed.

Spill Prevention measures shall be exercised when handling substances that contaminate stormwater. Releases of pollutants shall be corrected as soon as identified.

Insects & Rodents shall not be harbored in the planter/swale. Pest control measures shall be taken when insects/rodents are found to be present.

• If sprays are considered, then a mosquito larvicide, such as Bacillus thurendensis or Altoside formulations can be applied only if absolutely necessary, and only by a licensed individual or contractor.

## Operation and Maintenance Plan

• Holes in the ground located in and around the planter/swale shall be filled.

Check Dams shall control and distribute flow.

- Causes for altered water flow shall be identified, and obstructions cleared upon discovery.
- Causes for channelization shall be identified and repaired.

# **CULTURAL RESOURCES REPORT COVER SHEET**

DAHP Project Number:  (Please contact the lead agency for the project number. If associated to SEPA, please contact <a href="SEPA@dahp.wa.gov">SEPA@dahp.wa.gov</a> to obtain the project number before creating a new project.)
Author: Donald D. Pattee and Bill R.Roulette
Title of Report: Results of a Cultural Resources Study of the Proposed Rock Creek
Cove Resort Property, Stevenson, Washington
Date of Report: February 4, 2020
County(ies): Skamania Section: 1 Township: 2N Range: 7E
Quad: Bonneville Dam, OR-WA; Carson, WA-OR 2017 Acres: 6.4
PDF of report submitted (REQUIRED) Yes
Historic Property Inventory Forms to be Approved Online?  Yes No
Archaeological Site(s)/Isolate(s) Found or Amended?  Yes No
TCP(s) found?  Yes No
Replace a draft?  Yes No
Satisfy a DAHP Archaeological Excavation Permit requirement?  Yes # No
Were Human Remains Found? Yes DAHP Case # No
DAHP Archaeological Site #:   • Submission of PDFs is required.
<ul> <li>Please be sure that any PDF submitted to DAHP has its cover sheet, figures, graphics, appendices, attachments, correspondence, etc., compiled into one single PDF file.</li> </ul>
• Please check that the PDF displays correctly when opened.

# RESULTS OF A CULTURAL RESOURCES STUDY OF THE PROPOSED ROCK CREEK COVE RESORT PROPERTY, STEVENSON, WASHINGTON



By Donald D. Pattee, M.A., RPA 32246885, and Bill R. Roulette, M.A., RPA 11132,

Report submitted to

FDM Development, Inc. Kennewick, Washington

February 4, 2020

APPLIED ARCHAEOLOGICAL RESEARCH, INC., REPORT NO. 2292



# RESULTS OF A CULTURAL RESOURCES STUDY OF THE PROPOSED ROCK CREEK COVE RESORT PROPERTY, STEVENSON, WASHINGTON

By:

Donald D. Pattee, M.A., RPA 32246885 Bill R. Roulette, M.A., RPA 11132

Report submitted to

FDM Development, Inc. Kennewick, Washington

February 4, 2020

APPLIED ARCHAEOLOGICAL RESEARCH, INC., REPORT NO. 2292

#### **ABSTRACT**

FDM Development, Inc. (FDM) proposes to develop the Rock Creek Cove resort on an industrial property, formerly occupied by the Hegewald Veneer Mill (HVM), located in the western part of the town of Stevenson in Skamania County, Washington. Developments will include the construction of 14 vacation rental homes, a property management building, and paved parking areas around each structure.

The development site is within an urban exempt area of the Columbia River Gorge National Scenic Area. Therefore, the proposed project is not required to follow the guidelines for cultural resource surveys described in the Columbia River Gorge National Scenic Area Management Plan. However, the project is required to comply with the State Environmental Policy Act as implemented by Skamania County Code (16.04). The State Environmental Policy Act requires all developers to consider the impacts a project may have on the environment and to cultural resources before making permitting decisions. FDM contracted with Applied Archaeological Research, Inc. (AAR) to assist it in determining the effects of its proposed project on cultural resources.

AAR's study was designed to locate cultural resources that may be affected by the development and included background research and a field study. The latter included an intensive pedestrian survey and the excavation of four shovel test pits.

As a result, AAR determined that the entire project area had been impacted by the construction and operation of the HVM. Two concrete pads are all that remain of the mill operations. They mark the locations of the main sawmill building and another mill building. In AAR's opinion, the pads are not archaeological and they were not recorded as an archaeological resource.

In terms of Line 13 of the State Environmental Policy Act checklist, it is AAR finding that the project area does not contain any buildings, structures, or sites, that are listed in or eligible for listing in national, state, or local preservation registers. AAR recommends no further archaeological work is warranted in the current project area.

Although considered unlikely, there is always a possibility that an archaeological resource may be discovered during future development activity on the property. For that reason, the applicant and any contractors that may work on the property need to be aware that under the Revised Code of Washington at 27.53.060, it is unlawful to knowingly damage, deface, or destroy an archaeological site on public or private land in Washington. The Revised Code of Washington at 27.44.040 makes it a class C felony to knowingly remove, mutilate, deface, injure, or destroy any cairn or grave of any native Indian. Thus, in the event that archaeological materials, Indian cairns, or human remains are encountered during the development of the property, all construction activities must stop in the vicinity of the finds and the Department of Archaeology and Historic Preservation should immediately be notified and work halted in the vicinity of the finds until they can be inspected and assessed. Procedures outlined under Washington Administrative Code 25-48 will be followed and work will not resume until mitigation measures have been agreed upon.

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### **INTRODUCTION**

# **Project Description and Staffing**

FDM Development, Inc. (FDM) proposes to develop the former site of the Hegewald Veneer Mill (HVM) located at Rock Creek Cove resort into a resort that would include 14 vacation rental homes, a property management building, associated infrastructure, and paved parking areas. The development site is within an urban exempt area of the Columbia River Gorge National Scenic Area (CRGNSA). Therefore, the proposed project is not required to follow the guidelines for cultural resource surveys described in the CRGNSA Management Plan. However, the project is required to comply with the State Environmental Policy Act (SEPA) as implemented by Skamania County Code (16.04). SEPA requires all developers to consider the impacts a project may have on the environment and to cultural resources before making permitting decisions. To assist FDM in its compliance with SEPA requirements, Applied Archaeological Research, Inc. (AAR) conducted a cultural resource survey of the proposed development site.

Archaeological fieldwork for the project was supervised by Donald D. Pattee, M.A., RPA 32246885 who was assisted by Michelle R. Lynch, M.A., RPA 429967347. The project was under the technical supervision of Bill R. Roulette, M.A., RPA 11132, AAR's Principle Investigator. Mr. Pattee, Ms. Lynch, and Mr. Roulette meet the Secretary of the Interior's professional qualification standards.

#### **Conventions**

In this report, measurements for common distances, elevations, and areas are in United States customary units (e.g., feet, miles, and acres). Measurements related to archaeological techniques and artifact analyses are in metric units (e.g., meters, centimeters, and millimeters). Numbers in the thousands used to express ages and distances feature commas to denote thousands. Calendar dates and dates used to express years before present (B.P.) do not use commas to denote the thousands place but do use commas to denote the ten thousands place.

### **Description of the Project Area**

The proposed resort development site is in the western part of the town of Stevenson in Skamania County, Washington, in Section 1, Township 2 North, Range 7 East, Willamette Meridian (Figure 1). It is privately owned and encompasses 6.4 acres. It is composed of three contiguous tax parcels numbered 02070100130300, 02070100130400, and 02070100130200, that together form an irregularly-shaped tract that is maximally 1,022 feet (ft) measured north-to-south and 580 ft measured east-to-west. The property is located on a peninsula that projects into Rock Creek Cove on the northern bank of the Columbia River. The cove was created in 1937 as a result of flooding that occurred along the banks of the river east of Cascade Locks soon after the Bonneville Dam began operation. Its west side is bordered by Rock Creek Drive. Its other sides are defined by the boundaries of the proposed development footprint and the cove (Figure 2).

The project area is at an elevation of about 102 ft above mean sea level (amsl). Its surface has been artificially flattened and built up. The modifications are most likely related to the development of the property by the HVM in the early 1950s (see below). Its central part contains two concrete pads that mark the former locations of mill buildings. The largest pad is 337 ft long and 86 ft wide. It marks the former location of the main sawmill (Figure 3). The other pad is 59 ft long and 45 ft wide and most likely marks the location of a second mill building, possibly a machine shop.

Prior to AAR's fieldwork parts of the property had been disturbed by heavy equipment that was used to clear brush and remove trees. Cleared vegetation and soil were pushed into low piles that

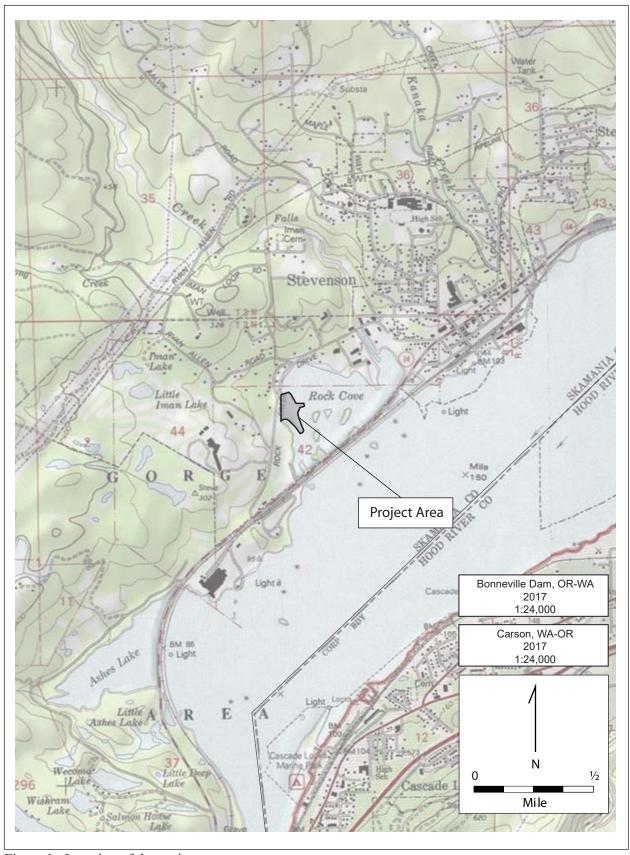


Figure 1. Location of the project area.



Figure 2. Aerial photomap of the project area.



Figure 3. Photographic overview looking east at the concrete foundation of the main sawmill building of the HVM.

remain in place (Figure 4). At least two trenches had been excavated in the eastern part of the property and partly backfilled (Figure 5). The ground surface in the parts of the property that were not disturbed or otherwise obscured by gravel or building foundations were covered in grasses, blackberry brambles, and a scattering of Douglas-fir, alder, and maple trees (Figure 6).

#### **Project Background**

In 2016, Skamania County initiated an inventory of all brownfield sites (i.e. abandoned properties where there may be environmental contamination) located in the county to better understand their impacts on surrounding communities and to study their potential for commercial development. As part of the inventory, the county conducted a Phase II environmental site assessment (ESA) of the project area to evaluate the potential environmental impacts associated with the historical operation of the HVM. No cultural resource investigations were conducted on the property in advance of or as part of the assessment. The ESA included the use of ground penetrating radar across the site to check for buried infrastructure (e.g. tanks, tank pits, pipes, or septic systems). In addition, ten test pits were excavated in select areas to extract soil samples to be analyzed for metals, petroleum, and dioxins. The GPR results showed that there were no buried infrastructure and no petroleum was detected in the soil. Some metals and dioxins were detected, but did not exceed contamination levels considered by the Model Toxics Control Act to be harmful to humans. The ESA recommended that no further environmental remediation of the site was warranted.



Figure 4. Photographic overview looking north of an area cleared of brush. The vegetation and displaced soil have been pushed into low piles.



Figure 5. Photographic overview looking northeast of an area that had been trenched prior to fieldwork.



Figure 6. Photographic overview looking west showing typical vegetation throughout the project area at the time of fieldwork.

# ENVIRONMENTAL, CULTURAL, AND HISTORICAL CONTEXTS

#### **Environmental Setting**

The project area is located in the southernmost part of the Southern Washington Cascade physiographic province where the mountains have been incised by the Columbia River Gorge. The province is characterized by deeply dissected and weathered mountains set on a generally western sloping terrace. It contains rugged mountainous areas, river floodplains, and low terraces.

The modern topography of the Gorge reflects the down cutting of the Columbia River through basalt bedrock. The basalt was laid down during the Miocene in a number of individual flows that collectively are known as the Columbia River Basalts. The lava from these flows originated in central and eastern Washington and Oregon and streamed westward down the Columbia River valley to the sea (Allen et al. 1986). Exposures of these flows can be seen in the steep walls framing the Gorge.

Following the deposition of the basalts, the Cascades were up-arched. As the mountains were rising, the Columbia River was cutting down through the range, creating its deep canyon. Later, toward the end of the Pliocene and into the Pleistocene, volcanic activity resumed in the Cascades, producing lava flows which filled the tributaries of the Columbia and which displaced the river to the north, near its present position. The strato volcano peaks of Mt. Hood, Mt. St. Helens, and Mt. Adams began to rise some 700,000 years ago, a process which continues into the present. The up-arching of the Cascades created a barrier to easterly flowing moist marine air and resulted in the climatic division of the region into the moist western and dry eastern portions (Allen et al. 1986). In the Columbia River Gorge, this climatic change occurs around White Salmon and Hood River, a short distance upriver, or east, of the project area.

Although the basalt flows of the Miocene laid the foundation for the physiography of the Gorge, the geological events of the Pleistocene shaped it into its present configuration. The most important of these events were the Missoula Floods (known variously as the Bretz or Spokane floods) that occurred between about 17,000 and 12,700 years ago (Clague et al. 2003; Waitt 1994). The floodwaters originated in Glacial Lake Missoula, a body of water formed when the Purcell Trench Lobe of the Cordilleran ice sheet blocked the Clark Fork River in Montana. When the waters of Lake Missoula breached the ice dam, a wall of water estimated to have been ca. 2,000 ft high was released. In a single flood, somewhere near 500 cubic miles of water rushed across the Columbia Plateau and entered the Columbia River system (Alt and Hyndman 1993:172). The tremendous force and volume of the floods scoured away the soils of the Gorge and altered the river valley from its previous V shape to its present U-shaped cross-sectional profile (Allen et al. 1986:159).

The floods led to the oversteepening of the Gorge walls, particularly in areas where the Columbia River basalts are underlain by the easily erodible Eagle Creek Formation. These conditions have made a nearly 50-square-mile area toward the west end of the Gorge prone to landslides. The project area is situated near the leading edge of a debris deposit from the quaternary-aged Red Bluff landslide, which is part of the greater Cascade Landslide Complex. The deposits extend further southward and are submerged in Rock Creek Cove (Pierson et al. 2016; Randall 2012).

The project area is in the *Tsuga heterophylla* zone, a classification of plant associations that is found throughout western Washington and Oregon in wet maritime climates between sea level and about 2,300 ft amsl (Franklin and Dyrness 1988). Throughout the zone, Douglas-fir, western hemlock, and western redcedar with few hardwoods dominate typical overstory vegetation in forested areas. Common forest understory plants throughout the zone include vine maple, hawthorn, wild rose, blackberry, thimbleberry, and snowberry.

The primary soil mapped within the project area is Arents, 0 to 5 percent slopes (Haggen 1990). It is an anthropogenic soil that developed as the result of disturbance and redeposition through various human activities such as mining, dredging of water bodies, road building, and construction (Sencindiver and Ammons 2000). It does not represent a native soil body, but rather formed in spoils that have been removed from their original context and redeposited. No single profile of Arents is typical. One commonly observed includes a 24-inch-thick "A horizon" of dark brown, gravelly sandy loam. The underlying material extends to a depth of 5 ft below surface and consists of stratified gravelly to very gravelly loamy sand (Haagen 1990).

# **Ethnographic Overview**

The project area is located at the eastern periphery of the traditional territory of the Cascade people that spoke an Upper Chinook dialect and were closely aligned with other Upper Chinook peoples that occupied both sides of the Columbia River between from roughly the mouth of the Washougal River to a point above Dallesport including the Hood River, White Salmon, Wasco, and Wishram (French and French 1998:360-363). The territory of the Cascades Chinook included lands on each side of the Columbia River in the vicinity of the Cascades of the Columbia, a section of river narrowed and obstructed with landslide debris where the river dropped about 40 ft in elevation through a series of rapids over a distance of several miles. The Cascades controlled the portages around the rapids and the important salmon fishery centered there.

The Cascade people and other Upper Chinookan groups lived in autonomous villages without overarching political organization or centralized government (French and French 1998:369). Villages were presided over by chiefs who held office based primarily on a system of hereditary leadership rights (Silverstein 1990:541). Chiefs were usually persons of the highest rank within the hierarchically organized Chinook society, and chiefly status was conferred on members of wealthy and politically

influential families. Status, class, and rank were used as organizational principles in Chinook society. Chiefs, along with shamans, warriors, and traders, formed a small upper class with slaves forming the bottom of the social hierarchy. Commoners ranged between these hierarchical poles and were probably ranked along numerous socially recognized gradations. High rank and high class was strongly linked to wealth.

Winters were spent in permanent settlements consisting of one or more rectangular, gabled-roofed, upright-cedar-plank houses (Hajda 1994; Silverstein 1990) that featured raised sleeping and storage platforms that lined the house walls. In 1805, Lewis and Clark encountered the Chinook village of Wishram on the north side of the Columbia River (near what is now Columbia Hills State Park) and described some 20 homes constructed of wood, the first wooden houses the expedition had seen since leaving Illinois (Wilke et al. 1983:75-76). Chinook subsistence was oriented toward fishing and root-and-berry gathering. Most subsistence activities were organized around small groups that dispersed to smaller camps focused on task-specific subsistence activities.

Native peoples that lived along the Columbia River came into contact with European and American sea-borne fur traders in the late-eighteenth century. Diseases introduced by the traders, especially small pox, influenza, and malaria, spread rapidly upriver and throughout the region with catastrophic results. The first historical reports of a malarial epidemic are from 1830. Within four years 75 to 90 percent of the regional native population was dead (Boyd 1985). Displaced groups and individuals formed *ad hoc* communities or joined those still existing, and either attempted to follow traditional patterns or adopted the life ways of the Euroamericans (Hajda and Boyd 1988:45-46).

#### **Historical Overview**

The first Euroamericans to pass through the Columbia River Gorge were explorers and fur traders in the early decades of the nineteenth century. Among the explorers were Lewis and Clark who led their Corps of Discovery expedition down the Columbia River in 1805, and David Thompson, who traversed the length of the Columbia River in 1811. After the establishment of a land-based fur trade around 1811, a greater number of Euroamericans traveled throughout the region in search of furs. Travel logs left by early traders in the region document the spread of disease among the native populations of the Columbia River as early as the 1830s, resulting in a catastrophic population loss (Minor et al. 1986:54-55). By 1834, missionaries began trickling into the region, followed several years later by the initial waves of pioneers heading to the Willamette Valley along the Oregon Trail. Between 1841 and 1851 all travelers and settlers heading west had to pass through the Columbia River Gorge, where, just east of the city of Stevenson, they were forced to portage along the north bank of the river around the rapids known as the Upper, Middle, and Lower Cascades.

The passing of the Oregon Donation Land Act of 1850 resulted in a steady influx of Euroamerican settlers that initially used the area for grazing livestock and logging (Mack and McClure 1999). As more settlers arrived to the region, small communities were established along the banks of the Columbia River, which provided needed services for travelers passing through the gorge. These included lodging, supplies, and improved portage routes. One such community was Stevenson, which shared the name of its founder, George Stevenson. The town was founded in 1893 and quickly became an important way-stop for travelers passing through the gorge. River transportation improved with the construction of the Cascade Locks in 1896 allowing boats to by-pass the cascades. Incoming travelers to the region could now navigate the Columbia River from Portland as far as The Dalles. Easier river travel spurred economic development in Stevenson and by 1900 the town featured two hotels, two saloons, two restaurants, as well as a general store, drug store, post-office, jail, print shop, and court house (Skamania County Chamber of Commerce 2020; Wilma 2006). The town was officially incorporated in 1908. That same year, the Spokane, Portland, and Seattle rail line arrived and connected the town to the major cities of the Pacific Northwest (Wilma 2006).

The rail line and the more navigable river resulted in logging and milling becoming one of the more important economic pursuits in the region as timber products could be transported with relative ease to Portland or Seattle and then shipped overseas where demand was high. In the following decades, the logging industry became vital to the economy of Stevenson. Trees logged in the hills backing the town were transported by flumes down to sawmills that lined the shoreline including the HVM.

The HVM operated between 1952 and 1973. It was primarily used for the production of wood veneer, which was peeled from tree logs and then pressed into 8-foot-long sheets (Hunt 1964). The sheets were used to line doors, table tops, and cabinetry panels. At the height of its operation, the mill produced 60,000,000 square feet of veneer annually (Hunt 1964). Waste produced from the process (e.g. wood chips or parts of the log not suitable for milling) was burned in two conical structures referred to at the time as "wigwam burners" (Hunt 1964). Tree logs were stored in Rock Creek Cove, which was enclosed by wooden booms that prevented the logs from floating downriver. In 1973, the mill was sold to Louisiana Pacific, which operated it until its closure in 1975. Around that same time, other sawmills in the Stevenson area closed resulting in the loss of hundreds of jobs and severely impacting the economy of the town. It did not fully recover until the early 1990s (Wilma 2006).

# **Historical Maps Research**

As part of the background research, historical maps were reviewed to determine the likelihood that the project area contains undocumented historic-era features and to trace land ownership. Maps reviewed include those produced by the General Land Office (GLO) as part of the cadastral survey and those prepared by the United States Geologic Survey (USGS). Historic aerial photographs were also reviewed.

The earliest maps that depict the project area are cadastral survey maps produced by the General Land Office (GLO) in 1860, 1876, 1903, and 1906. The project area is shown as devoid of developments on the maps (GLO 1860, 1876, 1903, 1906). An 1864 GLO map shows lands taken out of federal ownership through land claims. The project area is shown as within a 319.91-acre land claim filed by D. Baughman (GLO 1864).

A 30-minute (1:125,000) map published by the United States Geological Survey (USGS) in 1929 shows the project area before inundation of the Bonneville Pool (also known as Bonneville Lake) the reservoir behind Bonneville Dam (USGS 1929). No buildings or other developments are depicted in it (Figure 7). A 15-minute map published by the USGS in 1957 shows the project area after completion of the Bonneville Dam and formation of the reservoir behind it (USGS 1957). A large rectangular structure is shown on the map to be in the project area representing the main HVM sawmill building (Figure 8).

An aerial photograph taken of the mill sometime between 1952 and 1973 on display in the Columbia Gorge Interpretive Center Museum, shows that HVM in full development (Figure 9). The mill complex can be seen to cover the entire project area with much of it covered by buildings, what appear to be graveled surfaces, stockpiled wood products, and general debris. The photograph shows the main sawmill and the second mill building in locations corresponding to where concrete pads remain. It also shows two wigwam burners that were located in the southern part of the property (Western Ways, Inc., n.d.).

# Previous Archaeology in the Project Area and Vicinity

A review of records on file at the Washington State Department of Archaeology and Historic Preservation (DAHP) accessed online using its Washington Information System for Architectural and Archaeological Records Data (WISAARD) database showed that the project area has not previously been surveyed for cultural resources. Thirty-three cultural resource investigations have been conducted within

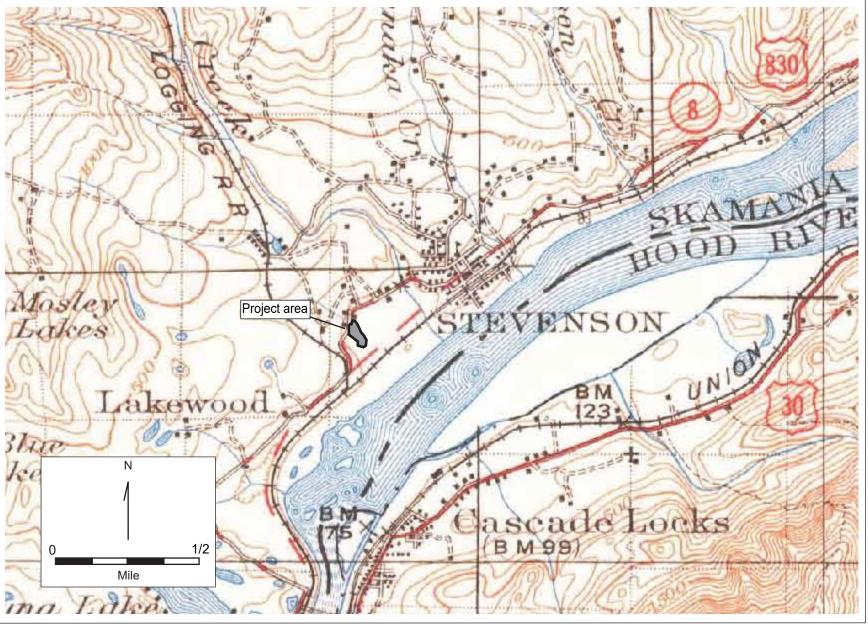


Figure 7. Location of the project area as depicted on the Hood River, Wash.-Oreg., 30-minute topographic quadrangle published in 1929.

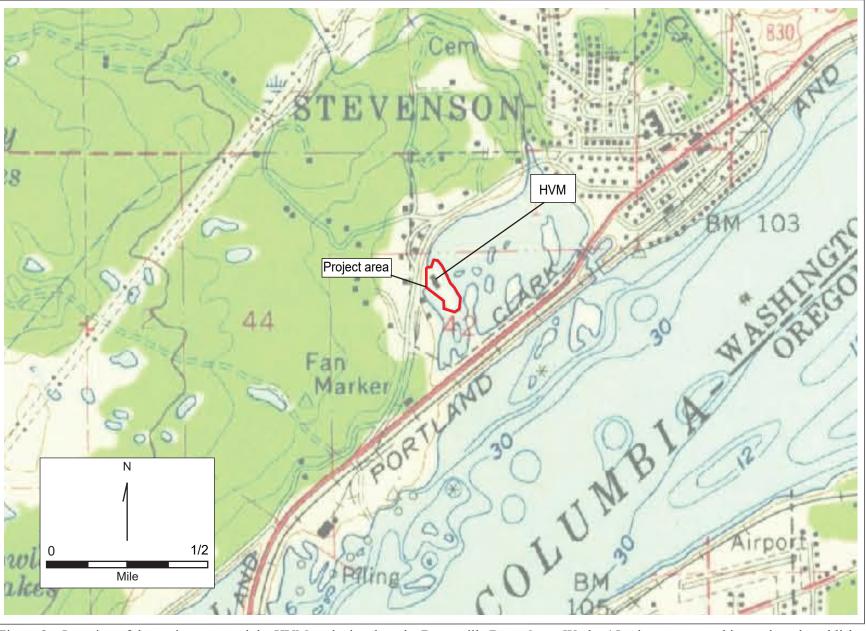


Figure 8. Location of the project area and the HVM as depicted on the Bonneville Dam, Oreg.-Wash., 15-minute topographic quadrangle published in 1957.



Figure 9. Aerial photomap taken of the HVM sometime between 1952 and 1973. Photomap is currently on display in the Columbia Gorge Interpretive Center Museum.

two miles of it (Table 1). The studies have generally consisted of reconnaissance and formal surveys that have resulted in the identification of multiple component sites 45SA20 and 45SA541, pre-contact sites 45SA210, 45SA600, 45SA633, 45SA650, pre-contact isolate 45SA585, and historic-era sites 45SA8, 45SA121, 45SA501, and 45SA502.

Of the previously recorded sites, 45SA20, the Ice House Lake site, has been the most intensively studied. The site was recorded during a cultural resources survey conducted by the University of Washington in advance of the construction of a powerhouse at Bonneville Dam (Mesrobian and Sunstrom 1976). It is located about 1.4 miles to the southwest of the project area on terraces overlooking the northern shore of the Columbia River. Evaluative test excavations were conducted at the site in 1988. They included a surface inspection as well as the excavation of six 1-x-1 meter (m) test units (TUs) and six auger test probes. The investigation resulted in the recovery of a variety of pre-contact and historicera artifacts as well as floral and faunal remains.

Pre-contact artifacts recovered from the site included 11,243 pieces of cryptocrystalline silicate (CCS), obsidian, basalt, and petrified wood debitage and 99 stone tools. Tools included projectile points, preforms, knife fragments, bifaces, flake knives, perforators, used flakes, hammerstones, pounders, anvils, choppers, cobble flake knifes, spall tools, abraders, and cores (Minor 1988). Most of the projectile points identified were small, narrow necked forms consistent with Types 7, 8, 10, and 12 described in Pettigrew's (1981) projectile point chronology of the Portland Basin. Broad-necked projectile points of the Type 2 variety were also observed (Pettigrew 1981).

The 439 historic-era artifacts recovered during the investigations included fragments of earthenware, porcelain, stoneware, and Chinese ware, clay pipes, vessel glass, machine cut nails, spikes, brace plates, iron bolts, staples, wire, bullets, metal scraps, and gunflint. A few pieces of charred nut shell and 148 animal bones were also recovered. Most of the bones were small fragments. Most were from sturgeon but they also included horse, elk, deer, cow, salmonids, and cyprinid bones (Minor 1988).

Minor (1988) determined that the site represented the village *Wahlala* (Curtis 1911) or *Walala* (Spier and Sapir 1930) occupied by the Cascade Chinook. It is described in the journal of Lewis and Clark as consisting of eight plank slab houses that were inhabited part of the year during the fishing season. Based on the results of the investigation, the site was interpreted to have been continually used by Chinook as a seasonal fishing village during the pre-contact period and into historic times. Initial occupation of the site was thought to have occurred 830 years ago. The site was likely abandoned around 1850 when the United States established a strong military presence throughout the Columbia River Gorge (see below). The site was recommended as eligible for listing on the National Register of Historic Places (NRHP).

The other multicomponent site within two miles of the project area is 45SA541. The site was recorded based on the inadvertent discovery of human remains in the side wall of a utility trench during the installation of buried telecommunications equipment. The discovery triggered emergency archaeological excavations and the screening of a sample of the spoils created during the trenching. Recovered were 86 human or potentially human bones and mixed historical; and prehistoric artifacts all of which were contained in a thick layer of imported fill (Paraso and Ellis 2010).

Of the previously recorded pre-contact resources, three of them (45SA210, 45SA585, and 45SA650) consist of low density, lithic scatters that have not been documented past the initial survey phase. Site 45SA210 was identified 1.5 miles to the southwest of the project area on the north shore of Ashes Lake. As documented, the site contains one desert side-notched projectile point, a piece of human bone, and pieces of lithic debitage (Cole and Southard 1971). Only lithic debitage was identified at the other resources with site 45SA585 containing 10 pieces of CCS and basalt debitage and isolated find 45SA650 containing a single piece of CCS debitage (Becker and Roulette 2017; Olander et al. 2011).

Table 1. Cultural Resource Surveys Conducted within 2 Miles of the Project Area

Table 1. Cultural Resource Surveys Conducted within 2 Miles of t		the Project Area		
Author(s) of Report/Year	Type of Investigation	Size of Study Area	Findings	
Cole and Southard 1971	Formal survey	Not listed	45SA210 identified and documented	
Dunnell and Lewarch 1974	Formal survey	Not listed	45SA8 identified and documented	
Mesrobian and Sundstrom 1976	Formal survey	Not listed	45SA20 identified and recorded	
Minor 1988	Evaluative testing	Not listed	Additional study at 45SA20 that refined its boundaries and expanded its artifact assemblage.	
Minor and Beckham 1988	Evaluative testing	Not listed	45SA121 identified and documented	
Freed 1989	Damage Assessment	Not listed	Additional study at 45SA20 that expanded its artifact assemblage.	
Boynton 1995	Formal survey	82 acres	Archaeological resources identified and documented at distances greater than 2 miles from the project area	
Musil 1999	Formal survey	120 acres	No archaeological resources identified	
Easton and Roulette 2002	Formal survey	Not listed	No archaeological resources identified	
Stilson 2002	Formal survey	4.4 acres	Archaeological resources identified and documented at distances greater than 2 miles from the project area	
Scott 2003	Cultural resource monitoring	47 mile linear cooridor	Archaeological resources identified and documented at distances greater than 2 miles from the project area	
White and Ozbun 2003	Reconnaissance survey	Not listed	No archaeological resources identified	
Boynton and Fagan 2006	Formal survey	4.2 acres	45SA501 and 45SA502 identified and documented	
Gall 2006	Formal survey	25.4 acres	No archaeological resources identified	
Dryden 2007	Reconnaissance survey	0.90 acre	No archaeological resources identified	
Dryden 2009	Reconnaissance survey	0.01 acre	No archaeological resources identified	
Lloyd-Jones and Ozbun 2009	Formal survey	5 acres	No archaeological resources identified	
Dryden 2010a	Reconnaissance survey/cultural resource monitoring	2 acres	No archaeological resources identified	
Dryden 2010b	Reconnaissance survey	0.15 acre	No archaeological resources identified	
Paraso and Ellis 2010	Emergency archaeological excavations	Not listed	45SA541 identified and documented	
Olander et al. 2011	Formal survey	Not listed	45SA585 identified and documented	
Kiers 2012	Formal survey	<0.1 acre	No archaeological resources identified	
Knutson et al. 2012	Formal survey	8.6 acres	45SA600 identified and documented.  Numerous other resources identified at distances greater than 2 miles from the project area.	
Harris et al. 2013	Formal survey	3.5 acres	No archaeological resources identified	
O'Donnchadha 2013	Formal survey	1 acre	No archaeological resources identified	
Bard et al. 2014	Formal survey	123.5 acres	Archaeological resources identified and documented at distances greater than 2 miles from the project area	
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Table 1. Cultural Resource Surveys Conducted within 2 Miles of the Project Area, continued

Author(s) of Report/Year	Type of Investigation	Size of Study Area	Findings	
Jenkins and Reese 2014	Formal survey	2.6 acres	No archaeological resources identified	
Pattee and Roulette 2014	Formal survey	8.26 acres	No archaeological resources identified	
Smith and Gall 2014	Formal survey	30 acres	Additional study at 45SA600 that refined its boundaries. 45SA633 identified and documented.	
Holschuh 2015	Formal survey	1 acre	No archaeological resources identified	
Becker and Roulette 2017	Formal survey	1 acre	45SA650 identified and documented	
Homan and O'Donnchadha 2017	Formal survey	52.51 acres	No archaeological resources identified	
Gall and Smith 2019	Formal survey	41.5 acres	Additional study at 45SA8 that refined its boundaries and expanded its artifact assemblage. Archaeological resources identified and documented at distances greater than 2 miles of the project area.	

Pre-contact sites 45SA600 and 45SA633 were observed to contain shallow pit features that had been excavated into a talus slope. The sites are located about two miles to the southwest of the project area. The date, origin, and function of the pits could not be determined. They are similar to those identified on the summit of Wind Mountain located approximately seven miles to the northeast of the project area, which are considered sacred to past and contemporary Native American groups. Because of this, the features were recorded as archaeological sites (Knutson et al. 2012; Smith and Gall 2014).

Historic-era site 45SA121 is located about 1.2 miles to the southwest and consists of the remnants of the U.S. Army's Fort Lugenbeel and the civilian town site of Upper Cascades. The town was established in 1851 and became one of the first frontier communities in the Columbia River Gorge. It contained hotels, homes, storage buildings, a portage tramway, and a sawmill. By 1855 the U.S. Army had established Fort Cascades at the Lower Cascades and Fort Rains at the Middle Cascades to the west to ensure the safe passage of troops and supplies from Fort Vancouver. Both forts were attacked and destroyed by Native Americans in 1856. Following the attack, the U.S. Army regained control of the area and constructed Fort Lugenbeel on a ridge above the community at Upper Cascades to deter future attacks (Minor and Beckham 1988). Evaluative testing at the site in 1988 resulted in the identification of multiple building foundations associated with the fort and town site as well as the recovery of 4,630 artifacts. These included ceramic and glass fragments, nails, spikes, bricks, various items related to firearms, clay pipe fragments, buttons, and faunal remains (Minor and Beckham 1988). The fort and town site were used between 1850 and 1880. The site has been listed on the NRHP under Criterion D.

Historic-era site 45SA8 was initially identified in 1974 as an historical homestead based on anecdotal information (Dunnell and Lewarch 1974). At the time of its recording, the location of the site was not field verified. In 2019, the site was the subject of a formal cultural resources survey that resulted in the discovery of a sparse, subsurface historic-era debris scatter. Observed artifacts included amber, aqua, amethyst, and colorless vessel glass, cut nails, several bottle bases, fragments of whiteware ceramics, and metal fragments (Gall and Smith 2019). Based on the identification of temporally sensitive artifacts during the investigation, the site deposit was determined to have formed between 1880 and 1920 (Gall and Smith 2019).

Historic-era sites 45SA501 and 45SA502 are located approximately 1 mile to the northeast of the project area. They were identified during a cultural resources survey conducted in advance of the construction of a residential subdivision. Site 45SA501 consists of a small dump of household debris, which includes oval Postum tins, a Hazel-Atlas bottle base, zinc caps, rusted cans, canning jars, and

fragments of machine molded glass. The dump has been interpreted to have formed in the early 20th century (Boynton and Fagan 2006). Site 45SA502 consists of the ruins of an historic-period residential structure that was constructed in 1895 (Boynton and Fagan 2006).

Two historic-era cemeteries, which were recorded as cultural resources, are located within two miles of the project area. They are sites 45SA555, the Iman Cemetery, and 45SA651, the Gropper Cemetery. The first is located on land that was owned by Feliz Grundy Iman and was established in 1889 (Anonymous n.d.a). The second is located on the northern end of Stevenson and was established in 1905 (Anonymous n.d.b).

#### METHODS AND RESULTS

#### **Fieldwork Methods**

Fieldwork was conducted on January 8 and 15, 2020. The approach to the fieldwork was informed by the results of the background research that showed that the entire development site had been significantly impacted by past development that appears to have included grading and leveling the ground surface. Subsequent to that soil and gravel were dumped across the landform and compacted. With that history of land use in mind, the potential for buried archaeological deposits to be present was assessed as very low. Consequently, the fieldwork consisted of an intensive surface survey and the excavation of four shovel-test-pits (STPs) to verify the suspected level of disturbance and to examine the character of subsurface conditions (Figure 10).

The STPs were 30 centimeters (cm) in diameter and were excavated in 20-cm or thinner levels to depths that ranged between 20 and 50 cm below surface (cmbs). All sediments removed from the probes were screened through one-eighth-inch-mesh hardware cloth. Afterward, the STPs were completely backfilled and their locations were recorded using a handheld Trimble Geo7X global positioning system (GPS) device. GPS data were then corrected and exported to a graphics program for final editing and formatting.

### **Results of the Field Investigations**

The ground surface was inspected by walking transects spaced no more than 10 m apart. Ground surface visibility was variable. In the parts of the property that were obscured by building foundations, gravel, or trampled blackberry brambles, surface visibility was zero percent. Areas that had been trenched and then backfilled prior to fieldwork had 100 percent visibility. Other areas of the property were covered in a thin layer of grass and duff. Surface visibility in these areas was about 25 percent. No artifacts were found on the ground surface. The two concrete pads, mentioned above, were observed. They appear to be all that remains of the HVM. All other mill facilities have been completely removed. The slabs are overgrown and covered with a thin layer of moss and grass.

No artifacts were found in the STPs. Soil profiles encountered during the excavations consisted entirely of fill material, which matched the description of Arents, 0 to 5 percent slopes mapped on the property. Profiles generally included a 5- to 20-cm-thick organic layer of very dark brown (7.5YR 2/2) sandy loam, which capped a 10- to 45-cm-thick layer of brown (10YR 4/3), sandy loam (Figure 11). At least three quarters of the soil matrix in the latter layer contained angular gravel intermixed with small to medium angular cobbles (Figure 12). STP 3 and 4 terminated at 20 cmbs due to an impenetrable layer of angular cobbles (Table 2).

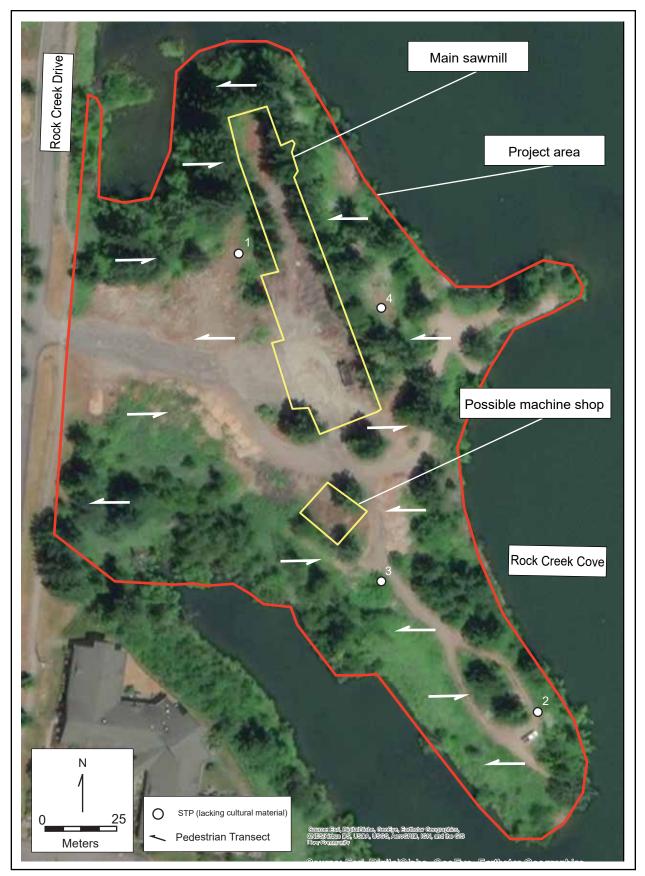


Figure 10. Aerial photomap of the project area showing the locations of the concrete slabs representing mill structures, STPs, and pedestrian transects walked.



Figure 11. Representative view of the gravelly fill encountered in the STPs.



Figure 12. Representative view showing the amount of rock found in the STPs.

Table 2. Summary Results of STPs Excavated

STP#	Depth (cmbs)	Sediments (Moist)	Results	
1	0-5	Organic layer of very dark brown (10YR2/2) sandy loam	No artifacts	
'	45-50	Brown (10YR4/3) sandy loam. Numerous angular gravels and cobbles.	No artifacts	
2	0-20	Organic layer of very dark brown (10YR2/2,) sandy loam	No artifacts	
2	20-50	Brown (10YR4/3) sandy loam. Numerous angular gravels and cobbles.	- No artifacts	
3 5-20	0-5	Organic layer of very dark brown (10YR2/2) sandy loam		
	5-20	Brown (10YR4/3) sandy loam. Numerous angular gravels and cobbles. Terminated at	No artifacts	
	3-20	impenetrable layer of angular cobbles.		
4	0-5	Organic layer of very dark brown (10YR2/2) sandy loam		
	5-20	Brown (10YR4/3) sandy loam. Numerous angular gravels and cobbles. Terminated at	No artifacts	
	5-20	impenetrable layer of angular cobbles.		

#### SUMMARY AND RECOMMENDATIONS

### **Summary**

This report has described the results of a cultural resources study conducted by AAR of a 6.4-acre property that FDM proposes to develop into the Rock Creek Cove resort. The study included background research and field investigations. The results of the background research indicate that the property has been significantly altered such that it has low potential to contain archaeological resources. AAR's fieldwork included an intensive surface survey and excavation of four STPs. No artifacts were found. Profiles exposed in the probes showed that a thick layer of imported gravelly fill covers the entire development site.

The only trace of the HVM consists of two concrete pads that mark the location of two of the mill buildings. In AAR's view, the pads are not archaeological and they were not were not recorded as an archaeological resource.

#### Recommendations

AAR's study was done to assist FDM in complying with SEPA as implemented by Skamania County Code (16.04). In terms of Line 13 of the SEPA checklist, it is AAR finding that the project area does not contain any buildings, structures, or sites, that are listed in or eligible for listing in national, state, or local preservation registers. AAR recommends no further archaeological work is warranted in the current project area.

Although considered unlikely, there is always a possibility that an archaeological resource may be discovered during future development activity on the property. For that reason, the applicant and any contractors that may work on the property need to be aware that under the Revised Code of Washington at 27.53.060, it is unlawful to knowingly damage, deface, or destroy an archaeological site on public or private land in Washington. Under the Revised Code of Washington at 27.44.040 it a class C felony to knowingly remove, mutilate, deface, injure, or destroy any cairn or grave of any native Indian. Thus, in the event that archaeological materials, Indian cairns, or human remains are encountered during the development of the property, all construction activities must stop in the vicinity of the finds and the DAHP should immediately be notified and work halted in the vicinity of the finds until they can be inspected and assessed. Procedures outlined under Washington Administrative Code at 25-48 will be followed and work will not resume until mitigation measures have been agreed upon.

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# CITY OF STEVENSON SMC 18 SHORELINE MANAGEMENT

Regarding a request by FDM Development Inc. to construct 19 )
cabins that will serve as nightly and weekly lodging, as well as an )
Event space to be used for private weddings, reunions, and parties. )

On-site parking, public pedestrian access, landscaping, and )
enhancements to the riverbank will also be provided. Additionally, )
a plat vacation is proposed to provide a more cohesive property )
under one ownership group. )

**PROPOSAL:** The applicant requests a Shoreline Substantial Development Permit (SSDP) to construct 19

cabins that will serve as nightly and weekly lodging, as well as an event space to be used for private weddings, reunions, and parties. On-site parking, public pedestrian access, landscaping, and enhancements to the riverbank will also be provided. Additionally, a plat vacation is proposed to provide a more cohesive property under one ownership group.

**APPLICANT:** Brad Kilby, AICP **ENGINEER:** Bruce Haunreiter, P.E.

Harper Houf Peterson Righellis Inc.

Harper Houf Peterson Righellis Inc.

205 SE Spokane Street, Suite 200

1220 Main Street, Suite 150

Portland, OR 97202 Vancouver, WA 98660 (503) 221-1131 (360) 750-1131

**OWNER:** Den Maldonado

FDM Development Inc.

PO Box 353

Ridgefield, WA 98642 (360) 719-0276

**LOCATION:** 968 SW Rock Creek Drive. 40 SW Cascade Avenue. The site has been assigned Tax Lot

Numbers 02-07-01-0-0-1302-00 and 20-07-01-0-0-1303-00 by the County Assessor.

**SHORELINE WATERBODY:** Rock Cove **SHORELINE ENVIRONMENT DESIGNATION:** Active Waterfront

SHORELINE USE PROPOSED: Commercial & Industrial (Water-Enjoyment), Land Division,

Recreational (Trail Parallel to the Shoreline, View Platform), Transportation & Parking Facilities

(Accessory Parking)

SHORELINE MODIFICATION PROPOSED: Vegetation Removal

**BACKGROUND:** Previously a veneer mill, the development site has been vacant for decades. Part of this time the site was under county ownership. In 1999, Skamania County divided the site into 3 legal lots. It remained vacant and was informally used for physical access to Rock Cove. In 2019, the

County sold the property to an investment group. That investment group obtained a

Mitigated Determination of Non-Significance under City File SEPA2020-01 for "a three-phased development, beginning with the condo-style units. Phase 2 will add the commercial venue

space and restore waterside portions of the property for enhanced, publicly-accessible observation and enjoyment. Phase 3 completes the development with the studio-sized units." A Shoreline Substantial Development Permit (SSDP) was issued for phase 1 of that proposal under City File SHOR2020-01. The SSDP was issued under the *Skamania County Shoreline Management Master Program* as it was adopted by the City in August 1975. The project was delayed during the COVID pandemic and SHOR2020-01 expired.

This proposal is for a new SSDP. The project's site plan is different from the previous approval and construction is proposed under a single phase. The proposal is subject to the *Stevenson Shoreline Master Program* as it was adopted in March 2022.

_____

#### STANDARDS, FINDINGS AND CONCLUSIONS

#### **SMC 18 SHORELINE MANAGEMENT**

Chapter 18.08 of the Stevenson Municipal Code is separated into 17 sections relating to management of shoreline water bodies (Columbia River, Rock Cove, Rock Creek). These sections adopt the Stevenson Shoreline Master Program (SMP) and detail procedures for obtaining approvals under the SMP. The SMP contains 7 chapters detailing submittal requirements, policies and regulations applicable when review activities are proposed in Shoreline Jurisdiction. Certain review activities require approval by the Shoreline Administrator, others require approval by the Stevenson Planning Commission, still others require approval by the Stevenson Planning Commission and the Washington Department of Ecology. The sections below relate to Shoreline Substantial Development Permits (SSDP), the mechanism involved when the Planning Commission approves review activities.

#### **SMC 18.08 - Shoreline Management**

Chapter 18.08 of the Stevenson Municipal Code (SMC) establishes procedural standards for implementation of the City's shoreline management program. The chapter is separated into 17 sections detailing program administration and project review. There are 14 sections reviewed prior to issuance of a Substantial Development Permit, 6 of which are the responsibility of the applicant. Findings and conclusions related to each section are detailed below

<u>CRITERION §18.08.010 – ADMINISTRATION AUTHORIZED.</u> "A. The "shoreline administrator" or "administrator" or that person's designee, is hereby vested with: [5 specific duties/authorities]

- B. The City of Stevenson Planning Commission is hereby vested with:
- 1. Authority to issue shoreline permits as required herein. "Shoreline permits" include shoreline substantial development permits, shoreline conditional use permits, and shoreline variances."
- FINDING(S): a. The proposal submitted involves activities, developments, and/or uses requiring issuance of a shoreline permit.
  - b. The Planning Commission is authorized to issue this shoreline permit.
- CONCLUSIONS OF LAW: This project will comply with SMC 18.08.010 subject to the review conducted herein.

<u>CRITERION §18.08.020 – SHORELINE MASTER PROGRAM AND MAP ADOPTION.</u> "A. There is made a part of this chapter a management plan which shall be known as the "Stevenson Shoreline Master Program," adopted March 17th, 2022, as well as a map which shall be officially known as the "Stevenson Shoreline Environment Designation Map." These documents shall be

made available to the general public upon request.

B. The Shoreline Environment Designation Map generally shows the shoreline areas of the city which are under the jurisdiction of the Act and the shoreline environments as they affect the various lands and waters of the city. The precise location of shoreline jurisdiction and shoreline environment boundaries shall be determined according the appropriate provisions of the SMP."

#### FINDING(S):

- a. The proposal is subject to review according to the provisions of the Stevenson Shoreline Master Program.
- b. The proponents' application included precise locations of shoreline jurisdiction and shoreline environment boundaries pursuant to the Stevenson Shoreline Management Program.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMC 18.08.020 without conditions.

<u>CRITERION §18.08.050 – APPLICABILITY OF PROVISIONS, SHORELINES DESIGNATED.</u> "A. Unless specifically exempted by state statute, all proposed uses and development occurring within shoreline jurisdiction must conform to chapter 90.58 RCW, the Shoreline Management Act, and the Stevenson Shoreline Master Program.

- B. This chapter applies to all areas within shoreline jurisdiction as designated in the SMP, including:
- 1. That portion of the Columbia River shoreline which lies within city limits. This chapter will apply to any Columbia River shoreline which lies within city limits. This chapter will apply to any Columbia River shoreline which is annexed into the city. The entire Columbia River shoreline is a Shoreline of State-Wide Significance;
  - 2. The Rock Cove shoreline;
- 3. That portion of the Rock Creek shoreline which lies within city limits. This chapter will apply to any Rock Creek shoreline which is annexed into the city.
- 4. Any portion of the Ashes Lake shoreline which is annexed into the city; provided, the annexed shoreline has been predesignated within the SMP."

#### FINDING(S):

- a. The proposal is not specifically exempted by state statute.
- b. The proposal is located within the shoreline jurisdiction of a portion of Rock Cove lying within city limits.
- c. The proposal must conform to the Shoreline Management Act and the Stevenson Shoreline Management Program.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMC 18.08.050 without conditions.

# <u>CRITERION §18.08.080 – SHORELINE PERMITS & APPROVALS—REQUIRED WHEN.</u> "A. Any person wishing to undertake activities requiring a Minor Project Authorization or a Shoreline Permit (Shoreline Substantial Development Permit, Shoreline Conditional Use Permit, or Shoreline Variance) within shoreline jurisdiction shall apply to the Shoreline Administrator for appropriate approval.

B. In addition to the provisions contained herein, the authorization to undertake use or development in shoreline jurisdiction is subject to review according to the applicability, criteria, and process described in the SMP, especially SMP Chapter 2."

#### FINDING(S):

- a. The proponents wish to undertake an activity requiring a Shoreline Permit and submitted a complete application for a Substantial Development Permit on August 11th, 2023.
- b. Review according to SMP Chapter 2 is addressed below.

CONCLUSIONS OF LAW: This will comply with SMC 18.08.080 without conditions.

<u>CRITERION §18.08.100 – PERMITS—APPLICATION PROCEDURE.</u> "A. Any person required to comply with the Shorelines Management Act of 1971 and this chapter shall obtain the proper application forms from the city planning department. The completed application shall then be submitted to the shoreline administrator.

- B. Upon receipt of an application, the shoreline administrator shall determine which category of proposal has been submitted:
- 1. Category A applications involve requests for all shoreline permits, including a) Shoreline Substantial Development Permits, b) Shoreline Conditional Use Permits, c) Shoreline Variances, and d) revisions to any previously authorized Category A proposal.
- 2. Category B applications involve requests for a) a Minor Project Authorization issued pursuant to WAC 173-27-050, b) limited utility extensions and bulkheads approved pursuant to WAC 173-27-120, c) revisions to any previously authorized Category B proposal, and d) extensions of shoreline substantial development permits and Minor Project Authorizations" C. After determining the application category, the administrator will then review the application for completeness according to this chapter and the SMP.

#### FINDING(S):

- a. The proponents submitted a complete application on August 11, 2023.
- b. Upon submission of the application, the Shoreline Administrator determined the application as complete for a proposal involving a Category A Shoreline Permit.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMC 18.08.100 without conditions.

<u>CRITERION §18.08.110 – PERMITS—NOTICE PUBLICATION.</u> "A. Within 14 days after a determination of completeness under SMC 18.08.100, the Shoreline Administrator shall provide a notice of application for all Category A proposals as follows:

- 1. Content. The content of the notice shall be identical to that set forth in WAC 173-27-110(2). In addition, the notice shall state the time and place of the open record public hearing to be held for the Category A proposal.
- 2. On-Site Notice. No less than 2 notices shall be posted by the administrator in conspicuous places on or adjacent to the subject property.
- 3. Mailing. The notice shall be mailed to a) the land owner, b) all property owners of record within a radius of 300 feet of the exterior boundaries of the subject property, c) all agencies with jurisdiction per chapter 43.21C RCW, and d) individuals, organizations, tribes, and agencies that request such notice in writing.
- 4. Newspaper. The notice shall be published at least once a week, on the same day of the week, for two consecutive weeks in a newspaper circulating and published within the city.
- B. Category B proposals reviewed under WAC 173-27-120 require the same notice of application as Category A proposals. All other Category B proposals do not require notice of application."

#### FINDING(S):

- a. The notice prepared for this proposal includes the content set forth in WAC 173-27-110(2).
- b. The notice was posted on site on August 22, 2023. On August 26, 2023 staff observed that the notice was removed. On August 29, 2023, the notices were replaced.
- c. The notice was mailed to the required recipients on or about August 22, 2023.
- d. The notice was published in *The Columbian* on August 29 and September 5, 2023

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMC 18.08.110 without conditions.

<u>CRITERION §18.08.120 – PERMITS—FEES.</u> "A. An application for an approval under this chapter shall be accompanied by an application fee payable to the city in an amount established and periodically adjusted by the city council.

B. Payment of an application fee does not guarantee that a permit will be issued.

<u>FINDING(S):</u> a. The proponents submitted the applicable fee on August 11th, 2023.

CONCLUSIONS OF LAW: This project will comply with SMC 18.08.120 without conditions.

<u>CRITERION §18.08.140 – PERMITS—INTERESTED PARTIES—COMMENT PERIOD.</u> "A. For any Category A proposal, any member

- of the public may provide written comments for thirty days after the last publication of the notice of application.
- B. For Category B proposals reviewed under WAC 173-27-120, any member of the public may provide written comments for twenty days after the last publication of the notice of application.
- C. During the public comment periods established in this section, any member of the public may also request to be notified of the action taken by the city.
- <u>FINDING(S):</u> a. The City received written comments from: (none).
  - b. The City received requests to be notified of action from: (none).
- <u>CONCLUSIONS OF LAW:</u> This project will comply with SMC 18.08.120 without conditions.
- <u>CRITERION §18.08.180 PLANNING COMMISSION ACTION—CATEGORY A PROPOSALS.</u> "A. No authorization to undertake proposed Category A use or development shall be granted by the planning commission until at least one open record public hearing has been held and the proposed use and development is determined to be consistent with the policy and provisions of the SMA and the SMP.
  - B. At the public hearing scheduled for consideration of a Category A proposal by the planning commission, the commission shall, after considering all relevant information available and evidence presented to it, either grant, conditionally grant, or deny the permit.
  - C. In granting or revising a permit, the commission may attach thereto such conditions, modifications and restrictions regarding the location, character and other features of the proposed development as it finds necessary. Such conditions may include the requirement to post a performance bond assuring compliance with other permit requirements, terms and conditions.
  - D. The decision of the planning commission shall be the final decision of the city on all applications for Category A proposals. The commission shall render a written decision including findings, conclusions and a final order, and transmit copies of its decision to the persons who are required to receive copies of the decision pursuant to Section 18.08.190.

## FINDING(S):

- a. The Planning Commission held an open record public hearing on October 9, 2023.
- b. This document catalogues the Planning Commission's consideration of information available and evidence presented regarding this Category A proposal.
- c. The Planning Commission has not required the posting of a performance bond to assure compliance with the permit.
- d. This document constitutes the written decision of the Planning Commission and the City's final decision on this permit.
- CONCLUSIONS OF LAW: This project will comply with SMC 18.08.120 upon satisfaction of the conditions contained herein.
- <u>CRITERION §18.08.185 SHORELINE ADMINISTRATOR ACTION—CATEGORY B PROPOSALS.</u> [THIS SECTION APPLIES TO A DIFFERENT TYPE OF PROJECT THAN HAS BEEN PROPOSED. NO CONSISTENCY REVIEW IS NECESSARY.]
- <u>CRITERION §18.08.190 NOTIFICATION AND FILING OF ACTION.</u> [THIS SECTION APPLIES TO CITY PROCEDURES AFTER A DECISION IS MADE. NO CONSISTENCY REVIEW IS NECESSARY.]
- CRITERION §18.08.200 APPEAL FROM PERMIT DECISION. "Any person aggrieved by the granting or denying of a substantial development permit, conditional use permit, variance, or by the rescinding of a permit pursuant to the provisions of this chapter may seek review from the shorelines hearing board. Such an appeal must be filed as a request for the same within twenty-one days of receipt of the final order and by concurrently filing copies of such request with ecology and the attorney general's office. The state hearings board regulations of RCW 90.58.180 and Chapter 461-08 WAC apply. A copy of such

appeal notice shall also be filed promptly with the City of Stevenson. Upon issuance of a final order after an appeal, the city shall provide said order to ecology according to WAC 173-27-130(10).

FINDING(S):

- a. The appeal process applies to the proponent and any person aggrieved by the City decision.
- b. The appeal period coincides with the timelines established in SMC 18.08.210.
- c. A condition is appropriate to provide guidance on this procedure.

CONCLUSIONS OF LAW: This project will comply with SMC 18.08.120 upon satisfaction the condition below.

#### **CONDITIONS:**

1. Timely appeals shall be filed by the proponent within 21 days of Receipt of the Final Decision.

<u>CRITERION §18.08.205 — APPEAL FROM ADMINISTRATOR DECISION.</u> [THIS SECTION APPLIES TO A DIFFERENT TYPE OF PROJECT THAN HAS BEEN PROPOSED. NO CONSISTENCY REVIEW IS NECESSARY.]

<u>CRITERION §18.08.210 — PERMIT ISSUANCE AND EFFECT.</u> "A. The effective date of a substantial development permit shall be the date of filing as provided in RCW 90.58.140(6).

B. Each shoreline permit shall contain a provision that construction pursuant to the permit shall not begin and is not authorized until twenty-one days from the date of filing with ecology, per WAC 173-27-190 or as subsequently amended, or until all review proceedings initiated within twenty-one days from the date of such filing have been terminated.

C. Issuance of a permit does not obviate the applicant from meeting requirements of other federal, state and county permits, procedures and regulations.

FINDING(S):

a. A condition is appropriate to incorporate SMC 18.08.210(B) into the permit decision.

CONCLUSIONS OF LAW: This project will comply with SMC 18.08.210 upon satisfaction of the conditions contained herein.

# **CONDITIONS:**

2. **Construction pursuant to this Permit shall not begin** and is not authorized until 21 days from the date of filing with Ecology, per WAC 173-27-190 or as subsequently amended, or until all review proceedings initiated within 21 days from the date of such filing have been terminated.

CRITERION §18.08.220 – PERMIT DURATIOIN—EXTENSIONS. "A. Construction activities shall be commenced, or where no construction activities are involved, the use or activity shall be commenced within two years of the effective date of an authorization or shoreline permit issued under this chapter. However, the city may authorize a single extension for a period not to exceed one year based on reasonable factors, if a request for extension has been filed before the expiration date and notice of the proposed extension is given to ecology and parties of record on the original authorization or permit.

B. Authorization to conduct development activities shall terminate five years after the effective date of an authorization or shoreline permit. However, the city may authorize a single extension for a period not to exceed one year based on reasonable factors, if a request for extension has been filed before the expiration date and notices of the proposed extension is given to ecology and parties of record on the original authorization or permit.

C. Upon a finding of good cause, based on the requirements and circumstances of the specific project proposed and consistent with the policies and provisions of the SMP and WAC 173-27, the city may adopt different time limits from those

set forth above as a part of action on a shoreline permit.

D. The time periods in this section do not include the time during which a use or activity was not actually pursued due to the pendency of administrative appeals or legal actions or due to the need to obtain any other government permits and approvals for the development that authorize the development to proceed, including all reasonably related administrative or legal actions on any such permits or approvals.

#### FINDING(S):

- a. Timelines differing from this set forth in SMC 18.08.220(A) & (B) are not necessary for this permit.
- b. Conditions are appropriate to incorporate the timelines of SMC 18.08.220(A), (B), and (D) into the permit decision.
- c. Conditions are necessary to ensure permit timelines continue to be met in the face of unforeseen delays under SMC 18.08.220(D).

CONCLUSIONS OF LAW: This project will comply with SMC 18.08.220 upon satisfaction of the conditions contained herein.

#### **CONDITIONS:**

- 3. **Within 2 years of the effective date of this permit,** construction activities associated with this permit shall commence or a written request for a maximum 1-year extension shall be submitted to the City. If construction activities do not commence accordingly, the permit shall expire.
- 4. **Within 5 years of the effective date of this permit,** all development activities associated with this permit shall terminate or a written request for a maximum 1-year extension shall be submitted to the City.
- 5. **Prior to the start of construction**, the proponent shall submit the City documentation sufficient to establish an accurate timeline of any activity justifying an extension of the permit's duration based on SMC 18.08.220(D). No such documentation will be accepted by the City after construction commences.

<u>CRITERION §18.08.235 – VARIANCE AND CONDITIONAL USE PERMITS—ECOLOGY REVIEW.</u> [THIS SECTION APPLIES TO A DIFFERENT TYPE OF PROJECT THAN HAS BEEN PROPOSED. NO CONSISTENCY REVIEW IS NECESSARY.]

<u>CRITERION §18.08.250 – ENFORCEMENT—PENALTIES.</u> "All provisions of this chapter shall be enforced by the shoreline administrator and/or a designated representative. The enforcement procedures and penalties contained in WAC 173-27 and RCW Chapter 90.58 are hereby incorporated by reference."

<u>FINDING(S):</u> a. A condition is appropriate to incorporate SMC 18.08.250 into the permit decision.

CONCLUSIONS OF LAW: This project will comply with SMC 18.08.250 upon satisfaction of the conditions contained herein.

#### **CONDITIONS:**

- 6. **Throughout the Duration of this Permit,** the proponents shall provide reasonable access to the Shoreline Administrator to ensure enforcement of this permit and the SMP.
- 7. **Throughout the Duration of Construction**, the proponents shall contact the Shoreline Administrator prior to constructing any change to the proposal to determine whether the

- change should be permitted and whether the permission should be through a revision to this Minor Project Authorization or through a Shoreline Permit.
- 8. **Prior to the Start of Construction** the applicants shall provide construction documents for approval by the Shoreline Administrator. The review shall be limited, ensuring the project's consistency with the proposal and ensuring the conditions of City permits have been appropriately incorporated. Should the Administrator fail to respond within 7 days of receipt, the construction documents shall be presumptively approved.

#### **Stevenson Shoreline Master Program**

The Stevenson Shoreline Master Program (SMP) is adopted by SMC 18.08.020 Chapter 18.08. The program is divided into 7 chapters. Each chapter contains several sections of standards addressing specific aspects of shoreline management. Findings and conclusions related to each section are detailed below.

#### **CHAPTER 1 - INTRODUCTION**

CRITERIA §1.1, §1.2, §1.3.1, §1.3.2, §1.4, §1.6, §1.7, §1.8, §1.9, §1.10. [These sections contains guidance applicable to all criterion in the SMP but no specific regulations. No consistency review is necessary.]

<u>CRITERION §1.3.3 – SHORELINE ENVIRONMENT DESIGNATION MAP.</u> "The approximate shoreline jurisdictional area and the Shoreline Environment Designations (SEDs) are delineated on the map(s), hereby incorporated as a part of this SMP that shall be known as the "Stevenson Shoreline Environment Designation Map" (See Appendix A).

The boundaries of the shoreline jurisdiction on the maps are approximate. The actual extent of shoreline jurisdiction for specific project proposals shall be based upon the actual location of the OHWM, floodway, and the presence and delineated boundaries of associated wetlands as determined after an on-site inspection and in accordance with SMP Sections 1.3.1 and 1.3.2, Chapter 3, Chapter 7, and RCW 90.58.030."

#### FINDING(S):

- a. The Critical Areas Report prepared by Ecological Land Services, Inc. and submitted with this proposal includes a map of shoreline jurisdiction based on the actual location of the OHWM as observed in the field.
- b. The proposal is located within an area designated as Active Waterfront and extends through an area designated Aquatic on the Stevenson Shoreline Environment Designation Map.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 1.3.2 without conditions.

<u>CRITERION §1.5 – SHORELINE MASTER PROGRAM APPLICABILITY TO DEVELOPMENT.</u> "The SMP shall apply to all land and waters under the jurisdiction of Stevenson as identified in SMP Sections 1.3.1, 1.3.2, and 1.3.3 above.

This SMP shall apply to every person (i.e., individual, partnership, corporation, association, organization, cooperative, public or municipal corporation, or agency of the state or local governmental unit however designated) that uses, develops, owns, leases, or administers lands, wetlands, or waters that fall under the jurisdiction of the SMA. The SMP shall not apply to federal agency activities on federal lands.

See SMP Chapter 2 below for more information on when a permit is required. The SMP applies to all review activities (i.e., shoreline uses, development, and modifications) proposed within shoreline jurisdiction. Some review activities under this program do not require a shoreline substantial development permit. However, such activities must continue to demonstrate compliance with the policies and regulations contained in this SMP in accordance with WAC 173-27-040(1)(b) and be authorized by a minor project authorization."

#### FINDING(S):

a. The SMP is applicable to this proposal by this proponent, for a project occurring within the jurisdiction of Stevenson as identified herein.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 1.5 without conditions.

# **CHAPTER 2 – ADMINISTRATIVE PROVISIONS**

CRITERIA §2.1, §2.2, §2.3.2, §2.4.1, §2.5, §2.7, §2.8, §2.9. [These sections contain guidance applicable to all and/or different types of project than has been proposed. No consistency review is necessary.]

<u>CRITERION §2.3.1 – PRE-APPLICATION CONFERENCE—REQUIRED.</u> "A pre-application conference for all proposed review activities within shoreline jurisdiction is required. The Shoreline Administrator may waive this requirement if the applicant requests such in writing and demonstrates that the usefulness of a pre-application meeting is minimal."

FINDING(S): a. A series of pre-application conferences, emails, and phone calls throughout the Spring and Summer of 2023 occurred in advance of this project's application.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 2.3.1 without conditions.

<u>CRITERION §2.3.3 – DETERMINATION OF ORDINARY HIGH WATER MARK.</u> "For any development where a determination of consistency with the applicable regulations requires a precise location of the OHWM, the mark shall be located precisely with assistance from Ecology and City staff, or a qualified professional, and the biological and hydrological basis for the location shall be included in the development plan. Where the OHWM is neither adjacent to or within the boundary of the project, the plan shall indicate the distance and direction to the nearest OHWM of a shoreline."

FINDING(S): a. The Critical Areas and FWHCA Report prepared by Ecological Land Services, Inc.,

a qualified professional, and submitted with this proposal includes a determination

of the OHWM as observed in the field.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 2.3.3 without conditions.

<u>CRITERION §2.4.2 – APPLICATION CONTENTS.</u> [This section contains 2 lists of required submittals for approval or projects in shoreline jurisdiction and provides the Shoreline Administrator authority to require additional information. For brevity, the full text of this section is omitted.]

FINDING(S): a. The findings of SMC 18.08.100 related to the acceptance of a complete

application are relevant to this criterion.

CONCLUSIONS OF LAW: This project will comply with SMP 2.3.3 without conditions.

CRITERION §2.4.3 – APPLICATION REVIEW & PROCESSING. "1. When an application is deemed complete, the Administrator may request third-party peer review of any report, assessment, delineation, or mitigation plan by a qualified professional and/or state or federal resource management agency. Such request shall be accompanied by findings supporting the Administrator's decision, which is appealable to the City Council. The City may incorporate recommendations from such third-party reports in findings approving or denying an application. In general, the cost of any third-party review will be the responsibility of the applicant; however, where a project would provide a beneficial public amenity or service, on a case-by-case basis by City Council action, costs may be shared by the City.

- 2. The Shoreline Administrator shall review the information submitted by the applicant and, after an optional site visit shall determine the category of project proposed according to SMC 18.08.100.
- 3. Applications shall be processed according to the timelines and notice procedures listed in SMC 18.08.100 through SMC 18.08.190, the review criteria of this Chapter, and WAC 173-27.
- 4. The City shall use an existing, or establish a new, mechanism for tracking all project review actions in shoreline areas, and a process to evaluate the cumulative effects of all authorized development on shoreline conditions."

#### FINDING(S):

- a. Third-party peer review occurred during the 2020 permitting process. No additional third-party peer review was deemed necessary by the Shoreline Administrator for this proposal.
- b. The Shoreline Administrator determined this project is a Category A Shoreline Substantial Development Permit.
- c. The findings of related to the Stevenson Municipal Code related to application and review procedures are relevant to this criterion.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 2.4.3 without conditions.

#### CRITERION \$2.6.1 - SHORELINE SUBSTANTIAL DEVELOPMENT PERMITS - PURPOSE - APPLICABILITY - CRITERIA. "The purpose

of a Shoreline Substantial Development Permit (SSDP) is to assure consistency with the provisions of the SMA and this SMP. In authorizing a SSDP, the City may attach conditions to the approval as necessary to assure the project is consistent with all applicable standards of the SMA and this SMP. The following criteria shall assist in reviewing proposed SSDPs:

- 1. SSDPs may not be used to authorize any use that is listed as conditional or prohibited in a shoreline designation.
- 2. SSDPs may not be used to authorize any development and/or use which does not conform to the specific bulk, dimensional, and performance standards set forth in this SMP.
- 3. SSDPs may be used to authorize uses which are listed or set forth in this SMP as permitted uses.
- 4. To obtain a SSDP, the applicant must demonstrate compliance with all of the following review criteria as listed in WAC 173.27.150:
  - a. That the proposal is consistent with the SMA;
  - b. That the proposal is consistent with WAC 173-27 Shoreline Management Permit and Enforcement Procedures; and
  - c. That the proposal is consistent with this SMP and SMC 18.08 Shoreline Management."

<u>FINDING(S):</u> a. The sum of the findings contained herein are relevant to review of this criterion.

CONCLUSIONS OF LAW: This project will comply with SMP 2.6.1 upon fulfillment of the conditions contained herein.

CRITERION §2.6.2 — SHORELINE SUBSTANTIAL DEVELOPMENT PERMITS — PERMIT PROCESS. "Proposals for SSDPs are subject to the City's permit procedures articulated in SMC 18.08 — Shoreline Management and the State's permit procedures articulated in WAC 173-27 — Shoreline Management Permit and Enforcement Procedures."

<u>FINDING(S):</u> a. The findings of SMC 18.08 are relevant to this criterion.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 2.6.2 without conditions.

#### **CHAPTER 3 – SHORELINE ENVIRONMENT DESIGNATIONS**

CRITERIA §3.1, §3.2.1, §3.2.2, §3.2.3, §3.2.4. [These sections contain guidance applicable to all and/or different locations than has been proposed. No consistency review is necessary.]

CRITERION §3.2.5 — ACTIVE WATERFRONT ENVIRONMENT. "1 Purpose: The purpose of the Active Waterfront Environment is to recognize the existing pattern of mixed-use development and to accommodate new water-oriented commercial, transportation, recreation, and industrial uses while protecting existing ecological functions of open space, floodplain, and other sensitive lands and restoring ecological functions in areas that have been previously degraded.

2. Location Criteria: The Active Waterfront SED may apply to shorelands that 1) currently support or 2) are appropriate and planned for water-oriented commercial, transportation, recreation, and industrial development that is compatible with protecting or restoring of the ecological functions of the area.

#### 3. Management Policies:

- a. Prefer uses that preserve the natural character of the area or promote preservation of open spaces and sensitive lands, either directly or over the long term. Allow uses that result in restoration of ecological functions if the use is otherwise compatible with the purpose of the environment and the setting.
- b. Give priority to water-oriented uses, with first priority to water-dependent, then second priority to water-related and water-enjoyment uses. For shoreline areas adjacent to commercially navigable waters, give highest priority to water-dependent uses.
- c. Prohibit new non-water-oriented uses, except: i) As part of mixed use development; ii) In limited situations where they do not conflict with or limit opportunities for water-oriented uses; iii) On sites where there is no direct access to the shoreline; iv) As part of a proposal that result in a disproportionately high amount of restoration of ecological functions.
- d. Assure no net loss of shoreline ecological functions as a result of new development through shoreline policies and regulations. Where applicable, new development shall include environmental cleanup and restoration of the shoreline to comply in accordance with any relevant state and federal law.
- e. Require public visual and physical access and implement public recreation objectives whenever feasible and where significant ecological impacts can be mitigated."

#### FINDING(S):

- a. The proposal is located in the Active Waterfront Shoreline Envinroment Designation (SED) and involves water-enjoyment commercial, a use preferred in the SED.
- b. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.
- c. Public visual and physical access is feasible on this site where significant ecological impacts can be mitigated and avoided.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 3.2.5 without conditions.

#### CHAPTER 4 – GENERAL PROVISIONS FOR ALL SHORELINE ACTIVITIES

CRITERIA §4.1, §4.4.5, §4.5, §4.8. [These sections contain guidance applicable to all and/or different locations or project types than has been proposed. No consistency review is necessary.]

#### CRITERION §4.2.3 — [CULTURAL RESOURCES] REGULATIONS]. "1 Site Inspections, Evaluations, and Surveys — Required When:

- a. When a shoreline use or development is within 500 feet of an area documented to contain, or likely to contain, archaeological, cultural, or historic resources based on information from DAHP, a prior archaeological report/survey, or a state or federal register, the applicant shall provide a site inspection and evaluation report prepared by a qualified cultural resource professional prior to issuance of any Shoreline Permit or approval, including a Minor Project Authorization. Work may not begin until the inspection and evaluation have been completed, and the City has issued its permit or approval.
- b. A survey to identify archaeological, cultural, and historic resources 50 years of age and older may be required to be conducted based on the recommendations of a cultural resources professional contained in the site inspection and evaluation report. The cultural resource survey process shall conform to the most recent update of DAHP's Standards for Cultural Resource Reporting found at this link:

#### https://dahp.wa.gov/sites/default/files/CR%20Update%20August%202018%20final.pdf.

- 2. Cultural Resources Avoidance. If an archaeological site inspection or evaluation identifies the presence of significant archaeological, cultural, or historic resources at the site, the applicant shall first seek to avoid impacts to the resource.
- 3. Cultural Resources Management Plan. If an archaeological site inspection or evaluation identifies the presence of significant archaeological, cultural, or historic resources that will be impacted by a project and if recommended by a qualified cultural resource professional, a cultural resource management plan shall be prepared prior to the City's approval of the project. A qualified cultural resource professional(s) shall prepare the cultural resource management plan. Cultural resource management plans shall be developed in consultation with DAHP and affected Tribes. In addition, a permit or other requirement administered by DAHP pursuant to RCW 27.44 and RCW 27.53 may apply. If the cultural resource professional determines that impacts to an archaeological, cultural, or historic resource can be adequately avoided by establishing a work limit area within which no project work or ground disturbance may occur, then a cultural resources management plan is not required.

- 4. Inadvertent discovery. If any item of possible archaeological interest (including human skeletal remains) is discovered on site during construction or site work, all the following steps shall occur:
- a. Stop all work in the immediate area (initially allowing for a 100' buffer, this number may vary by circumstance) immediately;
  - b. Implement reasonable measures to protect the discovery site, including any appropriate stabilization or covering;
  - c. Take reasonable steps to ensure the confidentiality of the discovery site;
  - d. Take reasonable steps to restrict access to the site of discovery;
  - e. Notify the City, DAHP, and Yakama, Nez Perce, Warm Springs, Umatilla, and Cowlitz tribes of the discovery.
  - f. A stop-work order will be issued.
  - g. The Shoreline Permit will be temporarily suspended.
- h. All applicable state and federal permits shall be secured prior to commencement of the activities they regulate and as a condition for resumption of development activities.
  - i. Development activities may resume only upon receipt of City approval.
- j. If the discovery includes human skeletal remains, the Skamania County Coroner and local law enforcement shall be notified in the most expeditious manner possible. The County Coroner will assume jurisdiction over the site and the human skeletal remains, and will make a determination of whether they are crime-related. If they are not, DAHP will take jurisdiction over the remains and report them to the appropriate parties. The State Physical Anthropologist will make a determination of whether the remains are Native American and report that finding to the affected parties. DAHP will handle all consultation with the affected parties as to the preservation, excavation, and disposition of the remains "

#### FINDING(S):

- a. A cultural resources study, was prepared for this site on February 4, 2020 by Applied Archaeological Research, Inc. which recommended an inadvertent discovery plan be followed.
- b. In 2020 under City File SEPA2020-01, a Mitigated Determination of Nonsignificance (MDNS) was issued on the basis of that report, and a mitigation measure incorporated an Inadvertent Discovery Policy into the project's expectations. The City received comments on the MDNS from DAHP requesting preparation and implementation of a monitoring plan. In its final decision on the Shoreline Substantial Development Permit under the old SMP, the City Council disregarded the DAHP request and no monitoring plan was required.
- c. Construction activities completed under the 2020 approvals involved installation of underground utilities; however additional excavations are likely under this permit and the current SMP is more directive related in terms of cultural resources protection.
- d. Through a proposal involving similar excavations in areas where cultural resources might be encountered, the City of Stevenson Public Works Department prepared and implemented a monitoring plan as part of it's inadvertent discovery policy.
- e. Conditions are appropriate to incorporate an inadvertent discovery policy and monitoring plan into the project's construction.

CONCLUSIONS OF LAW: This project will comply with SMP 4.2.3 upon fulfillment of the conditions below.

#### **CONDITIONS:**

9. In areas where excavation will exceed the depth of fill materials, an archaeological monitor will be required to be on site during the excavation. The monitoring plan shall be reviewed by DAHP, include an inadvertent discovery plan, and be incorporated into the project construction documents.

- <u>CRITERION §4.3.3 [ENVIRONMENTAL PROTECTION & NO NET LOSS] REGULATIONS.</u> "1. Mitigation Sequence. In order to ensure that review activities contribute to meeting the no net loss provisions by avoiding, minimizing, and mitigating for adverse impacts to ecological functions or ecosystem-wide processes, applicants shall describe how the proposal will follow the sequence of mitigation as defined below:
  - a. Avoid the impact altogether by not taking a certain action or parts of an action;.
  - b. Minimize the impacts by limiting the degree or magnitude of the action and its implementation by using appropriate technology or by taking affirmative steps (e.g., project redesign, relocation, timing to avoid or reduce impacts, etc.):
  - c. Rectify the impact by repairing, rehabilitating, or restoring the affected environment to the conditions existing at the time of the initiation of the project or activity;
  - d. Reduce or eliminate the impact over time by preservation and maintenance operations during the life of the action;
    - e. Compensate for the impact by replacing, enhancing, or providing substitute resources or environments; and
    - f. Monitor the impact and the compensation projects and take remedial or corrective measures when necessary.
  - 2. The mitigation sequence is listed in the order of priority. Applicants shall consider and apply lower priority measures only where higher priority measures are determined to be infeasible or inapplicable.
  - 3. SEPA Compliance. To the extent SEPA applies to a proposal, the analysis of environmental impacts and mitigation related to the proposal shall be conducted consistent with WAC 197-11—SEPA Rules and SMC 18.04—Environmental Policy.
  - 4. Cumulative Impacts. As part of the assessment of environmental impacts subject to this SMP, new uses, developments, and modifications shall evaluate and consider cumulative impacts of reasonably foreseeable future development on shoreline ecological functions. Evaluation of cumulative impacts shall consider:
    - a. Current circumstances affecting the shorelines and relevant natural processes;
    - b. Reasonably foreseeable future development and use of the shoreline; and
    - c. Beneficial effects of any established regulatory programs under other local, state, and federal laws.
  - 5. Mitigating for Impacts. When impacts related to a proposal require mitigation, the following shall apply:
    - a. The proposal shall achieve no net loss of ecological functions.
  - b. The City shall not require mitigation in excess of that necessary to assure the proposal 1) results in no net loss of ecological function and 2) does not have a significant adverse impact on other shoreline functions fostered by this SMP.
  - c. Compensatory mitigation shall give preference to measures that replace the impacted function directly and in the immediate vicinity of the impact. However, alternative compensatory mitigation located elsewhere in the same reach or watershed that addresses limiting factors or identified critical needs for shoreline resource conservation may be authorized, including appropriate actions identified in the Restoration Plan.
  - d. Unless waived by the City, authorization of compensatory mitigation shall require appropriate safeguards, terms or conditions (e.g. performance bonding, monitoring, conservation covenants) as approved by the City Attorney and necessary to ensure no net loss of ecological functions.
  - 6. Environmental protection and no net loss shall be achieved by complying with the combination of use regulations, shoreline setbacks, critical area buffers, and vegetation removal restrictions:
  - a. Shoreline Allowances & Setbacks Table 5.1 establishes a list of permitted, conditional, and prohibited uses in each shoreline environment designation (SED). This table also establishes the minimum shoreline setback applicable to each use, activity, or development within each SED where development cannot occur; and
  - b. Critical Areas Buffers Section 4.4 Critical Area provisions, including separately incorporated SMC 18.13 provisions that establish Wetland and Riparian buffer standards as additional areas where mitigation sequencing must be applied and unavoidable impacts must be mitigated; and
  - c. Modifications & Vegetation Shoreline modification standards, vegetation standards, and prescriptive mitigation measures of Chapter 6 apply to all vegetation impacts occurring within shoreline jurisdiction.

#### FINDING(S):

- a. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- b. Via written comment, the Washington Department of Ecology contends the applicant has not met its the burden of proof in describing the mitigation sequence was followed for this site.
- c. The mitigation sequence, as more fully described through supplemental

information submitted into the record at the public hearing on this application, has been appropriately followed.

d. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.

CONCLUSIONS OF LAW: This project will comply with SMP 4.3.3 upon fulfillment of the conditions contained herein.

CRITERION §4.4.3 – GENERAL CRITICAL AREA REGULATIONS. "1 The City of Stevenson shall not issue any Shoreline Permit (i.e., SSDP, SCUP, shoreline variance) or Minor Project Authorization (MPA), or otherwise issue any authorization to alter the condition of any land, water, or vegetation, or to construct or alter any structure or improvement in, over, or on a shoreline critical area or associated buffer, without first assuring compliance with the requirements of this section and SMC 18.13, as applicable.

- 2. Early Disclosure and Verification. When an applicant submits an application for any development proposal, it shall indicate whether any critical areas or buffers are located on or within 300 feet of the site. The presence of critical areas may require additional studies and time for review. However, the City shall review proposals involving critical areas protection under a single application, timeline, fee, and permit as the required Shoreline Permit or MPA. Early disclosure of critical areas will reduce delays during the permit review process. If the applicant states there are no known critical areas, the City should review and confirm whether critical areas exist, and, if critical areas are present, require the applicant to complete a critical areas report.
- 3. Studies generated as part other federal or state permit processes (e.g., SEPA submittals, biological opinions, biological evaluations, etc.) shall be provided and may be determined by the Administrator as adequate to satisfy the critical areas report requirements of this SMP if the project has been developed in enough detail to have evaluated site-specific impacts and mitigation measures.
- 4. New development and the creation of new lots are prohibited in all SEDs when they would cause foreseeable risk from geological conditions, or require structural flood hazard reduction measures in the floodway or CMZ, during the life of the development, consistent with SMP Section 5.4.8 Land Division, and other provisions of this Program."

#### FINDING(S):

- a. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- b. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.
- c. The applicant's narratives and reports responding to SMP 4.3, SMP 4.4, and SMC 18.13 result in protective buffers from Rock Cove. However, the submittals do not document the project's consistency with buffer demarcation requirements of SMC 18.13.057(A-C).
- d. The applicants supplied a Critical Areas Report in support of the 2020 permit request, which was subsequently reviewed and determined consistent with the Stevenson Critical Areas Code (SMC 18.13) by an independent third-party.
- e. Updated Critical Areas Report supplied in support of the current permit request retains the key findings of the previous assessment, while addressing additional development phases for which a permit was not previously requested.
- f. Via written comment, the Washington Department of Ecology contends the applicant's proposed on-site mitigation is insufficient to satisfy the Stevenson Critical Areas Code (SMC 18.13).
- e. Off-site mitigation, as more fully described through supplemental information submitted into the record at the public hearing on this application, is sufficient to address the additional development phases and satisfy the Stevenson Critical Areas Code.

CONCLUSIONS OF LAW: This project will comply with SMP 4.4.3 upon fulfillment of the conditions below.

#### **CONDITIONS:**

- 10. **Prior to the Start of Construction** the applicants shall update the Critical Areas and FWHCA Report to include construction staking and permanent demarcation of the functionally isolated buffer consistent with SMC 18.13.057(A and C) and, where appropriate, incorporate it into project construction documents.
- 11. **Prior to the Start of Construction** the property owner shall record a deed notice related to the critical habitat area. This approval, together with the updated Critical Areas Report and Buffer Enhancement Plan shall be attached to the notice.

CRITERION §4.4.4 – FISH & WILDLIFE HABITAT CONSERVATION AREA REGULATIONS. "1 Any use, development, or modification proposed within or adjacent to an FWHCA with which state or federally endangered, threatened, or sensitive species have a primary association, shall ensure the FWHCA is protected as required by this SMP. If the Shoreline Administrator determines that a proposal is likely to impact an FWHCA adversely, additional protective measures (e.g., protective buffer standards, mitigation, and monitoring programs under SMC 18.13) may be required..

- 2. Applicants shall provide a preliminary FWHCA assessment for all proposals involving riparian areas. The assessment must recognize the buffer necessary to ensure no net loss of ecological functions occurring at the reach-scale for the riparian area in question.
- 3. The City shall condition the approval of activities located in the FWHCA or its buffer as necessary. Approval conditions shall require the applicant to mitigate any potential adverse impacts according to the approved critical area report, mitigation, and monitoring plans.
- 4. Structures that prevent the migration of salmonids shall not be allowed in the portion of water bodies currently or historically used by anadromous fish. Fish bypass facilities shall be provided, as necessary, to allow the upstream and downstream migration of all salmonid life stages and shall prevent juveniles migrating downstream from being trapped or harmed."

FINDING(S): a. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.

b. The proposal involves no structures preventing migration of salmonids.

CONCLUSIONS OF LAW: This project will comply with SMP 4.3.3 upon fulfillment of the conditions contained herein.

<u>CRITERION §4.6.3 – PUBLIC ACCESS REGULATIONS.</u> "1 Consistent with legal/constitutional limitations, provisions for adequate public access shall be incorporated into all proposals for Shoreline Permits that have one or more of the following characteristics:

- a. The proposed development or use will create a demand for, or increase demand for public access;
- b. The proposed use is water-enjoyment, water-related, or non water-dependent, except for individual single-family residences not part of a development planned for 5 or more parcels;
  - c. The proposed use involves the subdivision of land into 5 or more parcels;
- d. The proposed development or use will interfere with existing access by blocking access or discouraging use of existing access;
  - e. The proposed development or use will interfere with public use of waters of the state;

- f. The proposed development or use will involve public funding or occur on public lands, provided that such access would not result in a net loss of ecological function. Public funding includes any funds from federal, state, municipal or local taxation districts.
- 2. Additional public access will not be required where suitable public access is already provided by an existing public facility on or adjacent to the site and the Planning Commission makes a finding that the proposed development would not negatively impact existing visual or physical public access nor create a demand for shoreline public access that could not be accommodated by the existing public access system and existing public recreational facilities in the immediate vicinity.
- 3. Public access will not be required where the applicant demonstrates it is infeasible due to at least one of the following:
  - a. Unavoidable health or safety hazards to the public exist that cannot be prevented by any practical means;
- b. Inherent security requirements of the use cannot be satisfied through the application of alternative design features or other solutions;
- c. The cost of providing the access, easement, or an alternative amenity are unreasonably disproportionate to the total long-term cost of the proposed development or other legal/constitutional limitations preclude public access;
  - d. Unacceptable environmental harm will result from the public access which cannot be mitigated;
- e. Significant unavoidable conflict between the proposed access and adjacent uses would occur and cannot be mitigated.
- 4. To meet any of the conditions under Regulation 3 above, the applicant must first demonstrate to the satisfaction of the Planning Commission that all reasonable alternatives have been exhausted including, but not limited to, the following:
  - a. Regulating access by such means as maintaining a gate and/or limiting hours of use;
  - b. Designing separation of uses and activities (e.g., fences, terracing, use of one-way glazings, hedges, landscaping);
  - c. Provisions for access at a site geographically separated from the proposal such as a street end, vista or trail system;
  - d. Sharing the cost of providing and maintaining public access between public and private entities.
- 5. For projects that meet the criteria of Regulation 3 above, the City may consider off-site public access or, if approved by the Planning Commission and agreed to by the applicant, the applicant may contribute a proportional fee to the local public access fund (payment in lieu).
- 6. If the City determines that public access is required pursuant to Regulation 1 above, the City shall impose permit conditions requiring the provision of public access that is roughly proportional to the impacts caused by the proposed development or use. The City shall demonstrate in its permit decision document that any such public access has a nexus with the impacts of the proposed development and is consistent with the rough proportionality standard.
- 7. When required, public access shall:
- a. Consist of a dedication of land or a physical improvement in the form of a walkway, trail, bikeway, corridor, viewpoint, park, deck, observation tower, pier, boat launch, dock or pier area, or other area serving as a means of view and/or physical approach to public waters and may include interpretive centers and displays, view easements, and/or decreased building bulk through height, setback, or façade limitations;
  - b. Include features for protecting adjacent properties from trespass and other possible adverse impacts;
  - c. Be fully developed and available for public use at the time of occupancy of the proposed use or activity;
  - d. Result in no net loss of shoreline ecological functions.
- 8. When required, physical public access shall be constructed to meet the following requirements for location, design, operation and maintenance:
- a. Public access sites shall be connected directly to the nearest public street or non-motorized trail through a parcel boundary, tract, or easement, wherever feasible;
- b. Signs indicating the public's right of access to shoreline areas shall be installed and maintained in conspicuous locations.
- c. Public access easements and permit conditions shall be recorded on the deed of title and/or on the face of a plat or short plat as a condition running in perpetuity with the land, provided, that the Planning Commission may authorize a conveyance that that runs contemporaneous with the authorized land use for any form of public access other than parallel pedestrian access. Said recording with the County Auditor's Office shall occur at the time of permit approval.
- d. Maintenance of the public access facility shall be the responsibility of the owner unless otherwise accepted by a public or nonprofit agency through a formal agreement approved by the City and recorded with the County Auditor's Office.
- e. Public access sites shall be made barrier-free for the physically disabled where feasible, and in accordance with the ADA.
- f. Any trail constructed shall meet the conditions described for shoreline areas in any trail or parks plan officially adopted by the City Council.
- 9. Views of the shoreline from public properties or substantial numbers of residences shall be protected through adherence to height and setback limits specified in this SMP. Where new development would completely obstruct or significantly

reduce the aesthetic quality of views from public properties or substantial numbers of residences, mitigation shall be required as follows:

- a. The City may require administrative modifications to standard setbacks, clustering of proposed structures, and modifications to landscaping and building massing when the Planning Commission determines that such modifications are necessary to maintain public views of the shoreline.
- b. The City shall work with the applicant to minimize the economic impacts of view mitigation. While upper story stepbacks and other changes to building placement and form may be required to provide view corridors, in no case shall the applicant be required to reduce the maximum building height for more than 30% of the building's width.
- c. The City may require specific public access improvements (e.g., public viewing decks, etc.) as mitigation in lieu of more significant modifications to site and building design when the Planning Commission finds such modifications would be an unreasonable financial burden on the applicant.
- 10. Where there is a conflict between water-dependent shoreline uses or physical public access and maintenance of views from public properties or substantial numbers of residences that cannot be resolved using the techniques in Regulation 9 above, the water-dependent uses and physical public access shall have priority, unless the Planning Commission finds a compelling reason to the contrary.
- 11. Future actions by the applicant, successors in interest, or other parties shall not diminish the usefulness or value of the public access provided."

#### FINDING(S):

- a. The proposal has one or more of the characteristics requiring public access.
- b. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- c. The site is subject to numerous public access easements which conflict with and/or where no public access project is proposed, however, the application makes no contention regarding the infeasibility of providing such public access.
- d. Via written comment, the Washington Department of Ecology contends the proposal has not appropriately addressed public access.
- e. Public access, as more fully described through supplemental information submitted into the record at the public hearing on this application, has been adequately provided at this site.

CONCLUSIONS OF LAW: This project will comply with SMP 4.6.3. without conditions.

#### CRITERION §4.7.3 – WATER QUALITY & NON-POINT SOURCE POLLUTION REGULATIONS. "1 Design, construction and operation of shoreline uses and developments shall incorporate measures to protect and maintain surface and groundwater quality in accordance with all applicable laws, so that there is no net loss of ecological functions.

- 2. Design, construction and operation of shoreline uses and developments shall incorporate measures to protect and maintain surface and groundwater quantity and quality in accordance with all applicable laws, so that significant impacts to aesthetic qualities or recreational opportunities do not occur. A significant impact to aesthetics or recreation would occur if a stormwater facility and accessory structures (e.g., fences or other features) have the potential to block or impair a view of shoreline waters from public land or from a substantial number of residences per RCW 90.58.320, or if water quality were degraded so as to discourage normal uses (e.g., swimming, fishing, boating, viewing, etc.).
- 3. Shoreline development and uses shall adhere to all required setbacks, buffers, and standards for stormwater facilities.
- 4. All review activities shall comply with the applicable requirements of all applicable City stormwater, drinking water protection, and public health regulations and the Stormwater Management Manual for Western Washington, including using low impact development techniques whenever feasible.
- 5. Sewage management. To avoid water quality degradation, sewer service is subject to the requirements outlined below.
- a. Any existing septic system or other on-site system that fails or malfunctions will be required to connect to the City sewer system if feasible, or make system corrections approved by Skamania County Community Development Department.
- b. Any new development, business, or multifamily unit shall connect to the City sewer system if feasible, or install an on-site septic system approved by Skamania County Community Development Department.

6. Materials requirements. All materials that may come in contact with water shall be untreated or treated wood, concrete, plastic composites or steel as approved by the USACE or WDFW, that will not adversely affect water quality or aquatic plants or animals."

<u>FINDING(S):</u> a. The proposal includes a Stormwater Report prepared consistent with the

Stormwater Management Manual for Western Washington.

b. The findings related to Chapter 5 are relevant to this criterion.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 4.7.3 without conditions.

#### **CHAPTER 5 – SHORELINE USE REGULATIONS**

CRITERIA §5.1, §5.3, §5.4.1, §5.4.2, §5.4.3, §5.4.5, §5.4.6, §5.4.7, §5.4.10, §5.4.12, §5.4.13. [These sections contain guidance applicable to all and/or different locations or project types than has been proposed. No consistency review is necessary.]

<u>CRITERION §5.2.2 – PROVISIONS APPLICABLE TO ALL USES.</u> "1. When determining allowable uses and resolving use conflicts within the City's shoreline jurisdiction, the following preferences shall apply in the order listed below:

- a. Reserve appropriate areas for protecting and restoring ecological functions to control pollution and prevent damage to the natural environment and public health.
  - b. Reserve shoreline areas for water-dependent and associated water-related uses.
  - c. Allow mixed uses projects that include or support water-dependent uses.
- d. Reserve shoreline areas for other water-related and water-enjoyment uses that are compatible with ecological protection and restoration objectives.
- e. Located single-family residential uses where they are appropriate and can be developed without significant impact to ecological functions or displacement of water-dependent uses.
- f. Limit nonwater-oriented uses to those locations where the above described uses are inappropriate or where nonwater-oriented uses demonstrably contribute to the objectives of the SMA.
- 2. New use and development shall be subject to the setback requirements and height limitations contained in Section 5.3 Shoreline Use Table, including Table 5.1 Shoreline Use & Setback Standards."
- <u>FINDING(S):</u> a. No Use conflicts are identified in association with this proposal.
  - b. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.

CONCLUSIONS OF LAW: This project will comply with SMP 5.2 upon satisfaction of the conditions contained herein.

- CRITERION §5.4.4.4 COMMERCIAL & INDUSTRIAL REGULATIONS. "a. Water-dependent commercial and industrial uses shall be given preference over water-related and water-enjoyment commercial and industrial uses. Second preference shall be given to water-related and water-enjoyment commercial and industrial uses over non-water-oriented commercial and industrial uses.
  - b. Prior to approval of water-dependent uses, the City shall review a proposal for design, layout and operation of the use and shall make specific findings that the use qualifies as a water-dependent use.
  - c. When allowed, industrial development shall be located, designed and constructed in a manner that assures no net loss of shoreline ecological functions.
  - d. Commercial development that is not water-dependent shall not be allowed over water except where it is located within the same existing building and is necessary to support a water-dependent use.
  - e Overwater and in-water construction of non-water-oriented industrial uses is prohibited. This provision is not intended to preclude the development of docks, piers, or boating facilities, or water-related uses that must be located in or over water (e.g., security worker booths, etc. that are necessary for the operation of the water-dependent or water-related use).
    - f. Only those portions of water-oriented industrial uses that require over or in-water facilities shall be permitted to

locate waterward of the OHWM, provided they are located on piling or other open-work structures, and they are limited to the minimum size necessary to support the structure's intended use.

- g. Water-related and water-enjoyment uses shall avoid impacts to existing navigation, recreation, and public access. h Non-water-oriented commercial and industrial development shall not be allowed unless:
- i. The use is part of a mixed-use project that includes water-dependent uses, and provides a significant public benefit with respect to provisions of public access or ecological restoration; or
- ii. Navigability is severely limited at the proposed site, and the commercial use provides a significant public benefit with respect to provision of public access or ecological restoration; or
- iiii. The site is designated for commercial use and is physically separated from the shoreline by another property or a public right-of-way.
- i. New commercial and industrial developments shall provide public access to the shorelines, subject to SMP Section 4.6.
- j. Public access and ecological restoration shall be considered as potential mitigation of impacts to shoreline resources and values for all water-related or water dependent development unless such improvements are demonstrated to be infeasible or inappropriate.
- k. New industrial developments shall mitigate for the impacts of the use's intensity by providing shoreline restoration consistent with the shoreline restoration plan adopted by the City.

#### FINDING(S):

- a. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- b. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.

CONCLUSIONS OF LAW: This project will comply with SMP 5.4.4 upon fulfillment of the conditions contained herein.

<u>CRITERION §5.4.8.4 – LAND DIVISION REGULATIONS.</u> "a Plats and subdivisions shall be designed, configured and developed in a manner that assures no net loss of ecological functions results from the plat or subdivision at full build-out of all lots..

- b. The layout of lots within 1) new plats and subdivisions, 2) plat amendments, or 3) boundary line adjustments shall:
- i. Prevent the need for new shoreline stabilization or flood hazard reduction measures that would cause significant impacts to other properties or public improvements or a net loss of shoreline ecological functions.
  - ii. Not result in lots containing inadequate buildable space due to critical areas and/or their buffers.
- c. To ensure the success of restoration and long-term maintenance, the City may require that critical areas and/or aquatic lands be placed in a separate tract which may be held by an appropriate natural land resource manager (e.g., homeowner's association, land trust, natural resource agency, etc.)."

#### FINDING(S):

- a. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- b. The proposed action under the City's Land Division Code is a combination of lots via boundary line adjustment. No additional lot is proposed.
- c. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 5.4.8 without conditions.

<u>CRITERION §5.4.9.4 – RECREATIONAL REGULATIONS.</u> "a Water-oriented recreational development shall be given priority and shall be primarily related to access, enjoyment, and use of the water and shorelines.

- b. Non-water-oriented recreational developments may be permitted only where it can be demonstrated that:
- i. A water-oriented use cannot feasibly locate on the proposed site due to topography and/or other physical features, surrounding land uses, or the site's separation from the water;
- ii. The proposed use does not usurp or displace land currently occupied by a water-oriented use and will not interfere with adjacent water-oriented uses;
  - iii. The proposed use will be of appreciable public benefit by increasing ecological functions together with public use,

enjoyment, or access to the shoreline.

- c. Non-water-oriented accessory uses (e.g., offices and parking areas that are part of recreational facilities) should be located landward of water-oriented facilities.
- d. Recreational facilities shall include features such as buffer strips, screening, fences, and signs, if needed to protect the value and enjoyment of adjacent or nearby private properties and natural areas from trespass, overflow and other possible adverse impacts.
- e. Recreation facilities shall demonstrate that they are located, designed, and operated in a manner consistent with the purpose of the shoreline environment designation in which they are located and will result in no net loss of shoreline ecological functions.
- f. Where fertilizers and pesticides are used in recreational developments, waters in and adjacent to such developments shall be protected from drainage and surface runoff.

#### FINDING(S):

- a. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- b. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.
- c. The proposal does not involve nonwatery-oriented recreational development, recreational conflicts with adjacent uses, or persistent use of fertilizers or pesticides.

CONCLUSIONS OF LAW: This project will comply with SMP 5.4.9 upon fullfillment of the conditions contained herein.

## <u>CRITERION §5.4.11.4 – TRANSPORTATION & PARKING FACILITIES REGULATIONS.</u> "a. Applications for redevelopment of transportation facilities in shoreline jurisdiction shall include:

- i. Analysis of alternative alignments or routes, including, where feasible, alignments or routes outside of shoreline jurisdiction;
  - ii. Description of construction, including location, construction type, and materials; and, if needed,
  - iii. Description of mitigation and restoration measures..
  - b. Proposed transportation projects shall plan, design, and locate where routes:
    - i. Will have the least possible adverse effect on unique or fragile shoreline features,
    - ii. Will not result in a net loss of shoreline ecological functions, and
    - iii. Will not adversely impact existing or planned water-dependent uses.
- c. Alternative designs for transportation facilities that have less impact on shoreline resources (i.e., narrower rights-of-way, realignment) shall be considered in compliance with the SMC.
- d. Roads and railroads of all types shall cross shoreline jurisdiction by the most direct route feasible, unless such a route would result in greater impacts on wetlands and fish and wildlife habitat conservation areas, or channel migration than a less direct route.
- e. Wherever feasible and in compliance with the SMC, transportation facilities, including local access roads and surface parking facilities, shall be shared across shoreline uses to reduce the need for redundant facilities.
  - f. New, replacement and enlarged transportation facilities shall provide public access pursuant to SMP Section 4.6.
- g. The City shall seek opportunities to obtain public easements and construct pedestrian connections over or under the railroad and state highway. The City shall place the pedestrian connection in its capital improvement plan and may require it as a condition of approval for Shoreline Permits, including permits involving new or replacement bridges and other transportation facilities.
- h. Primary parking facilities (pay parking lots, park-and-rides) are not allowed within shoreline jurisdiction. Accessory parking (including parking for vista purposes) and loading facilities necessary to support an authorized shoreline use are permitted.
- i. All of the following conditions shall be met when an accessory parking facility is proposed in the shoreline jurisdiction:
  - i. The facilities shall be located landward, adjacent to, beneath or within the building being served.
- ii. Upland parking facilities shall provide safe and convenient pedestrian circulation from the parking area to the shoreline.
- iii. Loading spaces for development in the shoreline jurisdiction shall be located on the landward or side wall of non-water-dependent uses or activities.

iv. All facilities shall provide parking suitable to the expected usage of the facility, with preference given to pavement or other dust-free all-weather surfaces.

v. All facilities shall be screened from adjacent, dissimilar uses through the use of perimeter landscaping, fencing, or some other approved material.

FINDING(S):

- a. The application contains detailed narratives, stormwater, landscape and mitigation plans contending consistency with this criterion.
- b. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.
- c. The proposal does not involve new, replacement, enlarged or redeveloped transportation facilities, roads, railroads, or primary parking facilities.

<u>CONCLUSIONS OF LAW:</u> This project will comply with SMP 5.4.11 without conditions.

#### **CHAPTER 6 – SHORELINE MODIFICATION PROVISIONS**

CRITERIA §6.1, §6.4.2, §6.4.3, §6.4.4, §6.4.5, §6.4.6. [These sections contain guidance applicable to all and/or different locations or project types than has been proposed. No consistency review is necessary.]

# <u>CRITERION §6.2.2 — GENERAL PROVISIONS FOR ALL SHORELINE MODIFICATIONS - REGULATIONS.</u> "All proposed shoreline modifications shall:

- a. Meet the mitigation sequencing requirements in SMP Section 4.3.
- b. Satisfy all specific shoreline modification provisions of this chapter.

<u>FINDING(S):</u> a. The findings of SMP Chapters 4, 5, and 6 are relevant to this criterion.

CONCLUSIONS OF LAW: This project will comply with SMP 6.2.2 upon satisfaction of the conditions contained herein.

<u>CRITERION §6.3 – GENERAL PROVISIONS FOR ALL SHORELINE MODIFICATIONS - REGULATIONS.</u> "The shoreline modification table below determines whether a specific shoreline modification is allowed within each of the shoreline environments. This table is intended to work in concert with the specific modification policies and regulations that follow, however, where there is a discrepancy between this table and the text of the SMP, the text shall take precedence....[Table 6.1 – Allowed Shoreline Modifications omitted for brevity]"

FINDING(S): a. The proposal involves Vegetation Removal, a permitted modification in the Active Waterfront SED.

CONCLUSIONS OF LAW: This project will comply with SMP 6.3 upon satisfaction of the conditions contained herein.

CRITERION §6.4.1.3 – VEGETATION REMOVAL - REGULATIONS. "a. Vegetation removal shall be limited to the minimum necessary to accommodate approved shoreline development that is consistent with all other provisions of this SMP. This includes the design, location, and operation of the structure or development, including septic drain fields, which shall minimize vegetation removal and meet all applicable requirements.

b. If removal of shoreline vegetation is unavoidable, vegetation removal shall be mitigated in accordance with the requirements in SMP Table 6.2 – Mitigation for Vegetation Removal within Shoreline Jurisdiction. Exceptions:

- i. The removal of native vegetation within established gardens, landscaping that serve a horticultural purpose shall not require mitigation under SMP Table 6.2.
- ii. Mitigation plans prepared by a qualified professional may establish mitigation ratios that deviate from SMP Table 6.2.
- c. No tree containing an active nest of an eagle, osprey, or other protected bird (as defined by WDFW or the Bald and

Golden Eagle Protection Act) shall be removed and the nest shall not be disturbed unless the applicant obtains approval from WDFW

- d. Vegetation removal conducted for the purposes outlined in SMC 18.13.025(D)(1)(a through d) shall comply with the regulations therein.
- e. Aquatic weed control shall be allowed only where the presence of aquatic weeds will affect native plant communities, fish and wildlife habitats, or an existing water dependent use adversely. Aquatic weed control efforts shall comply with all applicable laws and standards. [Table 6.2 Mitigation for Vegetation Removal within Shoreline Jurisdiction omitted for brevity]
- f. Mitigation Area, Location. The location of the mitigation area shall:
  - i. Be on site unless there is insufficient area on site;
  - ii. Improve an area of low habitat functionality;
  - iii. Be within 50 feet of the OHWM or as close as possible to the shoreline waterbody; and
  - iv. Prioritize south and west banks of waterbodies to provide shade.
- g. Mitigation Area, Monitoring.
  - i. The project shall be monitored annually for 5 years to document plant survivorship.
  - ii. Monitoring reports shall be provided to the Administrator once per year.
  - iii. The planted mitigation area shall achieve a plant survival standard of 80% at the end of 5 years.
- iv. Monitoring results may require additional/replacement planting to meet the survival standard. If the survival standard is not met, then additional planting may be required and the monitoring period extended.
- v. A conservation covenant may be established which prevents future development or alteration within the mitigation area."

#### FINDING(S):

- a. The proposal involves Vegetation Removal, a permitted modification in the Active Waterfront SED.
- b. Vegetation removal is unavoidable and subject to mitigation within and outside Fish & Wildlife Habitat Conservation Areas.
- c. The application contains detailed narratives, landscape and mitigation plans contending consistency with this criterion and SMP Table 6.2.
- d. The proposal does not involve trees with active nests of protected birds, vegetation removal under SMC 18.13.025(D)(1), or aquatic weed control.

CONCLUSIONS OF LAW: This project will comply with SMP 6.4.1 upon satisfaction of the conditions contained herein.

**CONDITIONS:** 

TO BE COLLATED UPON DECISION

#### **FINAL ORDER**

The preceding discussion describes the City's review of the information relevant to the SHOR2023-02 "Rock Cove Hospitality". The findings and conclusions of this document justify issuance of this Shoreline Substantial Development Permit under SMC 18.08.180. The project will be consistent with the policy and provisions of the SMA and the SMP upon satisfactions of the conditions listed herein.

DATED this ____ day of October 2023

## Jeff Breckel, Chair City of Stevenson Planning Commission

Action Expected	Condition	Performance Bond Eligible

Attachm	ents
1-	



#### Ben Shumaker <ben@ci.stevenson.wa.us>

#### SDP 968 SW Rock Creek Dr

**Tait, Meghan (ECY)** <mtai461@ecy.wa.gov> To: Ben Shumaker <ben@ci.stevenson.wa.us>

Fri, Sep 29, 2023 at 1:40 PM

Thank you for sending the critical areas report. Proposed development in the FWHCA does not trigger a variance per the City of Stevenson SMP. Below are my comments after reviewing the critical areas report.

The applicant must follow the required mitigation sequencing per SMP Section 4.3.2. This includes first avoiding impacts to the FWHCA, then minimizing impacts, and then providing compensatory mitigation for any unavoidable impacts. The applicant could increase avoidance of impacts to the FWHCA by reconfiguring the site plan and should avoid impacts to the greatest extent possible. The current site plan does not demonstrate proper mitigation sequencing as there is a large area outside of the FWHCA available for development. In addition, the proposed path should be located outside of the shoreline setback and FWHCA.

If the site plan is reconfigured, the buffer enhancement shall remain per City of Stevenson code 18.13.095(D)(3). This provision states that if the existing development causes the width of the remaining buffer to be less than 50%, the buffer remaining shall be enhanced. The buffer has been completely eliminated for much of the site following the functionally isolated buffer provision.

Thank you,

#### **Meghan Tait**

Wetland/Shoreland Specialist

Shorelands & Environmental Assistance Program

WA Department of Ecology | Vancouver Field Office

(360) 210-2783 | meghan.tait@ecy.wa.gov



This communication is public record and may be subject to disclosure as per the Washington State Public Records Act, RCW 42.56.

[Quoted text hidden]



#### Ben Shumaker <ben@ci.stevenson.wa.us>

#### SDP 968 SW Rock Creek Dr

**Tait, Meghan (ECY)** <mtai461@ecy.wa.gov>
To: Ben Shumaker <ben@ci.stevenson.wa.us>

Tue, Oct 3, 2023 at 11:02 AM

Hi Ben,

Thank you for providing the Department of Ecology the opportunity to review and comment on Rock Creek Hospitality located on parcels 02070100130300 and 02070100130200 in the City of Stevenson.

#### **Functionally Isolated Buffer**

The Fish and Wildlife Habitat Conservation Area (FWHCA) buffer is only functionally isolated when lawns, walkways driveways, or other mowed or paved areas, do not protect the FWHCA from adverse impacts due to pre-existing roads, structures or vertical separation (Stevenson CAO 18.13.095(D)(3)). The FWHCA buffer shall not be considered functionally isolated due to exposed dirt or vegetation clearing, such as the area of the property on the back side of the peninsula close to Rock Cove Drive. Any area where the remaining buffer is to be less than 50%, the reduced buffer shall be enhanced unless the area is utilized for activities consistent with water-dependent uses (Stevenson CAO 18.13.095(D) (3)(a)). The enhanced buffer area shall not count as mitigation and the buffer cannot be further reduced through averaging or on-site mitigation (Stevenson CAO 18.13.095(D)(3)(b)).

#### No Net Loss

The proposed development shall follow mitigation sequencing to achieve no net loss of shoreline ecological function. The required mitigation sequencing is first avoidance of impacts, including avoiding impacts to the FWHCA and native vegetation within shoreline jurisdiction, and then minimizing impacts to shoreline ecological function (SMP 4.3.2). The applicant should demonstrate how the project achieves no net loss, including how the site plan avoids and minimizes impacts to the FWHCA to the greatest extent possible.

Mitigation for vegetation removal within shoreline jurisdiction shall follow ratios outlined in SMP Table 6.2. The Critical Areas and FWHCA Report, dated May 3, 2023, prepared by Ecological Land Services Inc. does not address vegetation removal within shoreline jurisdiction or propose mitigation for vegetation removal following SMP Table 6.2.

#### **Shoreline Public Access**

If the proposed development is considered a water-related or water-enjoyment commercial use, the use must be open to the general public and the shoreline-oriented space within the project must be devoted to the specific aspects of the use that fosters shoreline enjoyment (WAC 173-26-020; SMP 7.2). It is unclear in the current site plan how the proposed development is providing public access.

Please let me know if I can provide further technical assistance or answer any questions.

#### **Meghan Tait**

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Shorelands & Environmental Assistance Program

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From: Ben Shumaker <ben@ci.stevenson.wa.us> Sent: Friday, September 29, 2023 1:14 PM

To: Tait, Meghan (ECY) <mtai461@ECY.WA.GOV>

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# State of Washington DEPARTMENT OF FISH AND WILDLIFE

Southwest Region 5 • 5525 South 11th St Ridgefield, WA 98642 Telephone: (360) 696-6211 • Fax: (360) 906-6776

October 6, 2023

City of Stevenson - Planning Department City Hall PO Box 371 Stevenson, WA 98648

Re: City of Stevenson - Notice of Public Hearing - Shorelines Substantial Development Permit

To whom it may concern,

Thank you for the opportunity to comment on the Shoreline Substantial Development Permit for 968 Rock Creek Drive (parcels 02070100130200, 02070100130300). Stevenson's Shoreline Master Program (SMP) was drafted in 2018, had public comment in 2019, and was approved in 2022. After the public comment period, the Washington Department of Fish and Wildlife (WDFW) published new riparian management recommendations entitled, Riparian Ecosystems, Volume 1: Science Synthesis and Management Implications (Quinn et al. 2020) and Riparian Ecosystems, Volume 2: Management Recommendations (Rentz et al. 2020). This is considered the Best Available Science (BAS) for managing riparian areas, so we would like to take this as an opportunity to offer direction for future projects. Additionally, we would like to acknowledge some concerns regarding riparian impacts, stormwater runoff, and shoreline armoring/bank protection. Below, we elaborate on these concerns and provide technical guidance for your consideration.

Riparian habitat provides valuable ecological functions including, but are not limited to: stream morphology, erosion and sedimentation process, fish and wildlife habitat availability, wood recruitment, stream temperature, shading, pollutant removal, and nutrient cycling (Quinn et al. 2020). WDFW's new riparian guidance recommends using Site Potential Tree Height at 200 years (SPTH200) to identify the riparian ecosystem that has the greatest functionality. Additionally, science supports a riparian width of 100ft as the distance that helps minimize pollutant runoff into streams (Rentz et al. 2020). SPTH200 utilizes the Natural Resource Conservation Service (NRCS) forest productivity site index values and is measured from the edge of the active floodplain/channel. Where NRCS data is missing, the riparian setback should encompass the width of the existing riparian vegetation community or the pollutant removal function of 100ft, whichever is greater. Since NRCS data is not available at this location, further evaluation is needed to determine the proper riparian setback to protect ecosystem functions. At a minimum, we do not recommend developing within 100ft of the ordinary high-water line to protect the pollutant removal function.

Additionally, this proposal will lead to an increase in shoreline use, allowing more opportunities for recreation and the chance for visitors to develop a greater appreciation for this scenic area. To minimize impacts to the riparian area with this increased traffic, we encourage constructing permeable trails and consolidating shoreline access. This area can be delineated with signage and educational materials that explain the importance of riparian ecosystems and functions. In addition, having an adequate number of trash receptacles will also be vital to minimizing human impact on the surrounding natural area. Implementing Best Management Practices (BMPs) will meet shoreline access objectives while protecting riparian ecosystem functions.

The impact of runoff from roadways on migrating salmon is of particular concern. In a study conducted by Tian et al (2021), they linked a chemical found in tires, commonly known at 6PPD-quinone, with decades of stormwater-linked coho mortality events in urban streams in the Pacific Northwest. Fragments of tire break off and are carried into streams from surface water runoff. While their study was focused on coho, they hypothesized that this compound was likely toxic to other aquatic organisms as well. The introduction of 6PPD-quinone into Rock Cove can be reduced by having effective stormwater management and ensuring riparian buffers are at least 100ft wide. Given this new research, we discourage any road surfaces within 100ft of the active floodplain/channel.

The Critical Areas and FWHCA Report (CAR) for the Rock Creek Cove Hospitality proposed development, prepared by Ecological Land Services (ELS) and dated May 3, 2023, describes the critical areas, potential impacts, and compensatory mitigation measures associated with this project. The CAR states approximately 65 percent of the shoreline is armored and satisfies the buffer exemption criteria per SMC 18.13.095(B)(3). If the existing riprap is being used as justification to lessen riparian protection, WDFW recommends further review to determine if the riprap was legally permitted and installed. We were unable to locate records that indicate if the riprap was legally permitted. Removing this riprap is a unique shoreline enhancement opportunity to restore shoreline functions. If bank protection is a concern, we encourage that the riprap is removed and replaced with a bioengineered solution to enhance shoreline function. Lastly, we recommend that the applicant evaluates design alternatives that do not impact the existing riparian vegetation. Avoiding impacts is required as outlined in SMC 18.13.055. Given the degraded nature of the riparian area, it is increasingly important to protect the existing vegetation. While enhancing the riparian area is beneficial, it does not mitigate for a reduction of riparian habitat.

Outside of providing technical assistance for land use proposals, WDFW issues Hydraulic Project Approval (HPA) permits for construction activities that impact the bed or bank of state waters. This includes issuing Emergency HPAs for bank stabilization when structures are threatened by erosion or flood events. Given the impacts of climate change, we precaution building close to floodplains. Building near floodplains increases risk to infrastructure so I would encourage being mindful of risks before approving developments. Ultimately, WDFW wants to avoid a situation where structures are approved for development near waters that will require future emergency protection. We encourage diligent review of this risk prior to approval.

We hope that these environmental impacts are considered and that the applicant considers alternative designs that further avoid impacts to riparian habitat. We are happy to provide additional technical assistance upon request.

Thank you,

Amaia Smith, Habitat Biologist

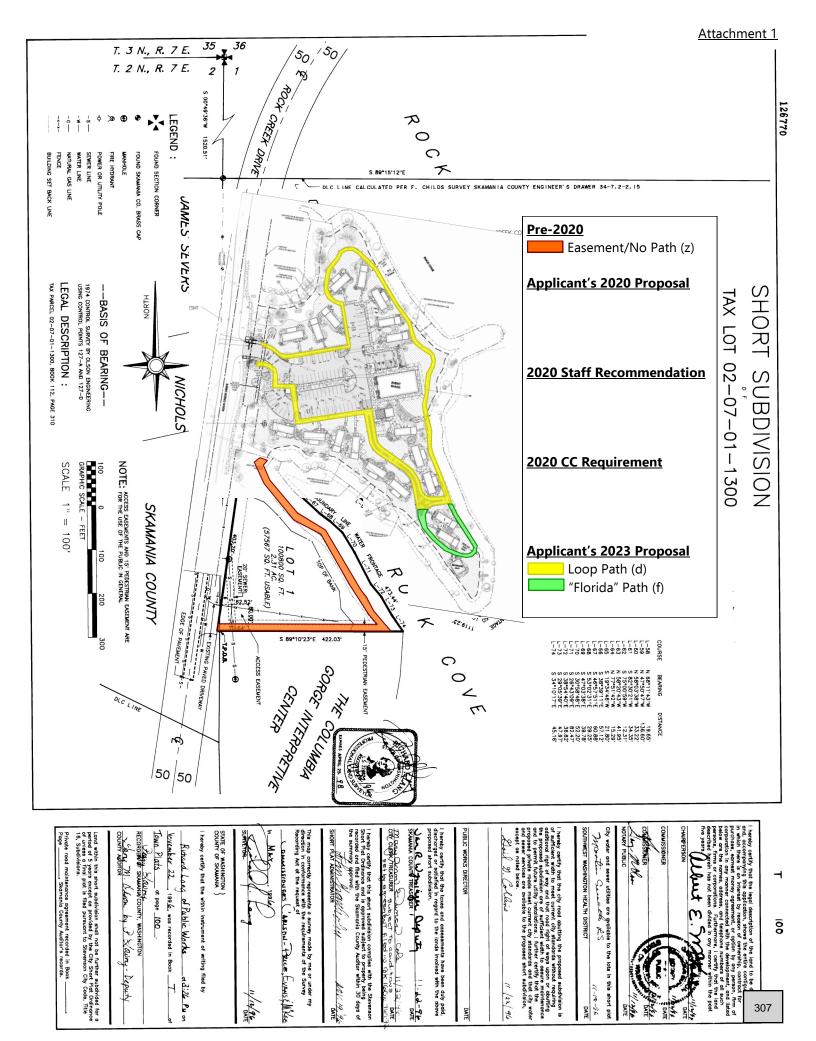
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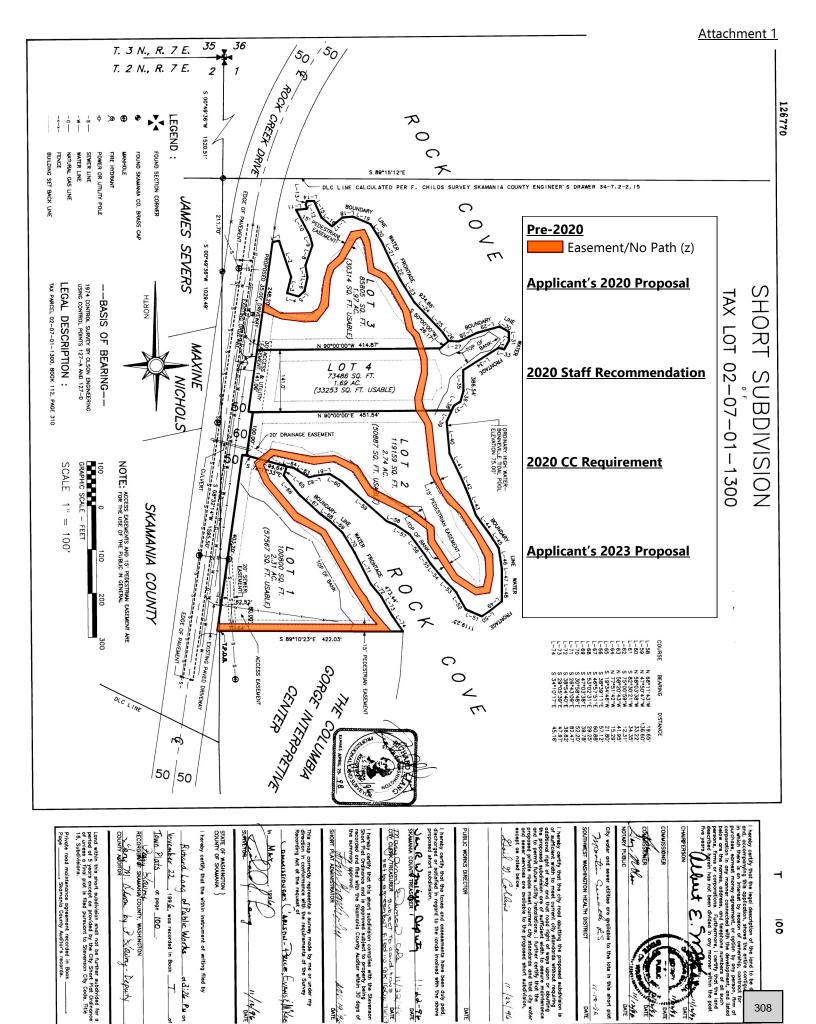
Washington Department of Fish and Wildlife

5525 S 11th St, Ridgefield, WA 98642

#### References

- Quinn, T., G.F. Wilhere, and K.L. Krueger, technical editors. 2020. Riparian Ecosystems, Volume 1: Science Synthesis and Management Implications. Habitat Program, Washington Department of Fish and Wildlife, Olympia
- Rentz, R., A. Windrope, K. Folkerts, and J. Azerrad. 2020. Riparian Ecosystems, Volume 2: Management Recommendations. Habitat Program, Washington Department of Fish and Wildlife, Olympia. https://wdfw.wa.gov/publications/01988
- Tian et al. 2021. A ubiquitous tire rubber–derived chemical induces acute mortality in Coho Salmon. Science, 371(6525), 185–189. https://doi.org/10.1126/science.abd6951





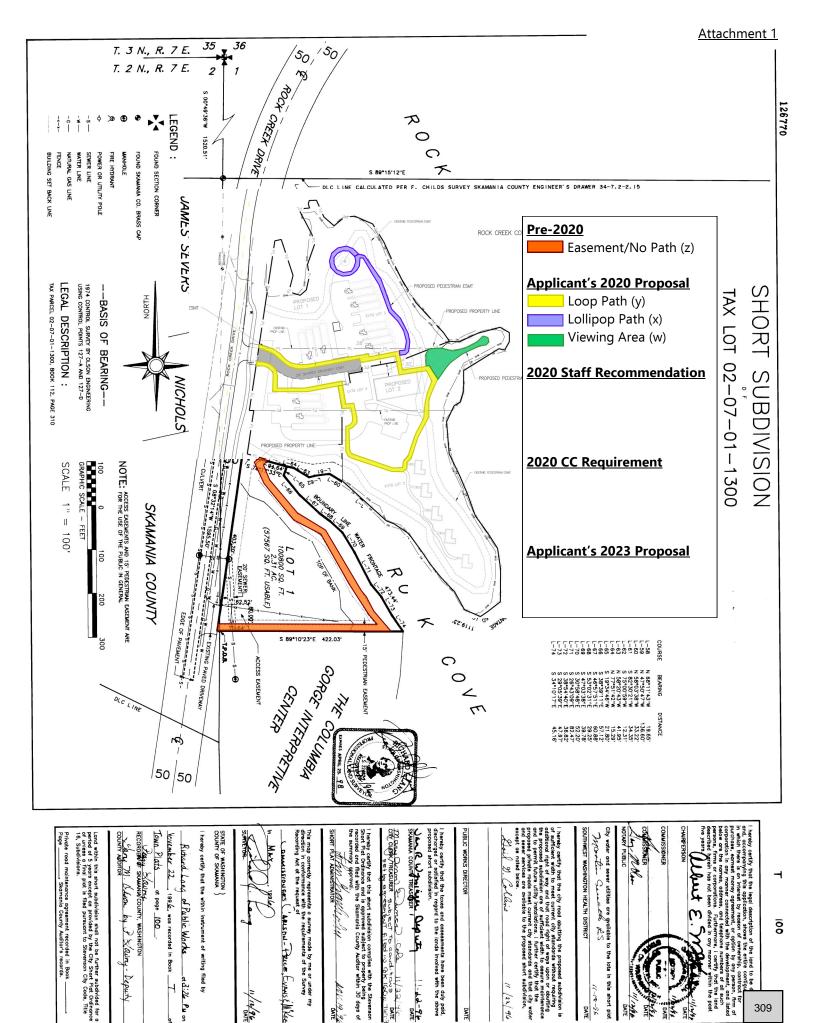
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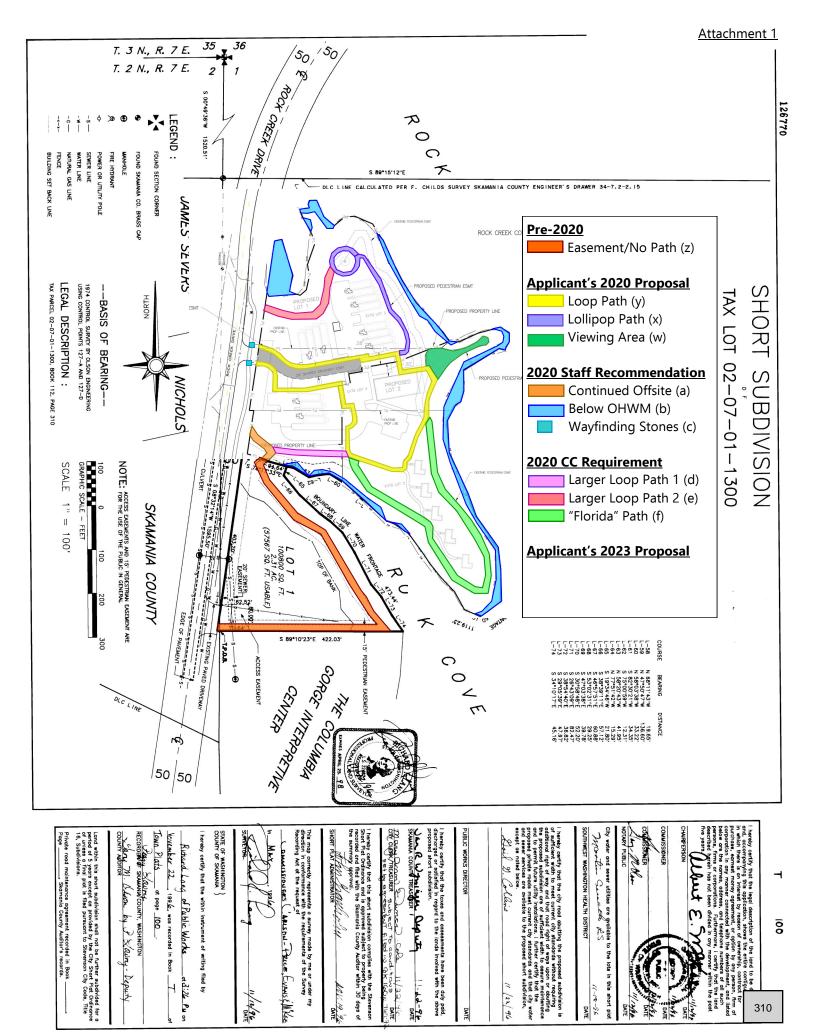


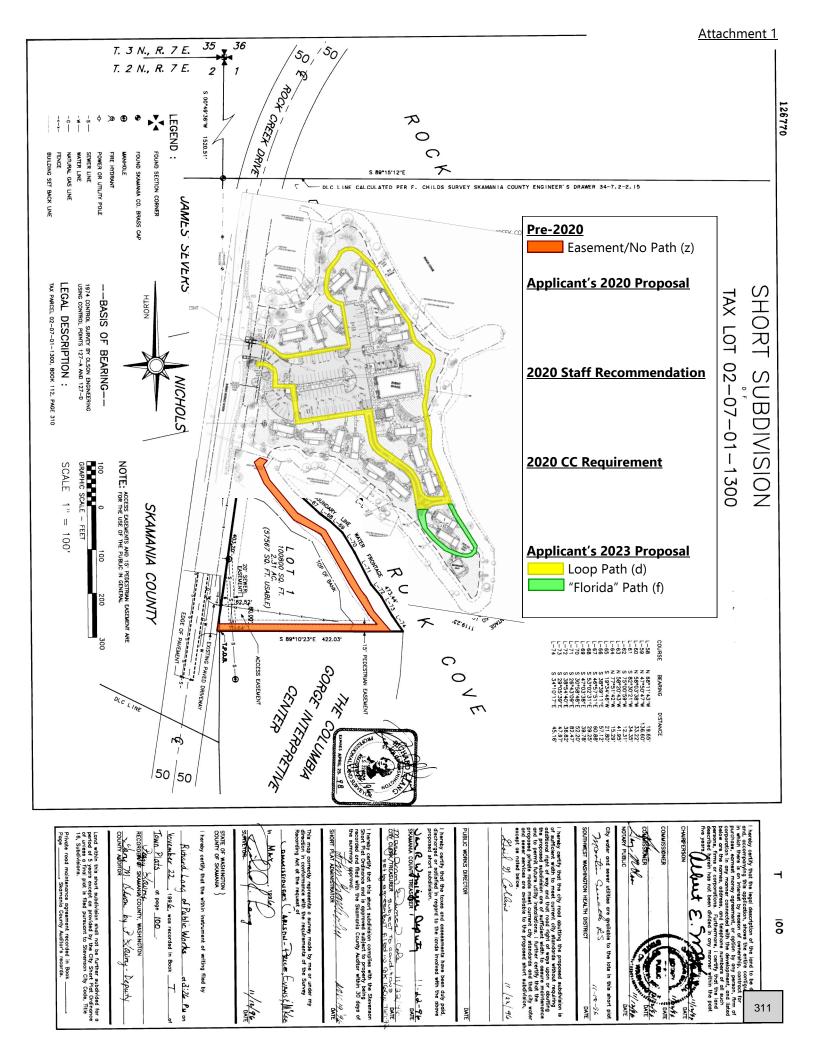
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### **ENHANCEMENT OPTIONS**

#### Shoreline Public Access & Trail Plan Goal Met:

Increase recreational opportunities for the public in the shoreline (RCW 90.58.020(6))

#### Sequence of Opportunities:

- City coordinates with private landowner and assisted living facility to understand opportunities and constraints for development of the existing pedestrian easements.
- City determines budget for shoreline enhancement options in coordination with the landowner
- City conducts public outreach to determine which enhancement options to prioritize
- City constructs and maintains shoreline recreation facilities



Description/Proposed Feature and Amenity	Proposed easement would allow for the extension of the		Category	Score	
	pedestrian trail along the shoreline, and a hand carry boat launch on the west side of the cove. Proposed features include:  1,000 LF of trail QTY: 1 new hand carry boat launch and boat wash station	GIS Score	5.3		
		Alignment with Existing Long Range Planning	Yes (1)		
	Remove boat launch		Community Support	4	
		Score Summary	20		
			Project Readiness Score	11	
Cost	\$549,000.				
Project Readiness	Coordination with multiple parties prior to design implementation  X Can be executed immediately X Enact by 2030 □ Enact by 2040 and beyond.  Collaboration can begin immediately. Design and construction could be possible by 2030.				
Public Access Type	X Physical Access (Beach/Boat Launch) X Visual (Trail/View Point) □ Other (Trailhead, Non-Physical, etc.)				
Project Type	☐ Maintenance/Rehabilitation ☐ Infrastructure Improvement ☐ New Infrastructure  X Restoration of Ecological Functions ☐ Acquisition/Easement ☐ Other — Educational resource				
Summary of Public Comments	The public comments were neutral to skeptical about the feasibility of this project; however, they also agreed it would be a popular and highly used public amenity if it were able to be constructed.				
Need Addressed	□ Continuous pedestrian experience □ Connection between districts □ Neighborhood Amenity □ Visitor Trailhead □ Non-motorized water access □ Reconnection to the Columbia River				
Proposed Next Steps	Work is partially on established public easements, city and WSDOT rights-of-way. The city would work with the landowner to provide shoreline trail easement adjustment to less environmentally complex locations for future public use, as well as a boat launch consideration. Include fish barrier removal study (Foster Creek) as part of this project. Work requires a moderate level of coordination between city, private property owner, and WSDOT.				
Permits Required	Shoreline Substantial Development Permit, Site Plan Application, and Critical Areas Checklist. Moderate permitting complexity is expected for this task. If launch and Foster Creek culvert replacement are considered, a US Army Corps of Engineers Section 404 permit, Ecology 401 Water Quality Certification and WDFW HPA permit will be required, making this a more complex effort.				
Mitigation Sequence & Environmental Impact	The recommended trail option was narrowed down to establish a set route around the shoreline in addition to signage and fencing, thereby minimizing impacts otherwise caused by having multiple routes in and around the shoreline. Shoreline vegetation impacted by this option will be mitigated for via enhancements in and around the shoreline. Further, the Foster Creek culvert evaluation can also be used to rectify the undersized culvert issue. No change to impact section.				
Potential Issues/ Additional Information	The shoreline is steep in parts. The trail could follow the top of slope to give public visual access to Rock Cove. Operations of Assisted living facility tend to discourage easier access to water. There are community concerns regarding the aesthetic quality (iron oxidizing bacteria) of the stormwater flowing at this location. Many large trees on the perimeter and shoreline areas of the site.				
Ongoing Maintenance & Estimated Annual Cost	To be determined.				
SMP Amendment	Not applicable.				
Comprehensive Plan Objectives Met	9.6, 9.7, 9.10 Pla	owntown an for JCCESS!	Not applicable.		
Recommended Option	Coordinate with private landowner and assisted for development of the existing pedestrian ease		ility to understand opportunities and	l constraints	